

STATE OF UTAH
DEPARTMENT OF ENVIRONMENTAL QUALITY
DIVISION OF WATER QUALITY
WATER QUALITY BOARD
P.O. BOX 144870
SALT LAKE CITY, UTAH 84114-4870

**Ground Water Discharge Permit
Permit No. UGW170004**

In compliance with the provisions of the Utah Water Quality Act, Title 19, Chapter 5, Utah Code Annotated 1953, as amended, the Act,

Jade Dalton
Lost Creek Hogs LLC
2.6 miles south of Circleville
Garfield County, Utah

Hereafter after referred to as Permittee, is granted a Ground Water Discharge and Construction Permit for the operation of a 3.7 million gallon containment basin at a Concentrated Animal Feeding Operation (CAFO). The containment basin is located on a tract of land encompassed in Sections 11 & 12, Township 31 South, Range 4 West, Salt Lake Base and Meridian, Garfield County, Utah.

This permit is based on representation made by the Permittee and other information contained in the administrative record. It is the responsibility of the Permittee to read and understand all provisions of this permit.

The facility shall be maintained and operated in accordance with conditions set forth in the permit and the Utah Administrative Rules for Ground Water Quality Protection (R317-6).

This permit shall become effective on August 9th, 2021

This permit and authorization to operate shall expire at midnight August 8th, 2026.

Signed this 6th day of August, 2021.



Erica Brown Gaddis, PhD
Director

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PART I CONSTRUCTION PERMIT ISSUANCE

A. AUTHORIZED CONSTRUCTION

As part of this ground water discharge permit, a construction permit is hereby issued to Dalton Finisher Farm as summarized below and detailed in Appendix A. Construction for this project will consist of four barns piped to a single 3.7-million-gallon basin.

B. DESIGN AND CONSTRUCTION

Under authority of the Utah Water Quality Act, Section 19-5-108(1) Utah Code Ann. 1953, as amended and Utah Administrative Code R317-1, the authorized facilities will be constructed in accordance with the engineering design plans and specifications attached as Appendix A.

Approved construction elements include:

- Construction of 4 Additional 1,100 Hog Finisher Barns (Barns). At the present time there are 8 existing Barns that contain up to 8800 Hogs. Therefore, with the additional 4 Barns the total number of Hogs would increase to 13,200 Hogs.
- Construction of a new 3.7 million waste containment basin (Pond 2) located North of the present waste containment basin (Pond 1). Waste water from all 12 barns will flow into Pond 1. A 12-inch pipe will be installed in Pond 1 at an elevation of 6175.25. The pipe will be installed in Pond 2 at an elevation of 6171.00 and waste water will gravity flow to Pond 2.
- The new containment basin will be lined with a Flexible Membrane Liner (FML)(60 mil High-Density Polyethylene HDPE).
- The waste water from both ponds will then be land applied to the fields that are part of Dalton Farm.
- All construction will be completed and tested in accordance with industry standard Construction Quality Assurance and Quality Control (CQA/QC) standards.

BAT Performance Monitoring - Best available technology monitoring will include minimum vertical freeboard.

- Minimum Vertical Freeboard – a minimum of 2 feet of vertical freeboard shall be maintained to ensure total containment.

Spill Containment - The permittee shall design, maintain and construct all pipelines and pumping facilities with a spill containment system that shall:

- Prevent any spills or leakage from any contact with the ground surface or ground water.

Any spill that does come into contact with the ground water that causes pollution or has the potential to cause pollution to waters of the state shall be reported in accordance with Part III.I.

- DWQ must approve the location of the monitoring wells to be installed near the containment basins.

PART II SPECIFIC CONDITIONS

A. GROUND WATER CLASSIFICATION

Based on the preliminary samples collected from the onsite monitoring wells in 2018, 2019, and 2020, Ground water at the CAFO site is classified as Class I Pristine Ground Water.

B. BACKGROUND GROUND WATER QUALITY

Background water quality values for the site are summarized in Table 1. Total dissolved solids (TDS) concentrations range from 112 to 210 milligrams per liter (mg/l) and no parameters are above Utah Ground Water Quality Standards in the area of Pond 1.

Table 1: Background Water Quality in Monitoring Wells (Units in mg/L)

Well ID	pH (units)	Total Dissolved Solids	Nitrate/Nitrite (as N)	Chloride	Bicarbonate
Dalton Farm MU	6.5-8.0	174.6	0.51	10.3	102.2

C. GROUND WATER PROTECTION LEVELS

Table 2 provides Ground Water Protection Levels for containment basin compliance monitoring wells Dalton Farm MU and Dalton Farm MD, and interim ground water protection levels for the new monitoring well (Dalton Farm MD2) which will be located down-gradient from Pond 2. No degradation of ground water greater than the Utah Ground Water Quality Standards will be allowed.

Table 2: Ground Water Protection Levels

Well ID	pH (units) ^(a)	Total Dissolved Solids ^(b)	Nitrate/Nitrite (as N) ^(b)	Ammonia ^(c)	Chloride ^(b)	Bicarbonate ^(b)
Dalton Farm MU	6.5-8.0	244.7	0.76	3	12.3	106.0
Dalton Farm MD	6.5-8.0	226.7	0.77	3	15.9	113.0
Dalton Farm MD2 ^(d)	6.5-8.0	226.7	0.77	3	15.9	113.0

(a) Equals Ground Water Quality Standard

(b) Based on mean x 1.25.

(c) Ammonia standard based on 0.25 x the EPA Lifetime Health Advisory of 30 mg/L

(d) Interim Ground Water Protection Levels for new down-gradient monitoring well.

D. BEST AVAILABLE TECHNOLOGY (BAT) STANDARD

The administration of this permit is founded on the use of Best Available Technology (BAT), in accordance with the requirements of UAC R317-6-1.3. The construction permit (PART I) issued with this discharge permit describes construction standards for the wastewater treatment containment basins. Compliance with the requirements for use of BAT will be demonstrated by construction, operation and maintenance of the containment basin according to the construction permit.

Achievement of these performance standards will be demonstrated by:

- 1) Only swine wastes may be disposed of in the containment basin.
- 2) No ground water degradation beyond permit limits established in Table 2 as measured by compliance monitoring wells.

- 3) Permitted Facilities. The facilities and equipment authorized under this permit are listed in Table 3.

TABLE 3: Permitted Facility and Components

Facility	Discharge Control Technology	Latitude	Longitude
Existing Containment Basin (Pond 1)	60-mil HDPE liner	N 38° 8' 2.86"	W 112° 15' 58.84"
New Containment Basin (Pond 2)	60-mil HDPE liner	<i>a</i>	<i>a</i>

a = pending completion of construction

4. Compliance Monitoring Wells – The Permittee shall monitor all compliance monitoring wells listed in Table 2 in accordance with the requirements under Part II.F below.
5. Protection of Monitoring Wells - All compliance monitoring wells must be protected from damage due to surface vehicular traffic or contamination due to surface spills, and shall be maintained in full operational condition for the life of this permit. Any compliance monitoring well that becomes damaged beyond repair or is rendered unusable for any reason will be replaced by the Permittee within 90 days or as directed by the Director.

E. BEST MANAGEMENT PRACTICES

- 1) The Permittee shall operate the facility such that the ground water quality standards (UAC R317-6-2) and ground water protection levels in Table 2 that were developed for this permit are not exceeded in the unconfined aquifer underlying the site, or other aquifers that may be impacted by facility operations. Utah ground water regulations also contain standards for contaminants such as metals, pesticides and volatile organic compounds. Accordingly, the Permittee must not discharge these or any other contaminants that could impair beneficial uses of the ground water.
- 2) Permittee shall ensure proper handling of plant wastewater, prompt cleanup of any releases, and an ongoing operation, inspection, and maintenance program for ancillary facilities associated with this permit.
- 3) Land application of containment basin wastes shall be managed in accordance with a Comprehensive Nutrient Management Plan (CNMP) developed by a certified nutrient management planner.

F. COMPLIANCE MONITORING REQUIREMENTS

1. General Provisions

- a) *Future Modification of the Monitoring Program* - If at any time the Director determines the monitoring program to be inadequate, Permittee shall submit within 30 days of receipt of written notice from the Director a modified monitoring plan that addresses the inadequacies noted by the Director.

- b) *Compliance Monitoring Period* - Monitoring shall continue upon issuance of this permit and throughout the term of this permit. For facilities that are constructed during the term of this permit, monitoring shall commence upon initiation of operation of the new facility.
- c) *Laboratory Approval* - All water quality analyses shall be performed by a laboratory certified by the State of Utah to perform such analysis.
- d) *Water Level Measurement* - In association with each well sampling event, water level measurements shall be made in each monitoring well prior to removal of any water from the well casing. These measurements will be made from a surveyed permanent single reference point clearly marked on the top of the well or surface casing. Measurements will be made to the nearest 0.01 foot.
- e) *Sampling Protocol* - Water quality samples will be collected, handled and analyzed in conformance with the current approved version of the *Water Quality Sampling, Handling, and Analysis Plan*. The results of ground water monitoring shall be reported in accordance with the schedule in Part II Section H.
- f) **Ground Water Analyses** - The following analysis shall be performed on all water samples collected from each **monitoring well**:
- i) Field Measurements: pH, specific conductance, temperature
 - ii) Laboratory Analysis:
 - Nitrate + Nitrite as nitrogen
 - Ammonia as nitrogen
 - Chloride
 - Bicarbonate
 - Total Dissolved Solids (TDS)
 - In addition, samples will be analyzed for the following major ions: carbonate, calcium, magnesium, potassium, and sodium.
 - g) **Wastewater Analyses** - The containment basin waste water shall be sampled annually prior to land application activity. The following analyses shall be performed on a representative wastewater sample from each **containment basin**:
 - i) Field Measurements: pH, specific conductance, temperature
 - ii) Laboratory Analysis:
 - Total Kjeldahl Nitrogen (TKN), ammonia (NH₃-N), nitrate+nitrite as nitrogen, sulfate, chloride, Total Dissolved Solids (TDS), sodium, potassium, calcium, magnesium, bicarbonate, phosphorus (Total as P).
- h) *Monitoring Frequency* – All new monitoring wells shall be sampled for the parameters listed in Part II.D.4(f) above on an accelerated quarterly basis until a minimum of eight (8) events have been completed to establish

the background ground water quality. Existing monitoring wells shall be sampled on a semi-annual basis once the accelerated background monitoring period is complete.

Sample collection, handling, and analysis shall be conducted in accordance with the most recently revised and approved version of *Water Quality Sampling, Handling, and Analysis Plan*. Analyses for nitrogen species shall be conducted at the same laboratory. Results of the containment basin wastewater performance monitoring accompanied by any supporting raw data shall be submitted to the Division of Water Quality with the next Ground Water Quality Monitoring Report.

- i) Certified Laboratory - All laboratory analyses shall be performed by a laboratory certified by the State of Utah in accordance with UAC R317-6-6.3.L.

2. Damage to Monitoring Wells

If a monitoring well is damaged, is otherwise rendered inadequate for its intended purpose, or if a previous hydraulic gradient between two monitor wells is reversed, the Director shall be notified in writing within five days of the Permittee becoming aware of the condition.

3. BAT Performance Monitoring Program

Permittee shall conduct a containment basin inspection and maintenance program. Documentation of compliance with this program shall be maintained on site for review by representatives of the Division.

G. NON-COMPLIANCE STATUS

1. Probable Out-of-Compliance Status - The permittee shall evaluate results of each ground water sampling event to determine any exceedance of the Ground Water Protection Levels found in Table 2 above. Upon determination that a Ground Water Protection Level has been exceeded in the ground water, the permittee shall:
 - a. Immediately re-sample the monitoring well found to be in probable out-of-compliance status for laboratory analysis of the exceeded protection level parameter(s). Submit the analytical results thereof, and notify the Director of the probable out-of-compliance status within 30 days of the initial detection.
 - b. Upon exceedance of any one parameter listed in Table 2 for two consecutive sampling events, immediately implement an accelerated schedule of monthly sampling analysis, consistent with the requirements of this permit. This monthly sampling will continue for at least two months or until the compliance status can be determined by the Director. Reports of the results of this sampling will be submitted to the Director as soon as they are available, but not later than 30 days from each date of sampling.
2. Out-of-Compliance Status Based on Confirmed Exceedance of Permit Ground Water Protection Levels
 - a. Out of Compliance Status shall be defined as follows:

For parameters that have been defined as detectable in the background and for which protection levels have been established, out-of-compliance shall be defined as two consecutive samples exceeding the protection level and the mean background concentration by two standard deviations.

- b. Notification and Accelerated Monitoring - upon determination by the permittee or the Director, in accordance with UAC R317-6-6.17, that an out-of-compliance status exists, the permittee shall:
 - i) Verbally notify the Director of the out-of-compliance status or acknowledge Director that such a status exists within 24 hours of receipt of data, and
 - ii) Provide written notice within 5 days of the determination, and
 - iii) Continue an accelerated schedule of monthly ground water monitoring for at least two months and continue monthly monitoring until the facility is brought into compliance as determined by the Director.
- c. Source and Contamination Assessment Study Plan - within 30 days after the written notice to the Director required in Part II.G. 2.b.ii above, the permittee shall submit an assessment study plan and compliance schedule for:
 - i) Assessment of the source or cause of the contamination, and determination of steps necessary to correct the source.
 - ii) Assessment of the extent of the ground water contamination and any potential dispersion.
 - iii) Evaluation of potential remedial actions to restore and maintain ground water quality, and ensure that the ground water standards will not be exceeded at the compliance monitoring locations.
- 3. Out-of-Compliance Status Based Upon Failure To Maintain Best Available Technology - In the event that BAT monitoring indicates a violation of any of the technology or performance management standards outlined in Part II .D and E of this permit, the permittee shall submit to the Director a notification and description of the violation in accordance with Part II.G of this permit.

H. REPORTING REQUIREMENTS

- 1. Water Monitoring - monitoring required in Part II.F above shall be reported according to the schedule in Table 4 below, unless modified by the Director:

Table 4: Compliance Monitoring Report Schedule

<u>Monitoring Period</u>	<u>Report Due Date</u>
January through June	August 1
July thru December	February 1

2. Ground Water Quality Sampling - reporting will include:
 - a. Field Data Sheets - or copies thereof, including the field measurements and other pertinent field data, such as: sampling location name/number, date and time, names of sampling crew, type of sampling: pump or grab, volume of water purged before sampling.
 - b. Water Level Measurements - water level measurements from ground water monitoring wells will be reported as measured depth to ground water from the surveyed casing measuring point, and ground water elevations as converted by casing measuring point elevations.
 - c. Laboratory Analytical Results - including date sampled, date received; and the results of analysis for each parameter, including: value or concentration, units of measurement, reporting limit (minimum detection limit for the examination), analytical method, and the date of the analysis.
3. Electronic Filing Requirements - The permittee will submit the required ground water monitoring data in the electronic format specified by the Director. The data may be submitted by electronic transfer, e-mail, PDF, compact disc, or other approved transmittal mechanism. In addition, a submittal of the hard copy data may be made if the Permittee prefers.
4. Monitoring Well As-Built Report - For each new well-constructed the permittee shall submit diagrams and descriptions of the final completion of the monitoring wells. The report will be submitted in the next Groundwater sampling report after the well is completed. The report shall include:
 - a. Casing: depth, diameter, and type of material.
 - b. Screen: length, depth interval, diameter, material type, slot size.
 - c. Sand Pack: depth interval, material type and grain size.
 - d. Annular Seals: depth interval, material type.
 - e. Surface Casing and Cap: depth, diameter, material type, protection measures constructed.
 - f. Elevation and Location: ground surface elevation, elevation of water level measuring point, latitude and longitude in hours, minutes and seconds.
 - g. Well construction description, well completion description, results of well pump tests or slug tests.

I. COMPLIANCE SCHEDULE

1. Permit Compliance Schedule Item #1 The Permittee shall install one ground water monitoring well completed in the shallow aquifer immediately down-gradient of the new containment basin. The monitoring well will serve as one compliance mechanism for monitoring any containment basin discharge. DWQ will approve the final location of monitoring wells prior to issuing approval for the use of the new wastewater containment basin.

2. Permit Compliance Schedule Item #2 Comprehensive Nutrient Management Plan. The Permittee shall submit an updated Comprehensive Nutrient Management Plan that follows all NRCS guidelines for manure drying, handling, and application. The updated plan shall be submitted no later than 60 days following the permit renewal date. The plan must be approved by the Director prior to the land application of waste water from Pond 2.

3. Permit Compliance Schedule Item #3 Final Closure Plan. In the event that the permittee decides to discontinue its operations at the facility the permittee shall notify the Director of such a decision and submit a Final Closure Plan. The Final Closure Plan shall be submitted no later than 180 days prior to the closure of the facility. The permittee shall resubmit Final Closure Plans within 60 days of receipt of written notice of deficiencies therein. Any material changes made to this plan shall require final approval of the Director.

PART III MONITORING, RECORDING AND REPORTING REQUIREMENTS

- A. REPRESENTATIVE SAMPLING
Samples taken in compliance with the monitoring requirements established under Part II shall be representative of the monitored activity.
- B. ANALYTICAL PROCEDURES
Water sample analysis must be conducted according to test procedures specified under UAC R317-6-6.3.L, unless other test procedures have been specified in this permit.
- C. PENALTIES FOR TAMPERING
The Act provides that any person who falsifies, tampers with, or knowingly renders inaccurate, any monitoring device or method required to be maintained under this permit shall, upon conviction, be punished by a fine of not more than \$10,000 per violation, or by imprisonment for not more than six months per violation, or by both.
- D. REPORTING OF MONITORING RESULTS
Monitoring results obtained during each reporting period specified in the permit, shall be submitted to the Director, Utah Division of Water Quality at the following address no later than the 15th day of the month following the completed reporting period:
State of Utah
Division of Water Quality
PO Box 144870
Salt Lake City, Utah 84114-4870
Attention: Ground Water Protection Section
Electronic document submission: <https://deq.utah.gov/water-quality/water-quality-electronic-submissions>.
- E. COMPLIANCE SCHEDULES
Reports of compliance or noncompliance with, or any progress reports on interim and final requirements contained in any Compliance Schedule of this permit shall be submitted no later than 14 days following each schedule date.
- F. ADDITIONAL MONITORING BY THE PERMITTEE
If the permittee monitors any pollutant more frequently than required by this permit, using approved test procedures as specified in this permit, the results of this monitoring shall be included in the calculation and reporting of the data submitted. Such increased frequency shall also be indicated.
- G. RECORDS CONTENTS
Records of monitoring information shall include:
1. The date, exact place, and time of sampling or measurements;
 2. The individual(s) who performed the sampling or measurements;
 3. The date(s) and time(s) analyses were performed;
 4. The individual(s) who performed the analyses;
 5. The analytical techniques or methods used; and,
 6. The results of such analyses.

H. RETENTION OF RECORDS

The permittee shall retain records of all monitoring information, including all calibration and maintenance records and copies of all reports required by this permit, and records of all data used to complete the application for this permit, for a period of at least three years from the date of the sample, measurement, report or application. This period may be extended by request of the Director at any time.

I. TWENTY-FOUR HOUR NOTICE OF NONCOMPLIANCE REPORTING

1. The permittee shall verbally report any noncompliance which may endanger public health or the environment as soon as possible, but no later than 24 hours from the time the permittee first became aware of the circumstances. The report shall be made to the Utah Department of Environmental Quality 24 hour number, (801) 536-4123, or to the Division of Water Quality, Ground Water Protection Section at (801) 536-4300, during normal business hours (Monday through Friday 8:00 am - 5:00 pm Mountain Time).
2. A written submission shall also be provided to the Director within five days of the time that the permittee becomes aware of the circumstances. The written submission shall contain:
 - a. A description of the noncompliance and its cause;
 - b. The period of noncompliance, including exact dates and times;
 - c. The estimated time noncompliance is expected to continue if it has not been corrected; and,
 - d. Steps taken or planned to reduce, eliminate, and prevent reoccurrence of the noncompliance.
3. Reports shall be submitted to the addresses in Part III.D, Reporting of Monitoring Results.

J. OTHER NONCOMPLIANCE REPORTING

Instances of noncompliance not required to be reported within 24 hours, shall be reported at the time that monitoring reports for Part II.H are submitted.

K. INSPECTION AND ENTRY

The permittee shall allow the Director, or an authorized representative, upon the presentation of credentials and other documents as may be required by law, to:

1. Enter upon the permittee's premises where a regulated facility or activity is located or conducted, or where records must be kept under the conditions of the permit;
2. Have access to and copy, at reasonable times, any records that must be kept under the conditions of this permit;
3. Inspect at reasonable times any facilities, equipment (including monitoring and control equipment), practices, or operations regulated or required under this permit; and,
4. Sample or monitor at reasonable times, for the purpose of assuring permit compliance or as otherwise authorized by the Act, any substances or parameters at any location.

PART IV COMPLIANCE RESPONSIBILITIES

- A. DUTY TO COMPLY
The permittee must comply with all conditions of this permit. Any permit noncompliance constitutes a violation of the Act and is grounds for enforcement action; for permit termination, revocation and reissuance, or modification; or for denial of a permit renewal application. The permittee shall give advance notice to the Director of any planned changes in the permitted facility or activity which may result in noncompliance with permit requirements.
- B. PENALTIES FOR VIOLATIONS OF PERMIT CONDITIONS
The Act provides that any person who violates a permit condition implementing provisions of the Act is subject to a civil penalty not to exceed \$10,000 per day of such violation. Any person who willfully or negligently violates permit conditions is subject to a fine not exceeding \$25,000 per day of violation. Any person convicted under Section 19-5-115(2) of the Act a second time shall be punished by a fine not exceeding \$50,000 per day. Nothing in this permit shall be construed to relieve the permittee of the civil or criminal penalties for noncompliance.
- C. NEED TO HALT OR REDUCE ACTIVITY NOT A DEFENSE
It shall not be a defense for a permittee in an enforcement action that it would have been necessary to halt or reduce the permitted activity in order to maintain compliance with the conditions of this permit.
- D. DUTY TO MITIGATE
The permittee shall take all reasonable steps to minimize or prevent any discharge in violation of this permit which has a reasonable likelihood of adversely affecting human health or the environment.
- E. PROPER OPERATION AND MAINTENANCE
The permittee shall at all times properly operate and maintain all facilities and systems of treatment and control (and related appurtenances) which are installed or used by the permittee to achieve compliance with the conditions of this permit. Proper operation and maintenance also include adequate laboratory controls and quality assurance procedures. This provision requires the operation of back-up or auxiliary facilities or similar systems which are installed by a permittee only when the operation is necessary to achieve compliance with the conditions of the permit.

PART V GENERAL REQUIREMENTS

- A. PLANNED CHANGES
The permittee shall give notice to the Director as soon as possible of any planned physical alterations or additions to the permitted facility. Notice is required when the alteration or addition could significantly change the nature of the facility or increase the quantity of pollutants discharged.
- B. ANTICIPATED NONCOMPLIANCE
The permittee shall give advance notice of any planned changes in the permitted facility or activity which may result in noncompliance with permit requirements.
- C. PERMIT ACTIONS
This permit may be modified, revoked and reissued, or terminated for cause. The filing of a request by the permittee for a permit modification, revocation and reissuance, or termination, or a notification of planned changes or anticipated noncompliance, does not stay any permit condition.
- D. DUTY TO REAPPLY
If the permittee wishes to continue an activity regulated by this permit after the expiration date of this permit, the permittee must apply for and obtain a permit renewal or extension. The application should be submitted at least 180 days before the expiration date of this permit.
- E. DUTY TO PROVIDE INFORMATION
The permittee shall furnish to the Director, within a reasonable time, any information which the Director may request to determine whether cause exists for modifying, revoking and reissuing, or terminating this permit, or to determine compliance with this permit. The permittee shall also furnish to the Director, upon request, copies of records required to be kept by this permit.
- F. OTHER INFORMATION
When the permittee becomes aware that it failed to submit any relevant facts in a permit application, or submitted incorrect information in a permit application or any report to the Director, it shall promptly submit such facts or information.
- G. SIGNATORY REQUIREMENTS
All applications, reports or information submitted to the Director shall be signed and certified.
1. All permit applications shall be signed as follows:
 - a. For a corporation: by a responsible corporate officer;
 - b. For a partnership or sole proprietorship: by a general partner or the proprietor, respectively.
 - c. For a municipality, State, Federal, or other public agency: by either a principal executive officer or ranking elected official.

2. All reports required by the permit and other information requested by the Director shall be signed by a person described above or by a duly authorized representative of that person. A person is a duly authorized representative only if:
 - a. The authorization is made in writing by a person described above and submitted to the Director, and,
 - b. The authorization specified either an individual or a position having responsibility for the overall operation of the regulated facility or activity, such as the position of plant manager, operator of a well or a well field, superintendent, position of equivalent responsibility, or an individual or position having overall responsibility for environmental matters for the company. (A duly authorized representative may thus be either a named individual or any individual occupying a named position.)
3. Changes to Authorization. If an authorization under Part V.G.2 is no longer accurate because a different individual or position has responsibility for the overall operation of the facility, a new authorization satisfying the requirements of Part V.G.2 must be submitted to the Director prior to or together with any reports, information, or applications to be signed by an authorized representative.
4. Certification. Any person signing a document under this section shall make the following certification:

"I certify under penalty of law that this document and all attachments were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gather and evaluate the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate, and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations."

H. PENALTIES FOR FALSIFICATION OF REPORTS

The Act provides that any person who knowingly makes any false statement, representation, or certification in any record or other document submitted or required to be maintained under this permit, including monitoring reports or reports of compliance or noncompliance shall, upon conviction be punished by a fine of not more than \$10,000 per violation, or by imprisonment for not more than six months per violation, or by both.

I. AVAILABILITY OF REPORTS

Except for data determined to be confidential by the permittee, all reports prepared in accordance with the terms of this permit shall be available for public inspection at the offices of the Director. As required by the Act, permit applications, permits, effluent data, and ground water quality data shall not be considered confidential.

J. PROPERTY RIGHTS

The issuance of this permit does not convey any property rights of any sort, or any exclusive privileges, nor does it authorize any injury to private property or any invasion of personal rights, nor any infringement of federal, state or local laws or regulations.

K. SEVERABILITY

The provisions of this permit are severable, and if any provision of this permit, or the application of any provision of this permit to any circumstance, is held invalid, the application of such provision to other circumstances, and the remainder of this permit, shall not be affected thereby.

L. TRANSFERS

This permit may be automatically transferred to a new permittee if:

1. The current permittee notifies the Director at least 30 days in advance of the proposed transfer date;
2. The notice includes a written agreement between the existing and new permittee containing a specific date for transfer of permit responsibility, coverage, and liability between them; and,
3. The Director does not notify the existing permittee and the proposed new permittee of his or her intent to modify, or revoke and reissue the permit. If this notice is not received, the transfer is effective on the date specified in the agreement mentioned in paragraph 2 above.

M. STATE LAWS

Nothing in this permit shall be construed to preclude the institution of any legal action or relieve the permittee from any responsibilities, liabilities, penalties established pursuant to any applicable state law or regulation under authority preserved by Section 19-5-117 of the Act.

N. REOPENER PROVISION

This permit may be reopened and modified (following proper administrative procedures) to include the appropriate limitations and compliance schedule, if necessary, if one or more of the following events occurs:

1. If new ground water standards are adopted by the Board, the permit may be reopened and modified to extend the terms of the permit or to include pollutants covered by new standards. The permittee may apply for a variance under the conditions outlined in R317-6-6.4.D.
2. If alternative compliance mechanisms are required.
3. If subsequent ground water monitoring data reveals the background water quality values in Part II Table 1 are not accurate.

APPNEDIX A CONSTRUCTION PERMIT ISSUANCE

**CONSTRUCTION PERMIT
PLANS AND SPECIFICATIONS**



State of Utah

SPENCER J. COX
Governor

DEIDRE HENDERSON
Lieutenant Governor

Department of
Environmental Quality

Kimberly D. Shelley
Executive Director

DIVISION OF WATER QUALITY
Erica Brown Gaddis, PhD
Director

June 29, 2021

Jade Dalton
Dalton Finisher Farm Sites
Dalton Hay Company, LLC.
PO Box 189
Circleville, UT 84723
Via Email

Subject: **Construction Permit** for Dalton Finisher Farm Addition

Dear Mr. Dalton:

On May 4, 2021, the Division of Water Quality (DWQ) received submission of the Utah Groundwater Discharge Permit Application for the Dalton Finisher Farm Addition. This submission included the engineering plans and specifications for the Construction Permit. This submission was prepared by GEM Engineering, Inc. and signed by Joel A. Myers, P.E.

The review of the engineering plans and specifications indicated that a few minor changes would be required before the Construction Permit could be issued. These were communicated by Woodrow Campbell to Joel Myers. In response on June 15, 2021, Mr. Myers sent in a new drawing with the cross sections of the new pond and corrected HDPE liner specifications. These have been reviewed and found to answer all of the required changes and the submittal for the Construction Permit is now complete.

The following is a summary of the proposed major construction projects:

- Construction of 4 Additional 1100 Hog Finisher Barns (Barns). At the present time there are 8 existing Barns that contain up to 8800 Hogs. Therefore, with the additional 4 Barns the total number of Hogs would increase to 13,200 Hogs.
- Construction of a new 3.7 million waste containment basin (Pond 2) located North of the present waste containment basin (Pond 1). Waste water from all 12 barns will flow into Pond 1. A 12-inch pipe will be installed in Pond 1 at an elevation of 6175.25. The pipe will be installed in Pond 2 at an elevation of 6171.00 and waste water will gravity flow to Pond 2.
- The waste water from both ponds will then be land applied to the fields that are part of Dalton Farm.

The plans and specifications, as submitted, comply with *the Utah Water Quality Rules, (R317, Utah Administrative Code)*. A **Construction Permit** is hereby issued as constituted by this letter, subject to the following conditions:

- 1. Any revisions or modifications to the approved plans and specifications must be submitted to DWQ for review and approval, before construction or implementation thereof. Please submit any changes for review and approval directly to Woodrow Campbell, P.E., of the DWQ Ground Water Protection Section.*
- 2. A written operations and maintenance manual, containing a description of the functioning of the facilities, an outline of routine maintenance procedures, and all checklists and maintenance logs needed for proper operation of the system, must be submitted and approved before the final inspection and operation of the system.*
- 3. The approved facilities must not be placed in service unless DWQ has conducted a final inspection, reviewed and approved the As-Built Construction Certification Report, and provided written authorization to place the constructed facilities in service.*
- 4. Construction activities that disturb one acre or more are required to obtain coverage under the Utah Pollutant Discharge Elimination System (UPDES) Storm Water General Permit for Construction Activities. The permit requires the development of a storm water pollution prevention plan (SWPPP) to be implemented and updated from the commencement of any soil disturbing activities at the site until final stabilization of the project. For more information, or to obtain permit coverage on-line, please go to: <https://deq.utah.gov/water-quality/storm-water-permits-updes-permits>*

The plans and specifications for this project have been stamped and signed by a Professional Engineer currently licensed to practice in the state of Utah. The construction design, inspection supervision, and written construction certification of all work associated with this Construction Permit must be performed by a Professional Engineer licensed to practice in the state of Utah.

This Construction Permit will expire one year from the date of its issuance, as evidenced by the date of this letter, unless substantial progress is made in constructing the approved facilities or the plans and specifications have been resubmitted and the construction permit is reissued. This permit does not relieve you, in any way, of your obligations to comply with other applicable local requirements. You may contact Southwest Utah Public Health Department at (435-673-3528) or Paul Wright Southwest District Engineer at (435-986-2562) for further assistance regarding local matters.

Because of the inherent hazard potential at lagoons and ponds, warning signs should be posted at these facilities to state the dangers of drowning and asphyxiation. Safety ropes (or equivalent) running down the pond side slopes, and fastened to posts at the top of the dikes should be available to allow anyone trapped in the ponds to escape.

Please contact Mr. Campbell at the beginning of construction to allow periodic inspections to be scheduled. Upon completion of the project, a final inspection and approval of the As-Built Construction Certification Report is required before the approval to operate the completed facilities can be issued. Please remain in contact with Mr. Campbell to schedule the final inspection. The Construction Certification Report with final as-built drawings must include test results for the following construction quality assurance and quality control (CQA/QC) elements:

Soil Subgrade

- Proctor Curves,
- Soil Classification,
- Field Compaction Testing, and
- Subgrade Acceptance Certification.

Concrete

- Concrete Mix Verification,
- Concrete ASTM Testing Method, Frequency, and Results,
- Concrete Testing Pass/Fail Criteria, and
- Crack Inspection and Repair.

Flexible Membrane Liner

- Panel Placement Log,
- Trial Seam Test Log,
- Seaming Record,
- Seam Test Record,
- Repair Log,
- As-Built Drawing,
- Manufactures Certification including QA/QC Testing of the Rolls, and
- Professional Engineer Certification.

If we can be of further assistance, please contact Mr. Woodrow Campbell at wwcampbell@utah.gov or (801) 536-4353.

Sincerely,

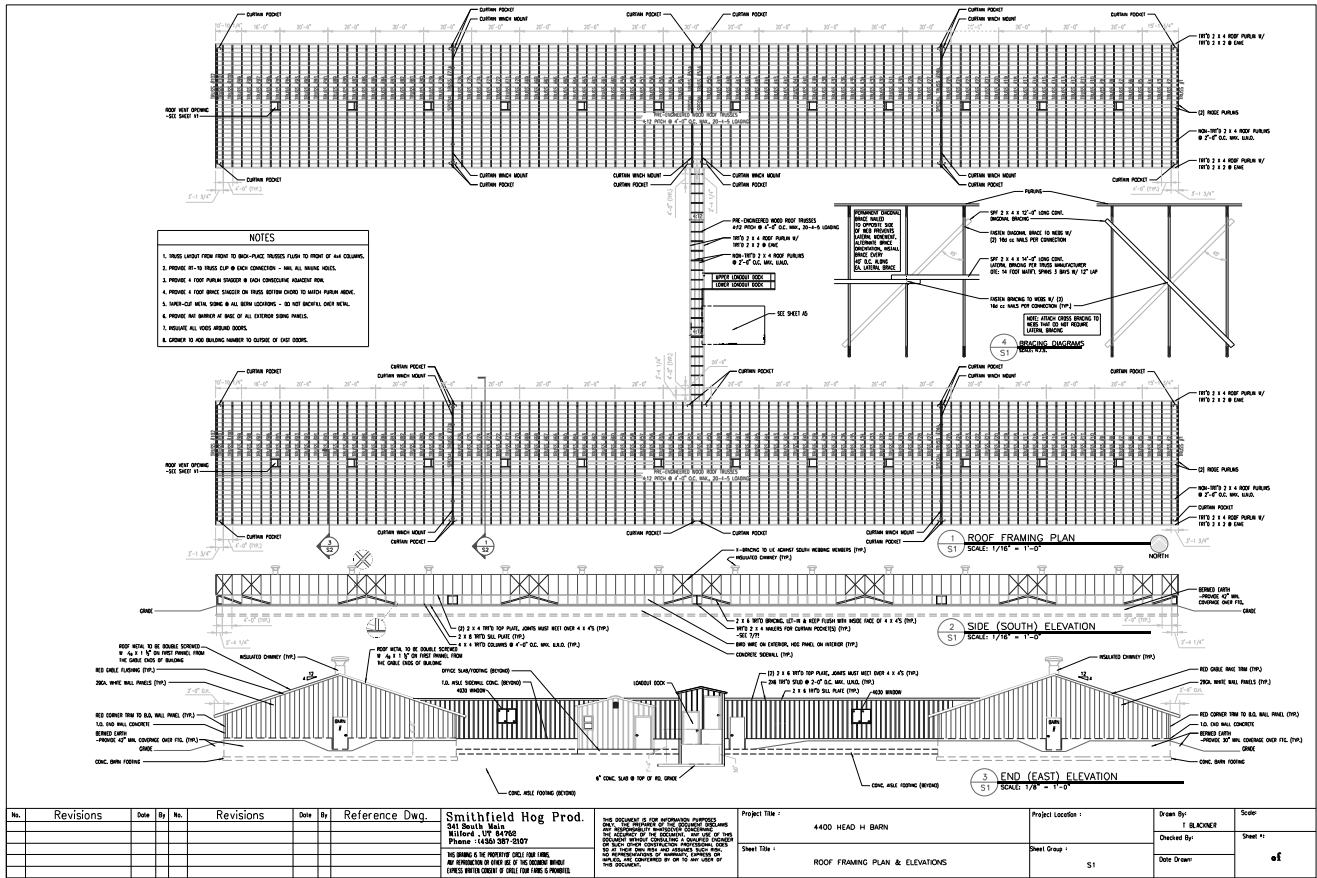


Erica Brown Gaddis, PhD
Director

EBG/WWC/DJH/blj

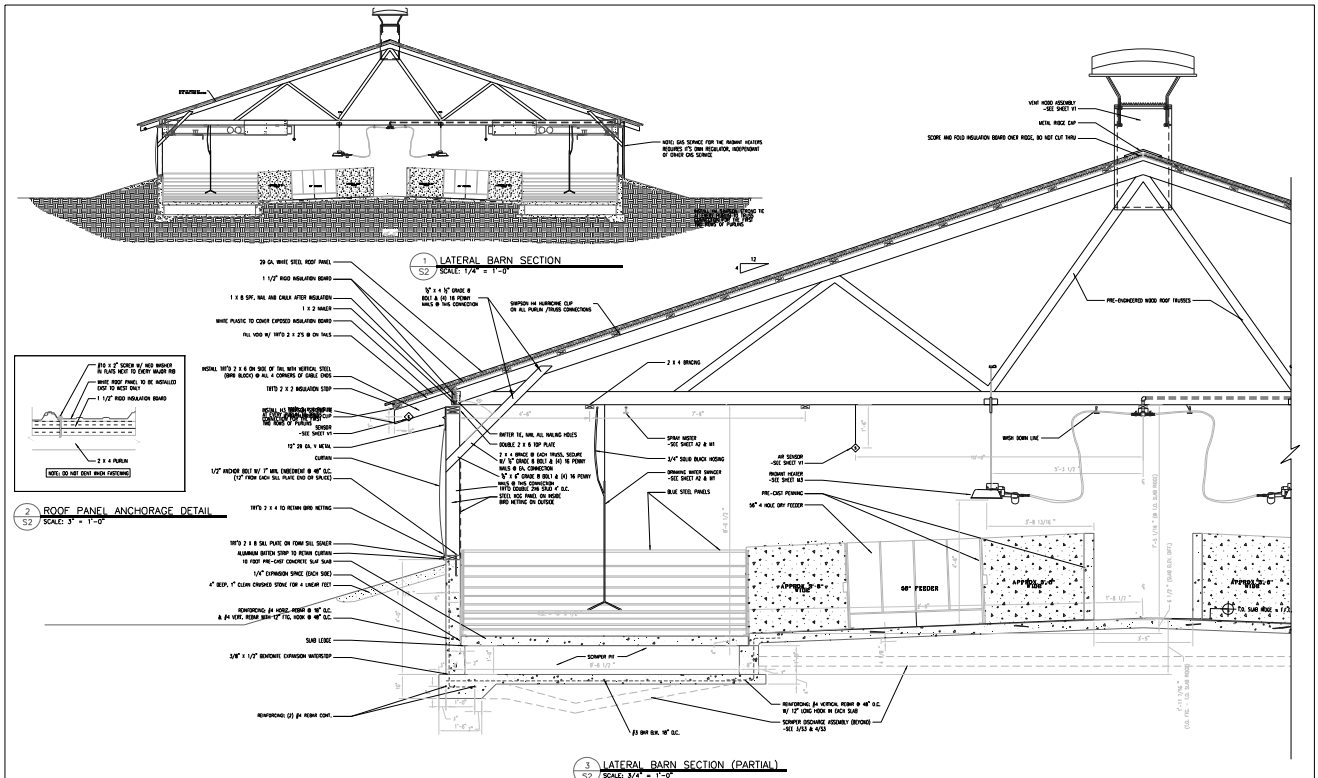
Enclosures: 1. Dalton Finisher Farms Plans (DWQ-2021-012545)
2. Dalton Finisher Farms Specifications (DWQ-2021-012547)

Cc: Via Email w/out Enclosures
Southwest Utah Public Health Department
Paul Wright Southwest District Engineer
Joel Myers, Gem Engineering, Inc.
Jim Webb



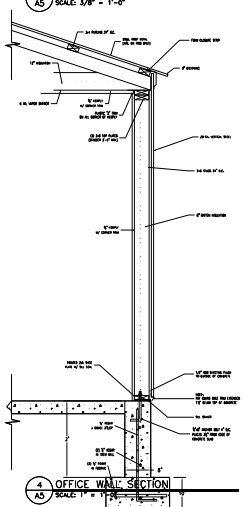
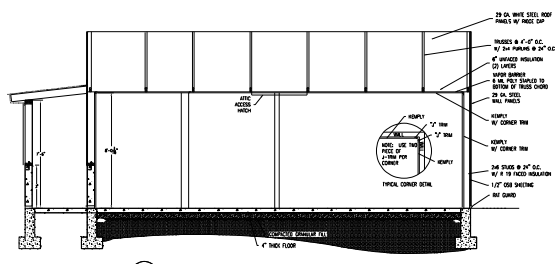
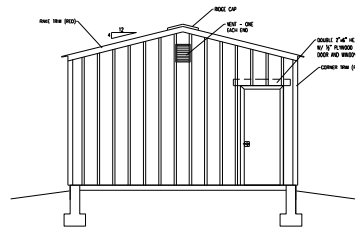
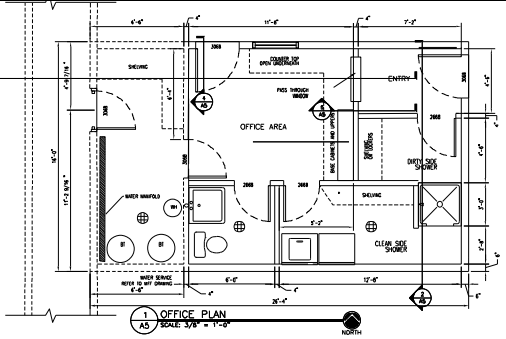
No.	Revisions	Date	By	No.	Revisions	Date	By	Reference Dwg.

Smithfield Hog Prod. 541 South Main Willford, VA 24387 Phone: 1-800-337-0007 NO BRIBING OR PAYMENTS TO BE MADE. ALL DIMENSIONS IN FEET OR INCHES UNLESS OTHERWISE SPECIFIED. UNLESS OTHERWISE SPECIFIED, ALL DIMENSIONS ARE TO FACE UNLESS NOTED OTHERWISE.	Project Title : 4400 HEAD H BARN Sheet Title : ROOF FRAMING PLAN & ELEVATIONS Project Location : Sheet Group : S1	Drawn By : E. SILVERMAN Checked By : Date Drawn : Scale :	Sheet #: 11 of 11
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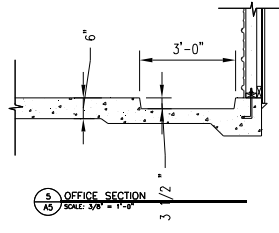


No.	Revisions	Date	By	No.	Revisions	Date	By	Reference Dwg.

Smithfield Hog Prod. 5455 South Main Willford, UT 84703 Phone: (435) 337-3327 NO CHANGES TO PROPERTY ORIGINALLY DRAWN. AN APPROVAL BY THESE OR ANY SIGNING ARCHITECT EXPRESSLY DENIES ANY LIABILITY FOR THE DESIGN OR CONSTRUCTION OF THIS PROJECT.	THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM ALL APPLICABLE AGENCIES AND AUTHORITIES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM ALL APPLICABLE AGENCIES AND AUTHORITIES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM ALL APPLICABLE AGENCIES AND AUTHORITIES.	Project Title : 4400 HEAD H. BARN	Project Location : 52	Drawn By : I. BLANCHER	Sheet No : of
SHEET TITLE : LATERAL SECTIONS & DETAILS	SHEET GROUP : 52	Checked By : 	Date Drawn : 	 	



- SPECIFICATIONS: CONCRETE CONTRACTOR**
- CONCRETE CONTRACTOR TO BE RESPONSIBLE FOR DESIGN OF CONCRETE IN THIS PROJECT.
 - CONCRETE CONTRACTOR TO PROVIDE ALL REINFORCING AND JOINTS WITH REINFORCING BARS AND JOINTS AS SHOWN ON DRAWINGS. ALL REINFORCING BARS SHALL BE EPOXY COATED UNLESS OTHERWISE NOTED.
 - CONCRETE CONTRACTOR TO PROVIDE ALL FORMWORK AND BRACING FOR ALL CONCRETE WORK. FORMWORK SHALL BE REINFORCED WITH 2\"/>
- SPECIFICATIONS: PLUMBING CONTRACTOR**
- PLUMBING CONTRACTOR TO BE RESPONSIBLE FOR DESIGN AND INSTALLATION OF ALL PLUMBING SYSTEMS IN THIS PROJECT.
 - PLUMBING CONTRACTOR TO PROVIDE ALL MATERIALS AND LABOR FOR ALL PLUMBING SYSTEMS. ALL MATERIALS SHALL BE APPROVED BY THE ARCHITECT PRIOR TO INSTALLATION.
 - PLUMBING CONTRACTOR TO PROVIDE ALL HANGERS AND BRACING FOR ALL PLUMBING SYSTEMS. HANGERS SHALL BE APPROVED BY THE ARCHITECT PRIOR TO INSTALLATION.
- SPECIFICATIONS: ELECTRICAL CONTRACTOR**
- ELECTRICAL CONTRACTOR TO BE RESPONSIBLE FOR DESIGN AND INSTALLATION OF ALL ELECTRICAL SYSTEMS IN THIS PROJECT.
 - ELECTRICAL CONTRACTOR TO PROVIDE ALL MATERIALS AND LABOR FOR ALL ELECTRICAL SYSTEMS. ALL MATERIALS SHALL BE APPROVED BY THE ARCHITECT PRIOR TO INSTALLATION.
 - ELECTRICAL CONTRACTOR TO PROVIDE ALL HANGERS AND BRACING FOR ALL ELECTRICAL SYSTEMS. HANGERS SHALL BE APPROVED BY THE ARCHITECT PRIOR TO INSTALLATION.
- SPECIFICATIONS: FRAMING CONTRACTOR**
- FRAMING CONTRACTOR TO BE RESPONSIBLE FOR DESIGN AND INSTALLATION OF ALL FRAMING SYSTEMS IN THIS PROJECT.
 - FRAMING CONTRACTOR TO PROVIDE ALL MATERIALS AND LABOR FOR ALL FRAMING SYSTEMS. ALL MATERIALS SHALL BE APPROVED BY THE ARCHITECT PRIOR TO INSTALLATION.
 - FRAMING CONTRACTOR TO PROVIDE ALL HANGERS AND BRACING FOR ALL FRAMING SYSTEMS. HANGERS SHALL BE APPROVED BY THE ARCHITECT PRIOR TO INSTALLATION.



No.	Revisions	Date	By	No.	Revisions	Date	By	Reference Dwg.	Smithfield Hog Prod.	Project Title :	Project Location :	Drawn By :	Scale :
									4400 HEAD H. BARN <td>4400 HEAD H. BARN <td>A4 <td>J. BLACKNER <td>1" = 1'-0"</td> </td></td></td>	4400 HEAD H. BARN <td>A4 <td>J. BLACKNER <td>1" = 1'-0"</td> </td></td>	A4 <td>J. BLACKNER <td>1" = 1'-0"</td> </td>	J. BLACKNER <td>1" = 1'-0"</td>	1" = 1'-0"
									OFFICE PLAN, ELEVATIONS, AND DETAILS <td></td> <td></td> <td>Checked By : <td>Sheet # :</td> </td>			Checked By : <td>Sheet # :</td>	Sheet # :
												Scale Drawn : <td>of</td>	of

MATERIAL LIST			
NO.	SYMBOL	DESCRIPTION	APPROXIMATE QTY
1	CONVET	ALL SYMBOLS ARE TO BE USED FOR SCHEDULES AS INDICATED OTHERWISE. ELECTRICAL CONTRACTOR SHALL FURNISH FOR CONNECTION AND CONNECTIONS.	
2	0	NO. CONDUIT SHALL BE RIGID PVC SCHEDULE 40 UNLESS OTHERWISE NOTED. ELECTRICAL CONTRACTOR SHALL FURNISH FOR CONNECTION AND CONNECTIONS.	SQUARE # 00
3	0	NO. CONDUIT SHALL BE RIGID PVC SCHEDULE 40 UNLESS OTHERWISE NOTED. ELECTRICAL CONTRACTOR SHALL FURNISH FOR CONNECTION AND CONNECTIONS.	SQUARE # 00
4	SP	BURIED VOLTAGE REGULATOR CONTROL, SINGLE PHASE, SINGLE THROAT, SPECIFICATION SHALL BE AS SHOWN ON SHEET 21.01.01. CONTRACTOR SHALL FURNISH AND INSTALL.	1
5	SS	BURIED VOLTAGE REGULATOR CONTROL, SINGLE PHASE, SINGLE THROAT, SPECIFICATION SHALL BE AS SHOWN ON SHEET 21.01.01. CONTRACTOR SHALL FURNISH AND INSTALL.	1
6	⊙	RECEPTACLE, DUPLEX, 125 VOLTS, 20 AMP, 3 WIRE, CONDUIT SHALL BE RIGID PVC SCHEDULE 40 UNLESS OTHERWISE NOTED. ELECTRICAL CONTRACTOR SHALL FURNISH AND INSTALL.	1
7	⊙	RECEPTACLE, DUPLEX, 125 VOLTS, 20 AMP, 3 WIRE, CONDUIT SHALL BE RIGID PVC SCHEDULE 40 UNLESS OTHERWISE NOTED. ELECTRICAL CONTRACTOR SHALL FURNISH AND INSTALL.	1
8	⊙	RECEPTACLE, DUPLEX, 125 VOLTS, 20 AMP, 3 WIRE, CONDUIT SHALL BE RIGID PVC SCHEDULE 40 UNLESS OTHERWISE NOTED. ELECTRICAL CONTRACTOR SHALL FURNISH AND INSTALL.	1
9	⊙	RECEPTACLE, DUPLEX, 125 VOLTS, 20 AMP, 3 WIRE, CONDUIT SHALL BE RIGID PVC SCHEDULE 40 UNLESS OTHERWISE NOTED. ELECTRICAL CONTRACTOR SHALL FURNISH AND INSTALL.	1
10	⊙	RECEPTACLE, DUPLEX, 125 VOLTS, 20 AMP, 3 WIRE, CONDUIT SHALL BE RIGID PVC SCHEDULE 40 UNLESS OTHERWISE NOTED. ELECTRICAL CONTRACTOR SHALL FURNISH AND INSTALL.	1
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13	⊙	RECEPTACLE, DUPLEX, 125 VOLTS, 20 AMP, 3 WIRE, CONDUIT SHALL BE RIGID PVC SCHEDULE 40 UNLESS OTHERWISE NOTED. ELECTRICAL CONTRACTOR SHALL FURNISH AND INSTALL.	1
14	⊙	RECEPTACLE, DUPLEX, 125 VOLTS, 20 AMP, 3 WIRE, CONDUIT SHALL BE RIGID PVC SCHEDULE 40 UNLESS OTHERWISE NOTED. ELECTRICAL CONTRACTOR SHALL FURNISH AND INSTALL.	1
15	⊙	RECEPTACLE, DUPLEX, 125 VOLTS, 20 AMP, 3 WIRE, CONDUIT SHALL BE RIGID PVC SCHEDULE 40 UNLESS OTHERWISE NOTED. ELECTRICAL CONTRACTOR SHALL FURNISH AND INSTALL.	1
16	⊙	RECEPTACLE, DUPLEX, 125 VOLTS, 20 AMP, 3 WIRE, CONDUIT SHALL BE RIGID PVC SCHEDULE 40 UNLESS OTHERWISE NOTED. ELECTRICAL CONTRACTOR SHALL FURNISH AND INSTALL.	1
17	⊙	RECEPTACLE, DUPLEX, 125 VOLTS, 20 AMP, 3 WIRE, CONDUIT SHALL BE RIGID PVC SCHEDULE 40 UNLESS OTHERWISE NOTED. ELECTRICAL CONTRACTOR SHALL FURNISH AND INSTALL.	1
18	⊙	RECEPTACLE, DUPLEX, 125 VOLTS, 20 AMP, 3 WIRE, CONDUIT SHALL BE RIGID PVC SCHEDULE 40 UNLESS OTHERWISE NOTED. ELECTRICAL CONTRACTOR SHALL FURNISH AND INSTALL.	1
19	⊙	RECEPTACLE, DUPLEX, 125 VOLTS, 20 AMP, 3 WIRE, CONDUIT SHALL BE RIGID PVC SCHEDULE 40 UNLESS OTHERWISE NOTED. ELECTRICAL CONTRACTOR SHALL FURNISH AND INSTALL.	1
20	⊙	RECEPTACLE, DUPLEX, 125 VOLTS, 20 AMP, 3 WIRE, CONDUIT SHALL BE RIGID PVC SCHEDULE 40 UNLESS OTHERWISE NOTED. ELECTRICAL CONTRACTOR SHALL FURNISH AND INSTALL.	1
21	⊙	RECEPTACLE, DUPLEX, 125 VOLTS, 20 AMP, 3 WIRE, CONDUIT SHALL BE RIGID PVC SCHEDULE 40 UNLESS OTHERWISE NOTED. ELECTRICAL CONTRACTOR SHALL FURNISH AND INSTALL.	1

ELECTRICAL INSTALLATION KEY	
1. "1" SYMBOL INDICATES DETAIL NUMBER/SHEET NUMBER.	
2. "2" SYMBOL INDICATES DETAIL NUMBER/SHEET NUMBER.	
3. "3" SYMBOL INDICATES DETAIL NUMBER/SHEET NUMBER.	
4. "4" SYMBOL INDICATES DETAIL NUMBER/SHEET NUMBER.	
5. "5" SYMBOL INDICATES DETAIL NUMBER/SHEET NUMBER.	
6. "6" SYMBOL INDICATES DETAIL NUMBER/SHEET NUMBER.	
7. "7" SYMBOL INDICATES DETAIL NUMBER/SHEET NUMBER.	
8. "8" SYMBOL INDICATES DETAIL NUMBER/SHEET NUMBER.	
9. "9" SYMBOL INDICATES DETAIL NUMBER/SHEET NUMBER.	
10. "10" SYMBOL INDICATES DETAIL NUMBER/SHEET NUMBER.	
11. "11" SYMBOL INDICATES DETAIL NUMBER/SHEET NUMBER.	
12. "12" SYMBOL INDICATES DETAIL NUMBER/SHEET NUMBER.	
13. "13" SYMBOL INDICATES DETAIL NUMBER/SHEET NUMBER.	
14. "14" SYMBOL INDICATES DETAIL NUMBER/SHEET NUMBER.	
15. "15" SYMBOL INDICATES DETAIL NUMBER/SHEET NUMBER.	
16. "16" SYMBOL INDICATES DETAIL NUMBER/SHEET NUMBER.	
17. "17" SYMBOL INDICATES DETAIL NUMBER/SHEET NUMBER.	
18. "18" SYMBOL INDICATES DETAIL NUMBER/SHEET NUMBER.	
19. "19" SYMBOL INDICATES DETAIL NUMBER/SHEET NUMBER.	
20. "20" SYMBOL INDICATES DETAIL NUMBER/SHEET NUMBER.	
21. "21" SYMBOL INDICATES DETAIL NUMBER/SHEET NUMBER.	

SPECIFICATIONS - ELECTRICAL	
1. PROVIDE ALL MATERIALS AND LABOR AS REQUIRED FOR A COMPLETE INSTALLATION.	
2. PROVIDE UNDERGROUND SERVICE FROM OFFICE TO HOME AT 200 AMP PANEL.	
3. PROVIDE ALL LEADS INCLUDING CABLES, WIRING, WIRING CHANNELS, WIRING TRAYS, WIRING DEVICES, AND WIRING MATERIALS.	
4. PROVIDE ALL LEADS INCLUDING CABLES, WIRING, WIRING CHANNELS, WIRING TRAYS, WIRING DEVICES, AND WIRING MATERIALS.	
5. PROVIDE ALL LEADS INCLUDING CABLES, WIRING, WIRING CHANNELS, WIRING TRAYS, WIRING DEVICES, AND WIRING MATERIALS.	
6. PROVIDE ALL LEADS INCLUDING CABLES, WIRING, WIRING CHANNELS, WIRING TRAYS, WIRING DEVICES, AND WIRING MATERIALS.	
7. PROVIDE ALL LEADS INCLUDING CABLES, WIRING, WIRING CHANNELS, WIRING TRAYS, WIRING DEVICES, AND WIRING MATERIALS.	
8. PROVIDE ALL LEADS INCLUDING CABLES, WIRING, WIRING CHANNELS, WIRING TRAYS, WIRING DEVICES, AND WIRING MATERIALS.	
9. PROVIDE ALL LEADS INCLUDING CABLES, WIRING, WIRING CHANNELS, WIRING TRAYS, WIRING DEVICES, AND WIRING MATERIALS.	
10. PROVIDE ALL LEADS INCLUDING CABLES, WIRING, WIRING CHANNELS, WIRING TRAYS, WIRING DEVICES, AND WIRING MATERIALS.	

LIGHTING FIXTURE SCHEDULE			
SYMBOL	DESCRIPTION	QTY	REMARKS
1	INDOOR	1	100 WATT
2	INDOOR	1	100 WATT
3	INDOOR	1	100 WATT
4	INDOOR	1	100 WATT
5	INDOOR	1	100 WATT
6	INDOOR	1	100 WATT
7	INDOOR	1	100 WATT
8	INDOOR	1	100 WATT
9	INDOOR	1	100 WATT
10	INDOOR	1	100 WATT
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12	INDOOR	1	100 WATT
13	INDOOR	1	100 WATT
14	INDOOR	1	100 WATT
15	INDOOR	1	100 WATT
16	INDOOR	1	100 WATT
17	INDOOR	1	100 WATT
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19	INDOOR	1	100 WATT
20	INDOOR	1	100 WATT
21	INDOOR	1	100 WATT

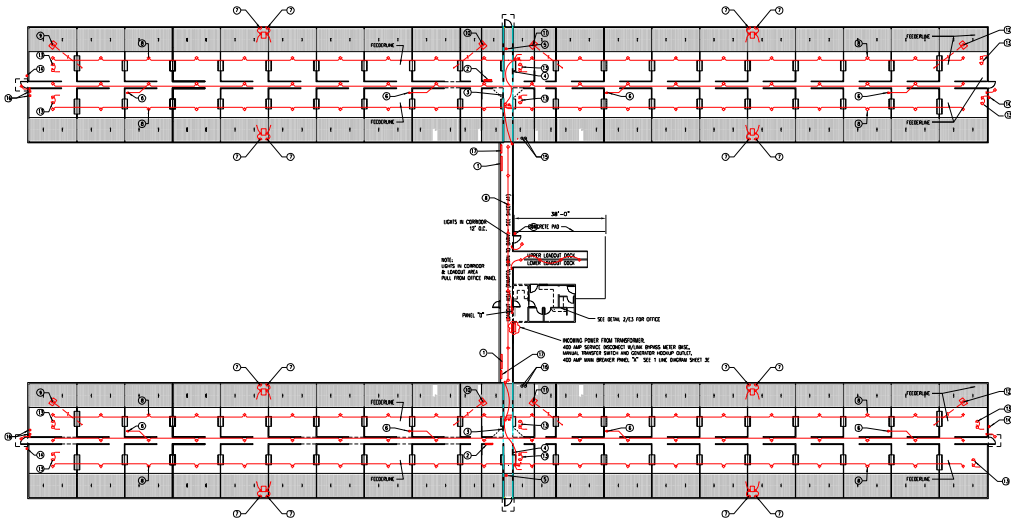
ELECTRICAL INSTALLATION NOTES	
1. CONDUIT SHALL BE RIGID PVC SCHEDULE 40 UNLESS OTHERWISE NOTED.	
2. ALL WIRING SHALL BE IN CONDUIT UNLESS OTHERWISE NOTED.	
3. ALL WIRING SHALL BE IN CONDUIT UNLESS OTHERWISE NOTED.	
4. ALL WIRING SHALL BE IN CONDUIT UNLESS OTHERWISE NOTED.	
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20. ALL WIRING SHALL BE IN CONDUIT UNLESS OTHERWISE NOTED.	
21. ALL WIRING SHALL BE IN CONDUIT UNLESS OTHERWISE NOTED.	

No.	Revisions	Date	By	No.	Revisions	Date	By	Reference Dwg.
1				1				
2				2				
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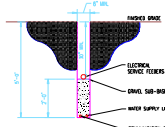
Smithfield Hog Prod.
 4400 HEAD H BARN
 21.01.01.01
 21.01.01.02
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 21.01.01.04
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 21.01.01.17
 21.01.01.18
 21.01.01.19
 21.01.01.20
 21.01.01.21

Project Title :
 4400 HEAD H BARN
 Sheet File :
 SCHEDULES & NOTES

Project Location :
 2651 E1
 Drawn By :
 T. BLACKNER
 Checked By :
 Date Drawn :
 Sheet #
 of



1 ELECTRICAL FLOOR PLAN
SCALE: 1/16" = 1'-0"



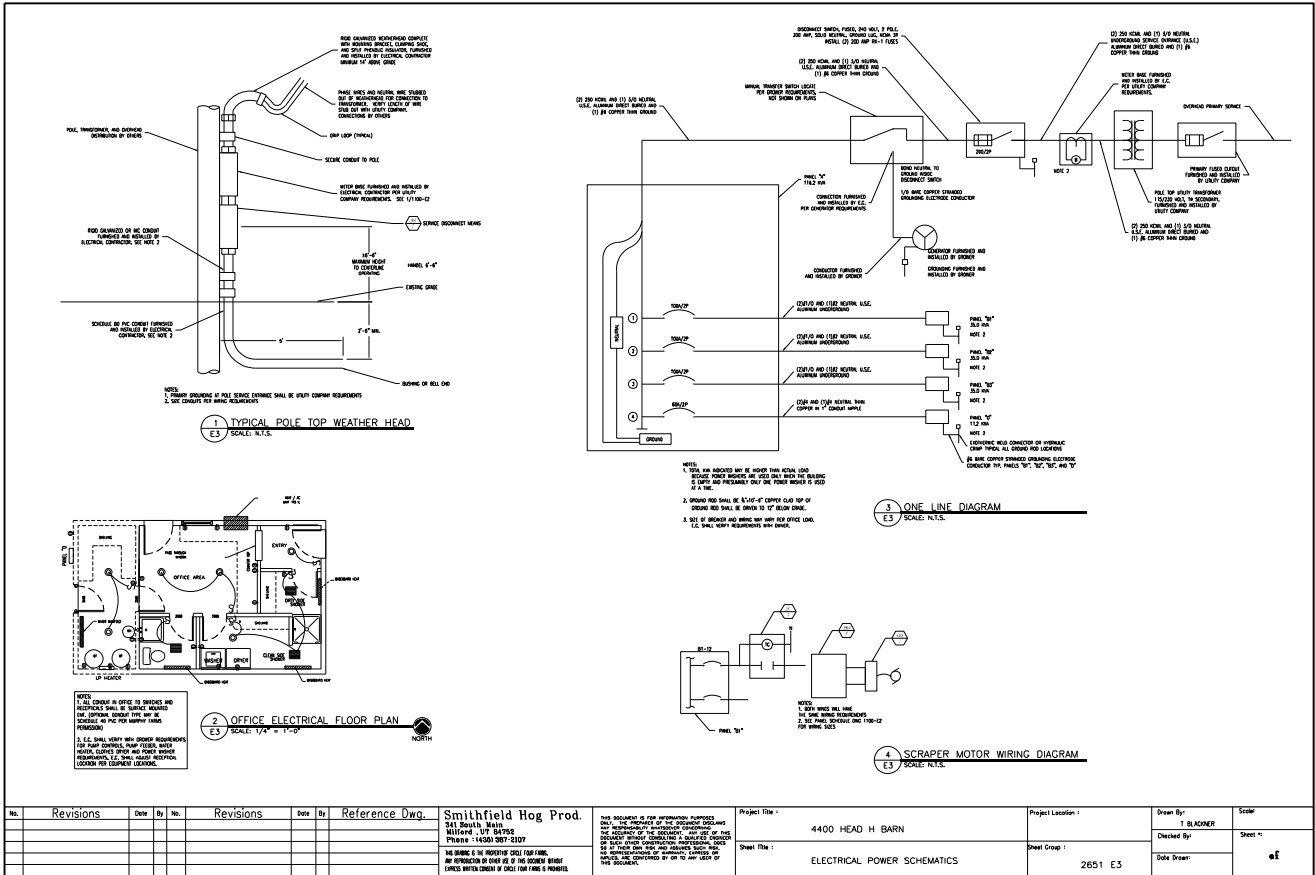
2 UTILITY TRENCH DRAIN (TYPICAL)
SCALE: 1/2" = 1'-0"

- PLAN DETAIL NOTES**
1. USE CONDUIT PANEL # 40" x 40" SEE SHEET 11
 2. USE CONDUIT PANEL
 3. KEEP CLEARANCE FOR CONDUIT PANEL 12" x 18" x 1/2" SEE SHEET 11 FOR MORE INFO. ALL CONDUIT SHALL BE FULL CODE UNDER PLUMBING CODE. THIS MUST BE 6" BELOW
 4. USE CONDUIT PANEL FOR CONDUIT PANEL 12" x 18" x 1/2" SEE SHEET 11 FOR MORE INFO. ALL CONDUIT SHALL BE FULL CODE UNDER PLUMBING CODE. THIS MUST BE 6" BELOW
 5. CONDUIT SHALL BE 1/2" BELOW
 6. CONDUIT SHALL BE 1/2" BELOW
 7. CONDUIT SHALL BE 1/2" BELOW
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 17. CONDUIT SHALL BE 1/2" BELOW
 18. CONDUIT SHALL BE 1/2" BELOW
 19. CONDUIT SHALL BE 1/2" BELOW
 20. CONDUIT SHALL BE 1/2" BELOW

- ELECTRICAL SHEET**
1. ALL SERVICES ARE TO BE LOCATED ALONG THE CENTER LINE OF THE MAIN & BARN TRACES IN FROM THE MAIN TO PANEL. ALL SERVICES SHALL BE FULL CODE UNDER PLUMBING CODE. THIS MUST BE 6" BELOW
 2. SERVICES ARE TO BE MADE BY THE MAIN. SERVICES SHALL BE FULL CODE UNDER PLUMBING CODE. THIS MUST BE 6" BELOW
 3. SERVICES ARE TO BE MADE BY THE MAIN. SERVICES SHALL BE FULL CODE UNDER PLUMBING CODE. THIS MUST BE 6" BELOW
 4. SERVICES ARE TO BE MADE BY THE MAIN. SERVICES SHALL BE FULL CODE UNDER PLUMBING CODE. THIS MUST BE 6" BELOW
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- SITE ELECTRICAL NOTES**
1. NORTH ORIENTATION OF SHEET AND BUILDING NUMBERING MAY VARY.
 2. SEE SHEET 11 FOR MORE INFO.
 3. SEE SHEET 11 FOR MORE INFO.
 4. ALL SERVICES ARE TO BE MADE BY THE MAIN. SERVICES SHALL BE FULL CODE UNDER PLUMBING CODE. THIS MUST BE 6" BELOW
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No.	Revisions	Date	By	No.	Revisions	Date	By	Reference Dwg.	Smithfield Hog Prod.	Project Title	Project Location	Drawn By	Scale
									4400 HEAD H BARN			T BLACKNER	Sheet 11
									ELECTRICAL FLOOR PLAN		E2	DATE DRAWN	6f



No.	Revisions	Date	By	No.	Revisions	Date	By	Reference Dwg.	Smithfield Hog Prod.	Project File	Project Location	Drawn By	Scale
									841 South Main Millsboro, DE 19966 Phone: (430) 387-3107	4400 HEAD H BARN	Sheet Group 1	2651 E3	2
									THIS DRAWING IS THE PROPERTY OF SMITHFIELD HOG PRODUCTS. IT IS TO BE USED ONLY FOR THE PROJECT AND LOCATION SPECIFICALLY IDENTIFIED HEREON. ANY REPRODUCTION OR USE AS OF THIS DRAWING WITHOUT WRITTEN PERMISSION OF SMITHFIELD HOG PRODUCTS IS STRICTLY PROHIBITED.	ELECTRICAL POWER SCHEMATICS			
												Checked By	Sheet #
												Scale Drawn	of

1 & Name: Panel 1000 Head H Barn E4

Expansion Box #

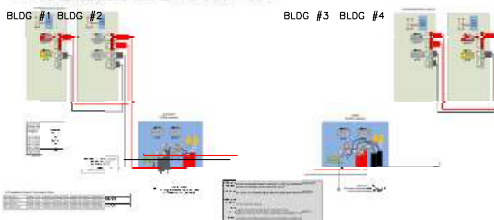
Barn 2

Note: Cards in box will likely need to be moved to match this layout

Slot #1	Slot #2	Slot #3	Slot #4	Slot #5	Slot #6
[Input: R relay Prox/ATC Output]	[Input: S relay Prox/ATC Output]	[Input: M relay Prox/ATC Output]	[Input: B relay Prox/ATC Output]	[Input: [Blank]	[Input: [Blank]

Slot #1	Slot #2	Slot #3	Slot #4	Slot #5	Slot #6
IN 1 Room Probe 3 (RM)	IN 2 Room Probe 2 (RM)	IN 3 Room Probe 3 (NE)	IN 4 Room Probe 4 (SE)	IN 5 Room Probe 1 (SE)	
IN 1 Room Probe 4 (RM)	IN 2 Room Probe 5 (DM)	IN 3 Room Probe 6 (SE)	IN 4 Room Probe 7 (SE)	IN 5 Room Probe 8 (SE)	
IN 1 Outside Probe	IN 2	IN 3	IN 4	IN 5	
OUT 1	OUT 2	OUT 3	OUT 4	OUT 5	
OUT 1	OUT 2	OUT 3	OUT 4	OUT 5	
OUT 1	OUT 2	OUT 3	OUT 4	OUT 5	
OUT 1	OUT 2	OUT 3	OUT 4	OUT 5	
OUT 1	OUT 2	OUT 3	OUT 4	OUT 5	
OUT 1	OUT 2	OUT 3	OUT 4	OUT 5	
OUT 1	OUT 2	OUT 3	OUT 4	OUT 5	
OUT 1	OUT 2	OUT 3	OUT 4	OUT 5	
OUT 1	OUT 2	OUT 3	OUT 4	OUT 5	
OUT 1	OUT 2	OUT 3	OUT 4	OUT 5	
OUT 1	OUT 2	OUT 3	OUT 4	OUT 5	
OUT 1	OUT 2	OUT 3	OUT 4	OUT 5	

1 EXPANSION BOX SCH. (TYP. EACH BARN)
SCALE: N.T.S.



3 RS 42 CABLE WIRE DIAGRAM
SCALE: N.T.S.

PANEL "B1" TYP.		120/208 VAC 3PH 4W.										225A	
CKT NO.	LOAD V.A. TYPE	LOAD DESCRIPTION	WIRE SIZE	P	PH	AMP SIZE	P	SIZE	LOAD DESCRIPTION	LOAD TYPE	LOAD V.A.	CKT NO.	
3	450 LVS	FEED MOTORS	12	1	20	A	25	1	12	LIGHTS ROOM #2	LVS	450	4
5	500	FEED MOTORS	12	1	15	C	-	-	-	SPARE	-	-	6
7	850 MTR	SOUTH LINE	12	2	15	B	15	2	12	2-24" FANS	MTR	850	8
9	850	FEED MOTORS	-	-	-	B	-	-	-	ROOM #2	-	850	10
11	850 MTR	INCOMING	12	2	15	C	15	2	12	ROOM #2	MTR	700	12
12	850	SW & SW CURTAIN MACHINE	-	-	-	A	-	-	-	ROOM #2	-	700	14
15	700 MTR	2-18" FANS	12	2	15	B	15	2	12	2-18" FANS	MTR	700	16
17	700	ROOM #1	-	-	-	C	-	-	-	ROOM #1 & #2	-	700	18
19	1000 MTR	HEATER ROOM #1	12	1	20	A	20	1	12	HEATER ROOM #2	MTR	1000	20
21	1000 MTR	PRE-HEAT HEATERS #1	12	1	15	B	20	1	12	HALLWAY CONV. RECEPT	MTR	1000	22
23	1000 MTR	PRE-HEAT HEATER #2	12	1	15	C	20	1	12	PRE-HEAT LIGHTS	LVS	1000	24
25	-	SPARE	-	-	-	A	20	1	12	SPARE	-	-	26
27	1100 LVS	HEAT LAMPS RM1-#1	12	1	20	B	20	1	12	HEAT LAMPS RM1-#1	LVS	1100	28
29	1100 LVS	HEAT LAMPS RM1-#2	12	1	20	C	20	1	12	HEAT LAMPS RM1-#2	LVS	1100	30
31	1100 LVS	HEAT LAMPS RM1-#3	12	1	20	A	20	1	12	HEAT LAMPS RM1-#3	LVS	1100	32
33	1100 LVS	HEAT LAMPS RM1-#4	12	1	20	B	20	1	12	HEAT LAMPS RM1-#4	LVS	1100	34
35	1100 LVS	HEAT LAMPS RM1-#5	12	1	20	C	20	1	12	HEAT LAMPS RM1-#5	LVS	1100	36
37	1100 LVS	HEAT LAMPS RM1-#6	12	1	20	A	20	1	12	HEAT LAMPS RM1-#6	LVS	1100	38
39	-	SPARE	-	-	-	B	20	1	-	SPARE	-	-	40
41	-	SPARE	-	-	-	C	20	1	-	SPARE	-	-	42

2 ELEC. BREAKER PANEL (TYP.)
SCALE: N.T.S.

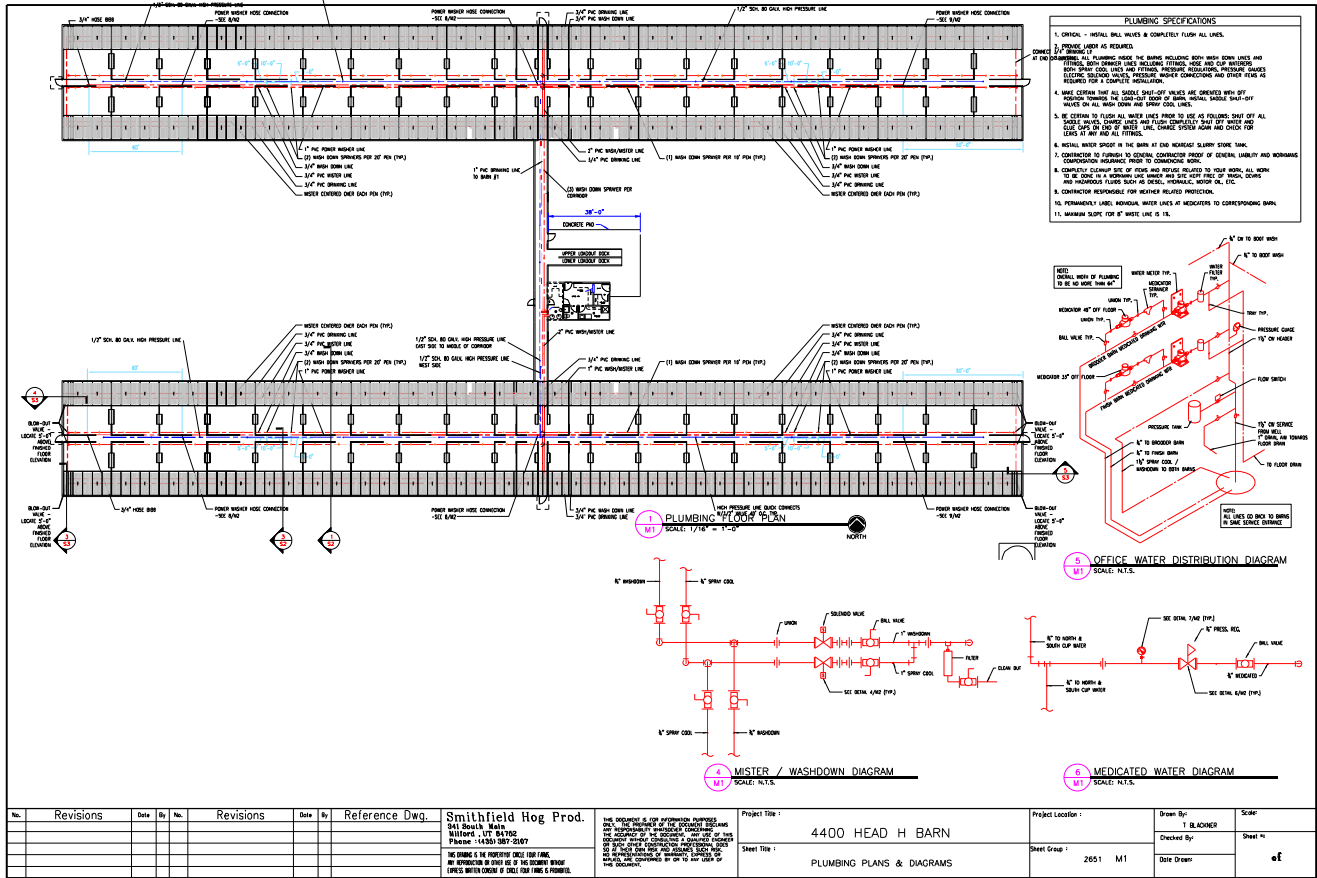
No.	Revisions	Date	By	No.	Revisions	Date	By	Reference Dwg.

Smithfield Hog Prod.
341 South Main
Bluffton, VT 05430
Phone (449) 587-6187

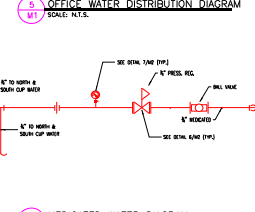
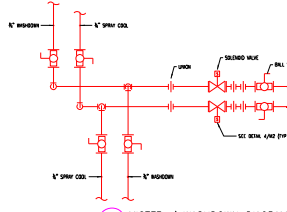
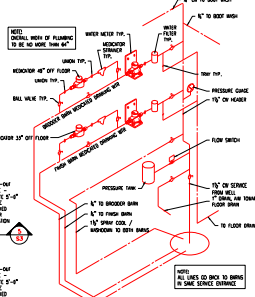
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NO DRIVING IS PERMITTED UNTIL THE ROAD IS OPEN TO TRAFFIC. PLEASE CHECK WITH THE STATE FOR THE LATEST INFORMATION.

Project No:	4400 HEAD H BARN	Project Location:		Drawn By:	J. BLANCHER
Sheet No.:	PANEL SCH & RELAY ASSIGN.	Sheet Count:	26 of 42	Checked By:	
		Date Drawn:			



- PLUMBING SPECIFICATIONS**
1. CRITICAL - METAL BALL VALVES & COMPLETELY FLUSH ALL LINES.
 2. REMOVE AIRS AS REQUIRED.
 3. VERIFY ALL PLUMBING WORK IS BEING INSTALLED WITH CORRECT JOINTS AND FITTINGS. ALL PLUMBING WORK SHALL BE INSTALLED IN ACCORDANCE WITH THE LATEST EDITIONS OF THE PLUMBING CODE, AS APPLICABLE. ALL PLUMBING SHALL BE INSTALLED IN ACCORDANCE WITH THE LATEST EDITIONS OF THE PLUMBING CODE, AS APPLICABLE. ALL PLUMBING SHALL BE INSTALLED IN ACCORDANCE WITH THE LATEST EDITIONS OF THE PLUMBING CODE, AS APPLICABLE.
 4. MAKE CERTAIN THAT ALL SCHEDULE 40S-STD VALVES ARE OPERATED WITH OFF-POSITION TOWARD THE END-UP SIDE OF BALL. METAL BALL VALVE SHUT-OFF SHALL BE TO THE END-UP SIDE OF BALL. CHECK SYSTEM AGAIN AND CHECK FOR LEAKS AT ALL JOINTS AND FITTINGS.
 5. BE CERTAIN TO FLUSH ALL WATER LINES PRIOR TO USE AS FOLLOWS: SHUT OFF ALL WATER SUPPLY, OPEN LINES AND FLUSH COMPLETELY SHUT OFF WATER AND LEAK TEST FOR 24 HOURS. LINE CHECK SYSTEM AGAIN AND CHECK FOR LEAKS AT ALL JOINTS AND FITTINGS.
 6. METAL WELDS SHALL BE IN THE BURN AT END WELDED SLOTTED STEEL LINES.
 7. CONTRACTOR TO PROVIDE TO COVER CONNECTIONS PROOF OF CORROSION INHIBITOR AND WORKING COMPENSATION INSURANCE PRIOR TO COMMENCING WORK.
 8. CONTRACTOR TO PROVIDE TO COVER CONNECTIONS PROOF OF CORROSION INHIBITOR AND WORKING COMPENSATION INSURANCE PRIOR TO COMMENCING WORK.
 9. TO BE DONE IN A SEPARATE LINE UNDER THE LINE THAT IS NOT TO BE DONE. ALL WORK AND MATERIALS FLUENTS SUCH AS DIESEL, HYDRAULIC MOTOR OIL, ETC.
 10. CONTRACTOR RESPONSIBLE FOR ALL OTHER RELATED PROVISIONS.
 11. PERMANENTLY LABEL INDIVIDUAL WATER LINES AS INDICATED TO CORRESPONDING BARN.
 12. MAINTAIN SLOPE FOR 1/8" WHITE LINE & 1/8".



No.	Revisions	Date	By	No.	Revisions	Date	By	Reference Dwg.

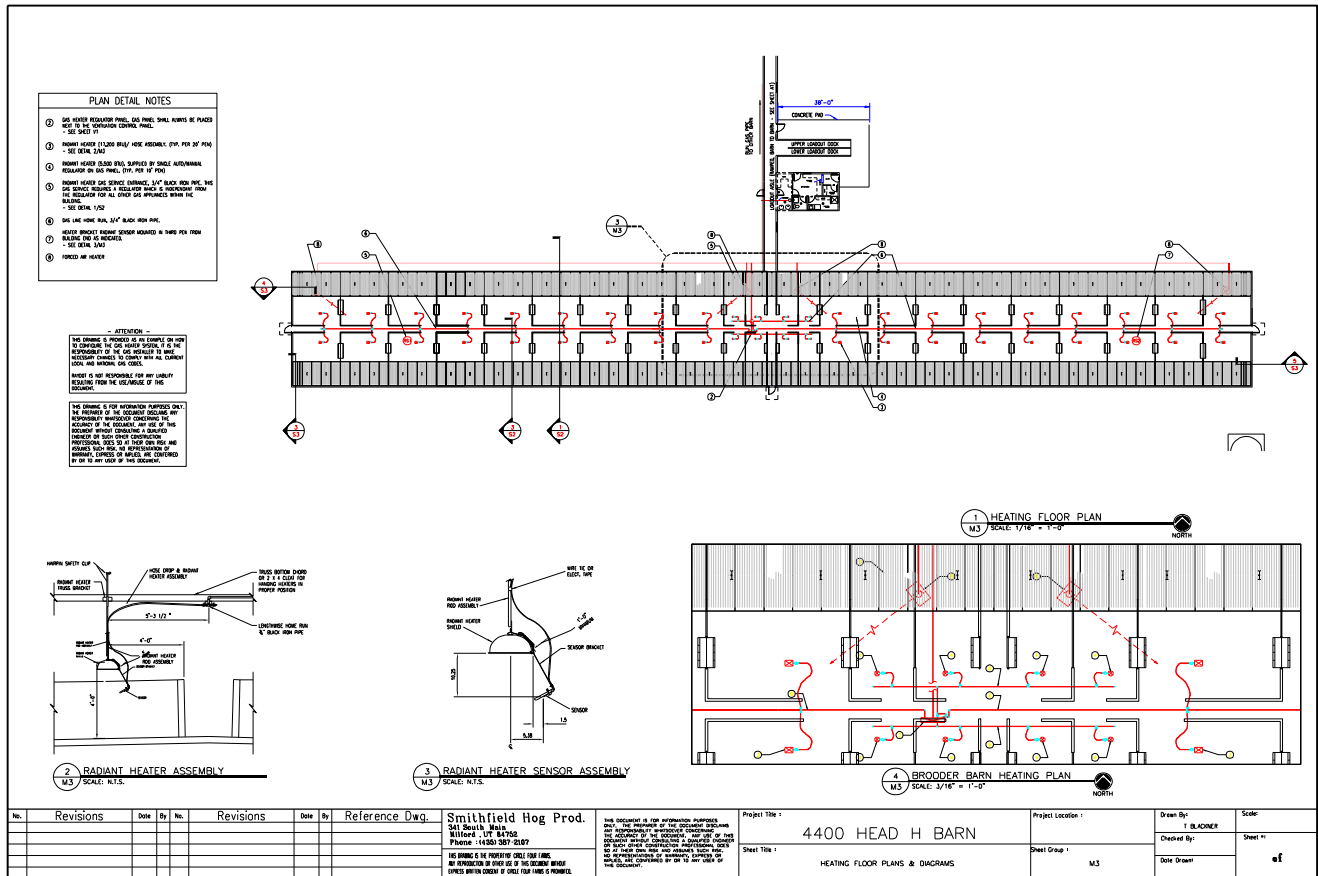
Smithfield Hog Prod. 341 South Main Williston, VT 05495 PHONE: 1-800-557-2207 <small>THE DRAWING IS THE PROPERTY OF THE DRAWER. NO REPRODUCTION OR USE OF THE DRAWING WITHOUT THE WRITTEN CONSENT OF THE DRAWER IS PERMITTED.</small>	Project Title: 4400 HEAD H BARN Sheet Title: PLUMBING PLANS & DIAGRAMS	Project Location: Sheet Group: 2651 M1	Drawn By: T. BLADNER Checked By: Date Drawn: Scale: as
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Signature: *Woodrow Campbell*
 Woodrow Campbell (Jun 29, 2021 09:07 MDT)

Email: wwcampbell@utah.gov

Signature: *Erica B. Addis*

Email: egaddis@utah.gov



No.	Revisions	Date	By	No.	Revisions	Date	By	Reference Dwg.

Smithfield Hog Prod. 5415 South 4000 Biltmore - UT 84702 Phone: 435-527-0227 HIS DRAWING IS THE PROPERTY OF DWQ CONSULTANTS. ANY REVISIONS OR CHANGES TO THIS DRAWING SHALL BE APPROVED BY DWQ CONSULTANTS. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO OBTAIN ALL NECESSARY PERMITS AND APPROVALS FROM THE LOCAL AUTHORITY.	Project Title : 4400 HEAD H BARN	Project Location :	Drawn By : T. BLACKNER	Scale :
SHEET TITLE : HEATING FLOOR PLANS & DIAGRAMS	SHEET GROUP : M3	DATE DRAWN :	CHECKED BY :	SHEET NO. : of

**APPENDIX B
PERMIT MODIFICATION APPLICATION
AND SUPPORTING DOCUMENTS**

Smithfield Hog Production

Groundwater Discharge Report and Application Modification of Existing Permit For Dalton Finisher Farm Sites Addition Garfield County, Utah

Prepared by
Joel A. Myers, P.E.
GEM Engineering, Inc.

January 7, 2021

Report Number: RE0958

January 7, 2021

Division of Water Quality

Utah Department of Environmental Quality

Salt Lake City, Utah 84114-4870

Subject: Groundwater Discharge Permit Application and Report
Modification of Existing Permit
For Smithfield Hog Production
Dalton Finisher Farm Addition
Garfield County, Utah

Enclosed are the application, required backup information and reports for the submission of the Utah Groundwater Discharge Permit Application for the project listed above. The project is to be located approximately 2.6 miles south of Circleville, in Garfield County, Utah.

We appreciate this opportunity to be of service on this phase of the project and look forward to being of service as the project progresses. If you have any questions, please contact this office at your convenience.

Sincerely,
GEM Engineering, Inc.



Joel A. Myers, P.E.
President

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ATTACHMENTS

Attachment A	Composite Location, & Topographic Map
Attachment B	Well Logs and Water Quality Report
Attachment C	Project Documents
Attachment D	Specifications and QA / QC for HDPE Liners
Attachment E	State of Utah Geological Survey Paper #1836 entitled "Ground-Water Conditions and Geologic Reconnaissance of the Upper Sevier River Basin, Utah"
Attachment F	Water Quality Handling & Analysis Plan

SECTION 1: INTRODUCTION

1.1 Nature of Application

Dalton Finisher Site Addition will be four additional 1100 hog finisher barns at the existing 8800 hog spaces for a site total of 13,200 hog spaces with a primary existing basin and an additional basin to contain the additional wastewater. The wastewater from the both the original lagoon and a newly constructed lagoon will be applied to the nearby fields annually.

1.2 Manure Handling System

The swine will be confined inside environmentally controlled buildings. The floors supporting the swine will consist of concrete slats (reinforced concrete slats spaced approximately 1.25" apart). Manure will be worked through the slats and temporarily stored in shallow concrete pits below. The pit floors and exterior walls will be constructed according to specifications and drawings, submitted in Attachment D, to assure wastewater is retained. The manure will be emptied approximately once a day into a temporary storage basin. No recycle water will be utilized. The barns will utilize a scraper plate manure collection system. The manure collected in the basin will be land applied at the appropriate time of the year for growing crops.

1.3 Topography and Soils

The topography surrounding the facility slopes roughly 2.5% down towards the Northwest (see **Attachment A**). The soil types in the area surrounding the facility site are typical alluvial materials consisting primarily of silt, sand, and gravel. The surface soil types at the proposed facility location are typically organic silt and silty sands.

The groundwater table is located roughly 43 to 72 feet below existing grade based on information from the closest well logs. The groundwater will be protected by certified Flexible Membrane Liners (FML), inspection procedures and monitoring wells.

1.4 Climate

Table 1-1 shows weather data collected near Circleville, Utah area roughly 2.6 miles north of the facility location.

The climate in the area is typically warm and dry in the summer and cold and dry in the winter.

Table 1-1 Weather Data For Circleville, Utah

	Jan	Feb	Mar	Apr	May	Jun
Average high in °F:	42	47	53	61	70	81
Average low in °F:	13	18	23	28	37	44
Av. precipitation in inch:	0.59	0.55	0.75	0.63	0.91	0.55
Average snowfall in inch:	5	2.9	3.6	1.0	0.2	0.0
	Jul	Aug	Sep	Oct	Nov	Dec
Average high in °F:	87	85	77	66	52	44
Average low in °F:	51	50	41	30	20	13
Av. precipitation in inch:	0.87	1.42	0.94	0.91	0.51	0.43
Average snowfall in inch:	0.0	0.0	0.1	0.6	1.9	3.8

Climate data for Circleville, UT Longitude: -112.27, Latitude: 38.17
 Average weather Circleville, UT - 84723 - 1961-1990 normals

Circleville, Utah weather averages

Annual average high temperature:	63.8°F
Annual average low temperature:	30.7°F
Average temperature:	47.25°F
Average annual precipitation - rainfall:	9.06 inches
Av. annual snowfall:	19.2 inches

1.5 Groundwater

The Utah Groundwater Discharge Permit Application was obtained from the Utah Department of Environmental Quality – Division of Water Quality web site and is incorporated into this report on the following 9 pages.

MAIL TO:

Division of Water Quality
Utah Department of Environmental Quality
Salt Lake City, Utah 84114-4870

Application No.: _____
Date Received: _____
(leave both lines blank)

UTAH GROUNDWATER DISCHARGE PERMIT APPLICATION

Part A - General Facility Information

Please read and follow carefully the instructions on this application form. Please type or print, except for signatures. This application is to be submitted by the owner or operator of a facility having one or more discharges to groundwater. The application must be signed by an official facility representative who is: the owner, sole proprietor for a sole proprietorship, a general partner, an executive officer of at least the level of vice president for a corporation, or an authorized representative of such executive officer having overall responsibility for the operation of the facility.

- 1. Administrative Information.** Enter the information requested in the space provided below, including the name, title and telephone number of an agent at the facility who can answer questions regarding this application.

Facility Name: Dalton Finisher Sites

Mail Address: Dalton Hay Company, LLC, P.O. Box 189, Circleville, UT 84723
(Number & Street, Box and/or Route, City, State, Zip Code)

Facility Legal Location* County: Garfield

T. 31S, R. 4W, Sec. 11, 12, North 1/2,

Site # 1 Lat. 38 ° 8 ' 2.86 "N. Long. 112 ° 15 ' 58.84 "W

*Note: A topographic map or detailed aerial photograph should be used in conjunction with a written description to depict the location of the facility, points of groundwater discharge, and other relevant features/objects. (See Attachment B)

Contact's Name: Jade Dalton Phone No.: (435) 616-3081

Title: Owner

- 2. Owner/Operator Information.** Enter the information requested below, including the name, title, and phone number of the official representative signing the application.

Owner

Name: Jade Dalton Phone No.: (435) 616-3081

Mail Address: Dalton Hay Company, LLC, P.O. Box 189, Circleville, UT 84723
(Number & Street, Box and/or Route, City, State, Zip Code)

Operator

Name: Same Phone No.: ()
(If different than Owner's above)

Mail Address: _____
(Number & Street, Box and/or Route, City, State, Zip Code)

Official Representative

Name: Same Phone No. (435) 577-2861

Title: Owner

- 3. Facility Classification** (check one)

- New Facility
- Existing Facility
- Modification of Existing Facility

MAIL TO:

Division of Water Quality
Utah Department of Environmental Quality
Salt Lake City, Utah 84114-4870

Application No.: _____
Date Received: _____
(leave both lines blank)

4. Type of Facility (check one)

- Industrial
- Mining
- Municipal
- Agricultural Operation
- Other, please describe: _____

5. SIC/NAICS Codes: NAICS-112210 – Hog Farms and Hog Production
Enter Principal 3 Digit Code Numbers Used in Census & Other Government Reports

6. Projected Facility Life: 20+ years

7. Identify principal processes used, or services preformed by the facility. Include the principal products produced, and raw materials used by the facility:

This facility will be utilized for hog production. Hogs will be raised to maturity and then transported to other off-site facilities by truck for processing

8. List all existing or pending Federal, State, and Local government environmental permits:

- | | |
|--|---------------------------|
| | <u>Permit Number</u> |
| <input checked="" type="checkbox"/> NPDES or UPDES (discharges to surface water) | UGW170004 _____] |
| CAFO (concentrated animal feeding operation) | _____ |
| <input type="checkbox"/> UIC (underground injection of fluids) | _____ |
| <input type="checkbox"/> RCRA (hazardous waste) | _____ |
| <input type="checkbox"/> PDS (air emissions from proposed sources) | _____ |
| <input type="checkbox"/> Construction Permit (wastewater treatment) | _____ |
| <input type="checkbox"/> Solid Waste Permit (sanitary landfills, incinerators) | _____ |
| <input checked="" type="checkbox"/> Septic Tank/Drainfield | <u>TBD by Health Dept</u> |
| <input type="checkbox"/> Other, specify _____ | _____ |

9. Name, location (Lat. _____ ° _____ ' _____ "N, Long. _____ ° _____ ' _____ "W) and description of: each well/spring (existing, abandoned, or proposed), water usage(past, present, or future); water bodies; drainages; well-head protection areas; drinking water source protection zones according to UAC 309-600; topography; and man-made structures within one mile radius of the point(s) of discharge site. Provide existing well logs (include total depth and variations in water depths).

<u>Name</u>	<u>Location</u>	<u>Description</u>	<u>Status</u>	<u>Usage</u>
-------------	-----------------	--------------------	---------------	--------------

See report and location maps included with this application

The above information must be included on a plat map and attached to the application.

Part B - General Discharge Information

Complete the following information for each point of discharge to groundwater. If more than one discharge point exists, photocopy and complete this Part B form for each discharge point.

1. **Location** (if different than Facility Location in Part A): County: Same as in Part A all sites
 T. _____, R. _____, Sec. _____, _____ 1/4 of _____ 1/4,
 Lat. _____ ° _____ ' _____ "N. Long. _____ ° _____ ' _____ "W

2. **Type of fluid to be Discharged or Potentially Discharged**
 (check as applicable)

Discharges (fluids discharged to the ground)

- Sanitary Wastewater: wastewater from restrooms, toilets, showers and the like
- Cooling Water: non-contact cooling water, non contact of raw materials, intermediate, final, or waste products
- Process Wastewater: wastewater used in or generated by an industrial process
- Mine Water: water from dewatering operations at mines
- Other, specify: Hog Production Waste Water

Potential Discharges (leachates or other fluids that may discharge to the ground)

- Solid Waste Leachates: leachates from solid waste impoundments or landfills
- Milling/Mining Leachates: tailings impoundments, mine leaching operations, etc.
- Storage Pile Leachates: leachates from storage piles of raw materials, product, or wastes
- Potential Underground Tank Leakage: tanks not regulated by UST or RCRA only
- Other, specify: None

3. **Discharge Volumes**

For each type of discharge checked in #2 above, list the volumes of wastewater discharged to the ground or groundwater. Volumes of wastewater should be measured or calculated from water usage. If it is necessary to estimate volumes, enclose the number in parentheses. Average daily volume means the average per operating day: ex. For a discharge of 1,000,000 gallons per year from a facility operating 200 days, the average daily volume is 5,000 gallons.

Discharge Type:	Daily Discharge Volume		all in units of
	(Average)	(Maximum)	
<u>None</u>	<u>0</u>	<u>0</u>	_____
_____	_____	_____	_____

4. **Potential Discharge Volumes**

For each type of potential discharge checked in #2 above, list the maximum volume of fluid that could be discharged to the ground considering such factors as: liner hydraulic conductivity and operating head conditions, leak detection system sensitivity, leachate collection system efficiency, etc. Attach calculation and raw data used to determine said potential discharge.

Discharge Type:	Daily Discharge Volume		all in units of
	(Average)	(Maximum)	
<u>Leakage</u>	<u>0</u>	<u>0</u>	_____
_____	_____	_____	_____

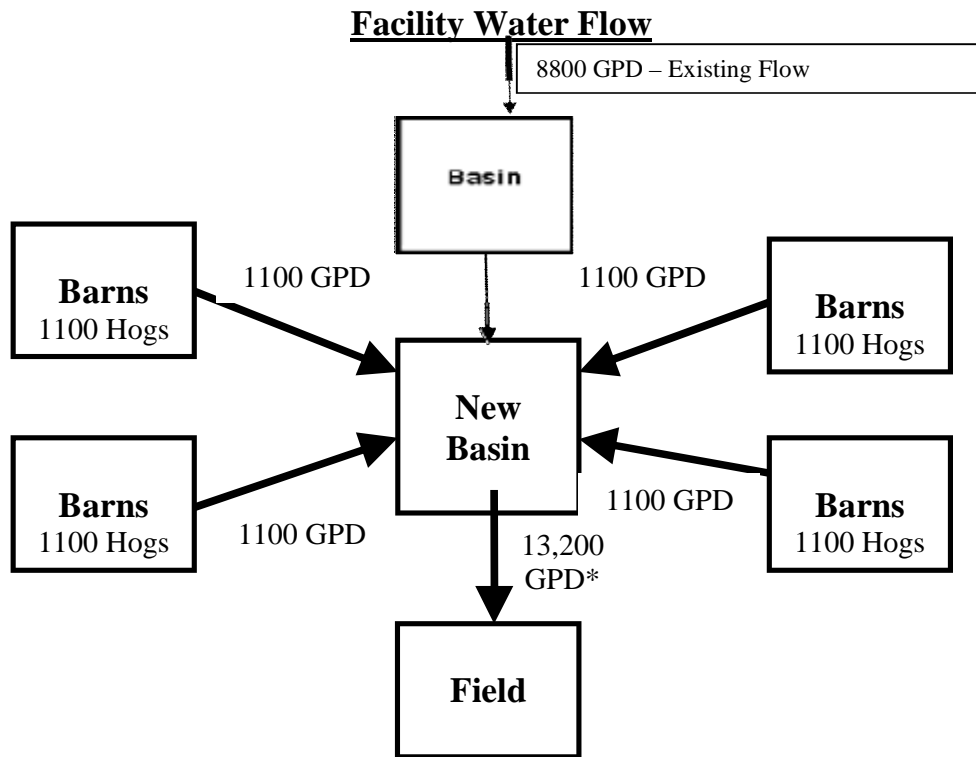
5. Means of discharge or Potential Discharge (check one or more as applicable)

- | | | | |
|-------------------------------------|--|--------------------------|--------------------------------|
| <input checked="" type="checkbox"/> | lagoon, pit, or surface impoundment (fluids) | <input type="checkbox"/> | industrial drainfield |
| <input checked="" type="checkbox"/> | land application or land treatment | <input type="checkbox"/> | underground storage tank |
| <input type="checkbox"/> | discharge to an ephemeral drainage
(dry wash, etc.) | <input type="checkbox"/> | percolation/infiltration basin |
| <input type="checkbox"/> | storage pile | <input type="checkbox"/> | mine heap or dump leach |
| <input type="checkbox"/> | landfill (industrial or solid wastes) | <input type="checkbox"/> | mine tailings pond |
| <input type="checkbox"/> | other, specify _____ | | |

6. Flows, Sources of Pollution, and Treatment Technologies

Flows. Attach a line drawing showing: 1) water flow through the facility to the groundwater discharge point, and 2) sources of fluids, wastes, or solids which accumulate at the potential groundwater discharge point. Indicate sources of intake materials or water, operations contributing wastes or wastewater to the effluent, and wastewater treatment units. Construct a water balance on the line drawing by showing average flows between intakes, operations, treatment units, and wastewater outfalls. If a water balance cannot be determined, provide a pictorial description of the nature and amount of any sources of water and any collection or treatment measures. See the following example.

This chart needs to be updated to reflect the existing facilities and the new ones. Total spaces is 13,200



* Flow from Basins to Field will be on an as needed basis with an average Total flow of 13,200 GPD* - 4400 GPD Average New Flow

7. Discharge Effluent Characteristics

Established and Proposed Groundwater Quality Standards - Identify wastewater or leachate characteristics by providing the type, source, chemical, physical, radiological, and toxic characteristics of wastewater or leachate to be discharged or potentially discharged to groundwater (with lab analytical data if possible). This should include the discharge rate or combination of discharges, and the expected concentrations of any pollutant (mg/l). If more than one discharge point is used, information for each point must be provided.

Hazardous Substances - Review the present hazardous substances found in the Clean Water Act, if applicable. List those substances found or believed present in the discharge or potential discharge.

Part C - Accompanying Reports and Plans

The following reports and plans should be prepared by or under the direction of a professional engineer or other groundwater professional. Since groundwater permits cover a large variety of discharge activities, the appropriate details and requirements of the following reports and plans will be covered in the pre-design meeting(s). For further instruction refer to the Groundwater Permit Application Guidance Document.

8. Hydrogeologic Report

Provide a Geologic Description, with references used, that includes as appropriate:

Structural Geology – regional and local, particularly faults, fractures, joints and bedding plane joints; **Stratigraphy** – geologic formations and thickness, soil types and thickness, depth to bedrock; **Topography** – provide a USGS MAP (7 ½ minute series) which clearly identifies legal site location boundaries, indicated 100 year flood plain area and applicable flood control or drainage barriers and surrounding land uses.

Provide a Hydrologic Description, with references used, that includes:

Groundwater – depths, flow directions and gradients. Well logs should be included if available. Include name of aquifer, saturated thickness, flow directions, porosity, hydraulic conductivity, and other flow characteristics, hydraulic connection with other aquifers or surface sources, recharge information, water in storage, usage, and the projected aerial extent of the aquifer. Should include projected groundwater area of influence affected by the discharge. Provide hydraulic gradient map indicating equal potential head contours and groundwater flow lines. Obtain water elevations of nearby wells at the time of the hydrologic investigation. Collect and analyze groundwater samples from the uppermost aquifer which underlies the discharge point(s). Historic data can be used if the applicant can demonstrate it meets the requirements contained within this section. Collection points should be hydraulically up and downgradient and within a one-mile radius of the discharge point(s). Groundwater analysis should include each element listed in Groundwater Discharge Permit Application, Part B7.

NOTE Failure to analyze for background concentrations of any contaminant of concern in the discharge or potential discharge may result in the Executive Secretary's presumptive determination that zero concentration exist in the background groundwater quality.

Sample Collection and Analysis Quality assurance – sample collection and Preservation must meet the requirements of the EPA RCRA Technical Enforcement Guidance Document, OSWER-9959.1, 1986 [UAC R317-6-6.3(I,6)]. Sample analysis must be performed by State of Utah certified laboratories and be certified for each of the parameters of concern. Analytical methods should be selected from the following sources [UAC R317-6-6.3L]: Standard Methods for the Examination of Water and Wastewater, 20th Ed.,1998; EPA, Methods for Chemical Analysis of Water and

Wastes, 1983; Techniques of Water Resources Investigation of the U.S. Geological Survey, 1998, Book 9; EPA Methods published pursuant to 40 CFR Parts 141, 142, 264 (including Appendix IX), and 270. Analytical methods selected should also include minimum detection limits below both the Groundwater Quality Standards and the anticipated groundwater protection levels. Data shall be presented in accordance of accepted hydrogeologic standards and practice.

Provide Agricultural Description, with references used, that includes:

If agricultural crops are grown within legal boundaries of the site the discussion must include: types of crops produced; soil types present; irrigation system; location of livestock confinement areas (existing or abandoned).

Note on Protection Levels:

After the applicant has defined the quality of the fluid to be discharged (Groundwater Discharge Permit Application, Part B), characterized by the local hydrogeologic conditions and determined background groundwater quality (Hydrogeologic Report), the Executive Secretary will determine the applicable groundwater class, based on: 1) the location of the discharge point within an area of formally classified groundwater, or the background value of total dissolved solids. Accordingly, the Executive Secretary will determine applicable protection levels for each pollutant of concern, based on background concentrations and in accordance with UAC R317-6-4.

9. Groundwater Discharge Control Plan:

Select a compliance monitoring method and demonstrate an adequate discharge control system. Listed are some of the Discharge Control Options available.

No Discharge – prevent any discharge of fluids to the groundwater by lining the discharge point with multiple synthetic and clay liners. Such a system would be designed, constructed, and operated to prevent any release of fluids during both the active life and any post-closure period required.

Earthen Liner – control the volume and rate of effluent seepage by lining the discharge point with a low permeability earthen liner (e.g. clay). Then demonstrate that the receiving groundwater, at a point as close as practical to the discharge point, does not or will not exceed the applicable class TDS limits and protection levels* set by the Executive Secretary. This demonstration should also be based on numerical or analytical saturated or unsaturated groundwater flow and contaminant transport simulations.

Effluent Pretreatment – demonstrate that the quality of the raw or treated effluent at the point of discharge or potential discharge does not or will not exceed the applicable groundwater class TDS limits and protection levels* set by the Executive Secretary.

Contaminant Transport/Attenuation – demonstrate that due to subsurface contaminant transport mechanisms at the site, raw or treated effluent does not or will not cause the receiving groundwater, at a point as close as possible to the discharge point, to exceed the applicable class TDS limits and protection levels* set by the Executive Secretary.

Other Methods – demonstrate by some other method, acceptable to the Executive Secretary, that the groundwater class TDS limits and protection levels* will be met by the receiving groundwater at a point as close as practical to the discharge point.

*If the applicant has or will apply for an alternate concentration limit (ACL), the ACL may apply instead of the class

TDS limits and protection levels.

Submit a complete set of engineering plans and specifications relating to the construction, modification, and operation of the discharge point or system. Construction Permits for the following types of facilities will satisfy these requirements. They include: municipal waste Containment Basins; municipal sludge storage and on-site sludge disposal; land application of wastewater effluent; heap leach facilities; other process wastewater treatment equipment or systems.

Facilities such as storage piles, surface impoundments and landfills must submit engineering plans and specifications for the initial construction or any modification of the facility. This will include the design data and description of the leachate detection, collection and removal system design and construction. Provide provisions for run on and run-off control.

10. Compliance Monitoring Plan:

The applicant should demonstrate that the method of compliance monitoring selected meets the following requirements:

Groundwater Monitoring – that the monitoring wells, springs, drains, etc., meet all of the following criteria: is completed exclusively in the same uppermost aquifer that underlies the discharge point(s) and is intercepted by the upgradient background monitoring well; is located hydrologically downgradient of the discharge point(s); designed, constructed, and operated for optimal detection (this will require a hydrogeologic characterization of the area circumscribed by the background sampling point, discharge point and compliance monitoring points); is not located within the radius of influence of any beneficial use public or private water supply; sampling parameters, collection, preservation, and analysis should be the same as background sampling point; groundwater flow direction and gradient, background quality at the site, and the quality of the groundwater at the compliance monitoring point.

Source Monitoring – must provide early warning of a potential violation of groundwater protection levels, and/or class TDS limits and be as or more reliable, effective, and determinate than a viable groundwater monitoring network.

Vadose Zone Monitoring Requirements – Should be: used in conjunction with source monitoring; include sampling for all the parameters required for background groundwater quality monitoring; the application, design, construction, operation, and maintenance of the monitoring system should conform with the guidelines found in: Vadose Zone Monitoring for Hazardous Waste Sites; June 1983, KT-82-018(R).

Leak Detection Monitoring Requirements – Should not allow any leakage to escape undetected that may cause the receiving groundwater to exceed applicable groundwater protection levels during the active life and any required post-closure care period of the discharge point. This demonstration may be accomplished through the use of numeric or analytic, saturated or unsaturated, groundwater flow or contaminant transport simulations, using actual filed data or conservative assumptions. Provide plans for daily observation or continuous monitoring of the observation sump or other monitoring point and for the reporting of any fluid detected and chemical analysis thereof.

Specific Requirements for Other Methods – Demonstrate that: the method is as or more reliable, effective, and determinate than a viable groundwater monitoring well network at detecting any violation of groundwater protection levels or class TDS limits, that may be caused by the discharge or potential discharge; the method will provide early warning of a potential violation of groundwater protection levels or class TDS limits and meets or exceeds the requirements for Vadose zone or leak detection monitoring.

Monitoring well construction and groundwater sampling should conform to A Guide to the Selection of Materials for Monitoring Well Construction. Sample collection and preservation, should conform to the EPA RCRA Technical Enforcement Guidance Document, OSWER-9950.1, September, 1986. Sample analysis must be performed by State-certified laboratories by methods outlined in UAC R317-6-6.3L. Analytical methods used should have minimum detection levels which meet or are less than both the groundwater quality standards and the anticipated protection levels.

11. Closure and Post Closure Plan: The purpose of this plan is to prevent groundwater contamination after cessation of the discharge or potential discharge and to monitor the discharge or potential discharge point after closure, as necessary. This plan has to include discussion on: liquids or products, soils and sludges; remediation process; the monitoring of the discharge or potential discharge point(s) after closure of the activity.

12. Contingency and Corrective Action Plans: The purpose of this Contingency plan is to outline definitive actions to bring a discharge or potential discharge facility into compliance with the regulations or the permit, should a violation occur. This applies to both new and existing facilities. For existing facilities that may have caused any violations of the Groundwater Quality Standards or class TDS limits as a result of discharges prior to the issuance of the permit, a plan to correct or remedy any contaminated groundwater must be included.

Contingency Plan – This plan should address: cessation of discharge until the cause of the violation can be repaired or corrected; facility remediation to correct the discharge or violation.

Corrective Action Plan – for existing facilities that have already violated Groundwater Quality Standards, this plan should include: a characterization of contaminated groundwater; facility remediation proposed or ongoing including timetable for work completion; groundwater remediation.

Certification

I certify under penalty of law that this document and all attachments were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gather and evaluate the information submitted. Based on my inquiry of the person or persons who manage the system or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate, and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations.

Jade Dalton - Owner
NAME & OFFICIAL TITLE (type or print)

435-616-3081
PHONE NO. (area code & no.)

SIGNATURE

DATE SIGNED

SECTION 2: DESCRIPTION OF PRODUCTION FACILITIES

The facility is to be located in the north ½ of sections 11 and 12, T31S, R4W SLB & M. The site will be an 13,200 hog finisher site with a double basin to contain the waste. The waste will be contained only temporarily and will be utilized as fertilizer on the adjacent fields to grow crops. There will be no treatment of the waste except that which occurs naturally as the waste sets in the containment basin before it is applied to the fields as fertilizer.

Potential discharges would include the possible leaking of the basins or the pipes into or out of the facility. Both the influent into and the effluent out of the facility will be closely monitored. Therefore, any leakage will be identified by a coresponding drop in the amount of either influent or effluent. Furthermore, in compliance with groundwater discharge permit requirements, monitoring wells will be installed to verify that the groundwater is not contaminated due to sewage leakage.

2.1 Farm Site Population

There is one farm site proposed for this permit. The sites will consist of 8 (1100) head wean to finisher barns containing pigs sized from 60 to 270+ pounds. Table 1-2 summarizes the swine population anticipated for the farm sites:

Table 1-2

Animal Type	Average Animal Weight (lbs)	Population	Total Live Animal Weight (LAW) for Animal Type (lbs)
Finisher Pig	150	13,200/site	1,980,000

2.2 Farm Site Locations

The locations of the finishing farms are identified on Attachment A. Table 1-3 indicates the latitude and longitude of the site.

Table 1-3

Farm Number	Latitude	Longitude
1	N 38° 8' 2.86"	W 112° 15' 58.84"

SECTION 3: GEOLOGIC AND HYDRAULIC EVALUATION

3.1 Geologic Conditions

The rocks in and around the Upper Sevier River Valley range in age from Triassic, Jurassic, Cretaceous, Tertiary and Quaternary. The mountains surrounding the basin contain rocks of Precambrian through Tertiary age; these rocks are of sedimentary, metamorphic, and igneous types. Volcanic rocks of Tertiary and Quaternary age and consolidated-to-unconsolidated sedimentary deposits compose the basin fill. The valley fill material of the Circle Valley consists of alluvial deposits of silt, clay and sand up to gravel size materials. The thickness of valley fill deposits may be up to 680 feet in thickness in the vicinity near the proposed sites.

3.1.1 Faulting & Seismicity

The Upper Sevier River Valley lies within a zone of pronounced seismic activity. There are many faults in the Tushar Mountains approximately 3.5 miles to the west of the proposed farm sites with the closest mapped faults to the site being the Sevier Valley-Maysvale-Circleville area faults approximately 3.3 miles northwest of the proposed farm sites.

It does not appear that any known active faults transect the proposed farm sites.

From southwestern Utah to northwestern Montana (Christenson and Dean, 1983).

3.2 Stratigraphy

The stratigraphy at the site generally consists of alluvium and colluvium (Quaternary) The following description was taken from "Geologic Map of Utah and Ground-Water Conditions . and Geologic Reconnaissance of the Upper Sevier River Basin, Utah".

Faulting, erosion, and deposition by streams have shaped the several ground-water basins in the upper Sevier River basin. The valley fill in these basins has been derived from the consolidated and unconsolidated formations in the uplands that surround the valleys. In Circle and Grass Valley basins all the sediments are derived from volcanic rocks; in Panguitch and East Fork

Valley basins, the sediments are derived from both volcanic and sedimentary rocks. The sediments includes old alluvium, young alluvium, and flood-plain deposits.

Circle Valley is about 8 miles long and is more than 6 miles wide at Circleville. The altitude of the valley floor ranges from about 6,000 feet at the north end to about 6,200 feet at the south end. The valley is bordered on the west by the Tushar Mountains, which reach an altitude of more than 11,000 feet, and on the east by the Sevier Plateau.

3.3 Topography and Drainage

The proposed farm sites located in the Central Sevier Valley as described previously. The topographical slope at the proposed site and the surrounding area is approximately 2.5%. The approximate elevation at the proposed farm sites is approximately 6220 feet above sea level (see **Attachment A**).

3.4 Hydrologic Description

USGS topographic maps show that the Sevier River is approximately 7500 feet to the northwest from the proposed site. The Sevier River runs primarily in a north north-east direction in the Circle valley. Based on the location of the proposed sites the Sevier River is not likely to have an impact or be impacted by the proposed site location.

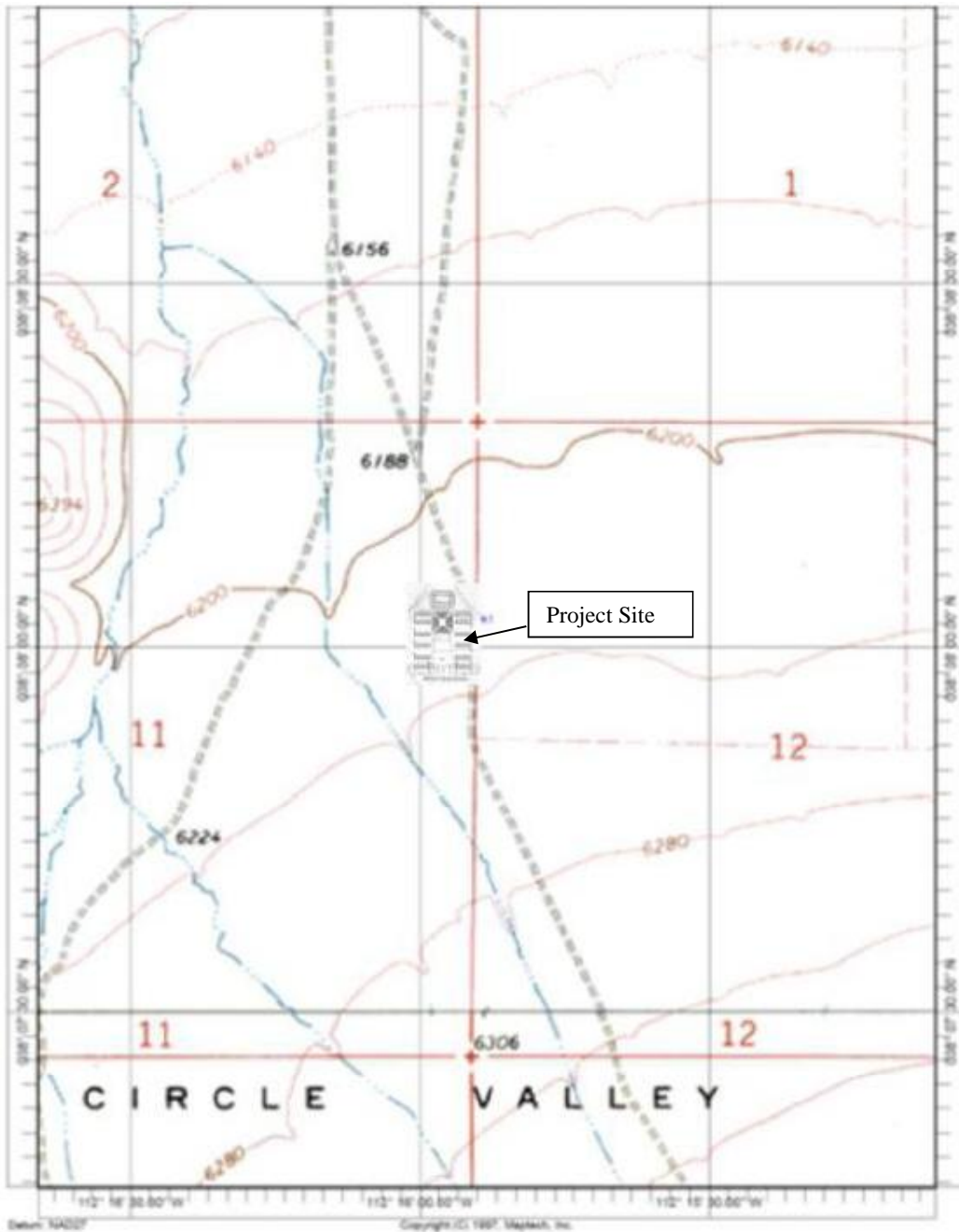


Figure 3-1 Topographic Map (USGS Map)

3.4.1 Groundwater Reservoir

In the State of Utah Geological Survey Paper #1836 entitled "Ground-Water Conditions and Geologic Reconnaissance of the Upper Sevier River Basin, Utah" (Carpenter, Robinson & Bjorklund), the hydraulic properties of the groundwater reservoir in the area

of the proposed farm sites are documented. The information from this publication is drawn upon freely in the following discussion.

The groundwater reservoir underlying the proposed sites is in the Circle Valley portion of the Sevier River Basin. The ground water deposit in the vicinity of the farm sites is mostly unconsolidated and semi-consolidated alluvial deposits that form interbedded lenticular soil layers. Pump tests conducted by the Department of Natural Resources suggest that the reservoir acts as a single aquifer over time, but due to the lenticular nature of the deposits, extensive hydraulic continuity most likely does not exist everywhere. The unconsolidated deposits that make up the reservoir are composed of gravel, sand, silt and clay. The thickness of the reservoir exceeds 500 feet through the central valley and is approximately 100 to 300 feet thick under the proposed facility location.

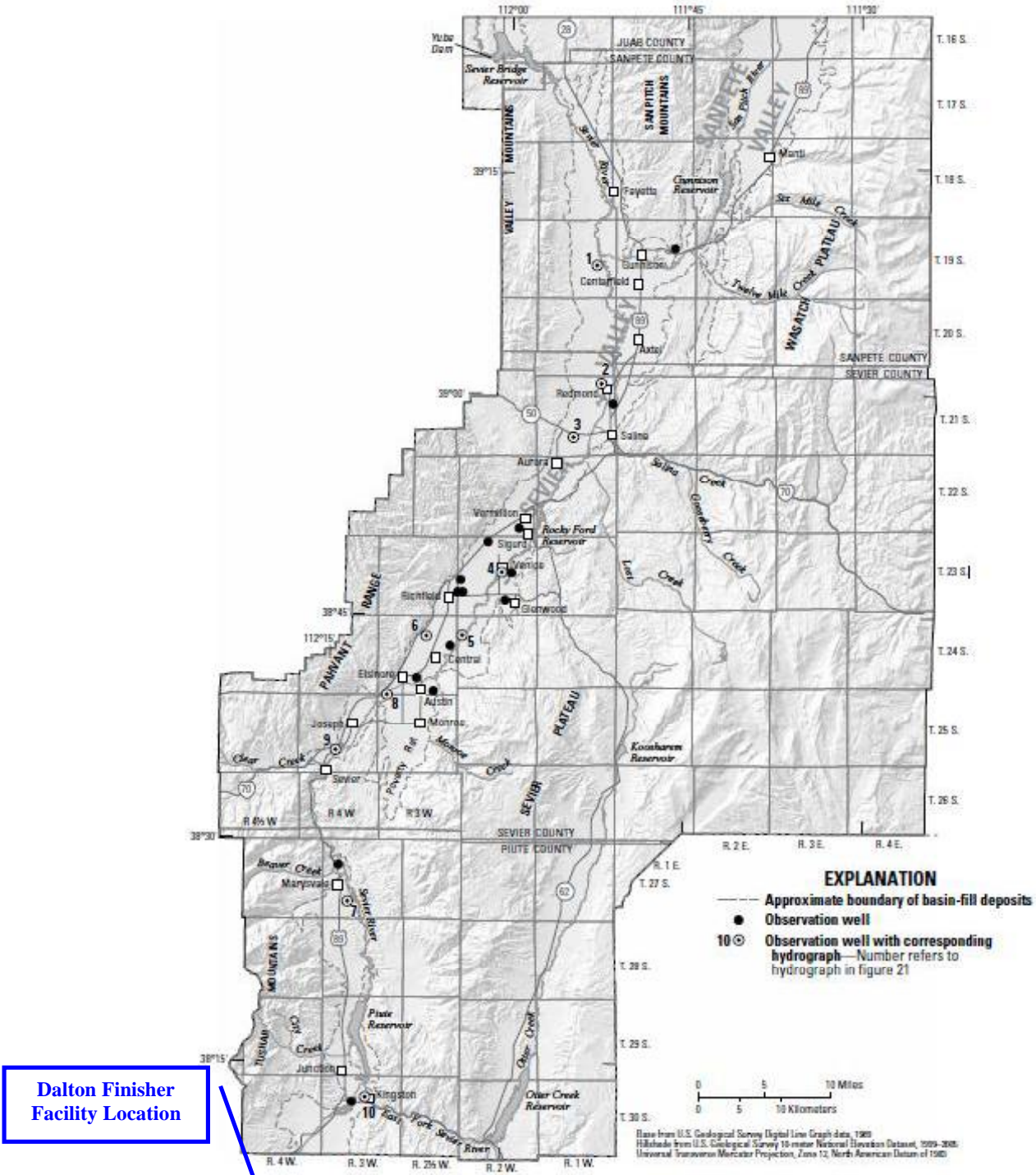


Figure 3-2 Well Locations Near Dalton Finisher Farm Sites

3.4.2 Groundwater Movement

The groundwater in the southern portion of the Circle Valley of the Sevier River Basin is recharged by ephemeral streams (Mostly the Sevier River), subsurface inflow from bedrock in the mountains, precipitation on the valley floor. The groundwater in the area flows to the northwest generally down in the direction of the flow of the Sevier River. - (Appendix F - Sevier "Ground-Water Conditions and Geologic Reconnaissance of the Upper Sevier River Basin, Utah" (Carpenter, Robinson & Bjorklund)) the ground water slope in the basin is estimated 0.22 to 0.3 % to the north - northwest under the proposed site. The groundwater's approximate depth under the proposed facility site is 45 to 65 feet below existing ground level in the vicinity of the proposed sites.

The rate of lateral movement in the aquifer is extremely slow compared to that of a surface stream. The well logs for the wells in the area indicate silty sand and sandy clay at water table depth. Therefore, the percentage of sand in the aquifer beneath the site can be assumed to be between 10% and 15%. The Transmissivity for the full underlying aquifer thickness is approximately 3,000 to 5000 ft² /day as interpreted from the 1993 study as shown in **Figure 3-5 below.**

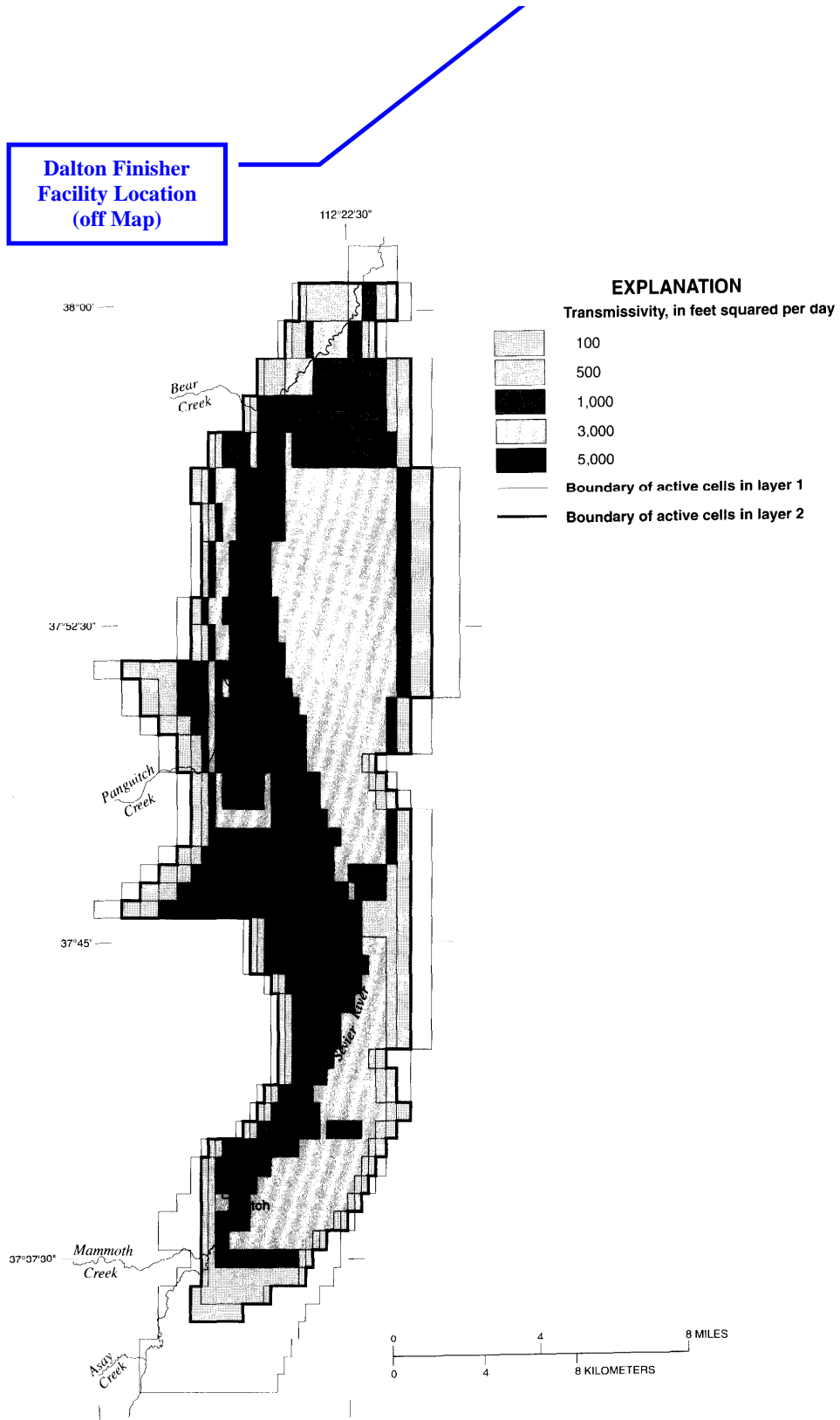


Figure 19 Distribution of transmissivity used in layer 2 of the digital-computer model.

Figure 3-5 Transmissibility of Lower and Upper Artesian Aquifers

3.4.3 Groundwater Quality

Existing wells referred to as piezometric wells in this report have been used to analyze the groundwater quality surrounding the proposed sites. Some of these wells are described in **Table 3-1** and the water quality test results are shown in table 1-4 below. The well logs and portion of water quality report from the USGS and Utah Department of Natural Resources are included in Appendix B.

Table 1-4 - Water Quality Test Results 2016 – Upper Sevier River Valley Area – Groundwater Conditions in Utah, Spring of 2016

Potassium dissolved, in mg/L	Sodium, dissolved, in mg/L	ANC, fixed end point, lab, in mg/L as CaCO ₃	Bromide, dissolved, in mg/L	Chloride, dissolved, in mg/L	Fluoride, dissolved, in mg/L	Silica, dissolved, in mg/L	Sulfate, dissolved, in mg/L	Solids, dissolved, residue at 180°C, in mg/L	Nitrate plus nitrite, dissolved, in mg/L as N	Orthophosphate, dissolved, in mg/L as P
<i>Upper Sevier River Area</i>										
5.89	14.5	181	0.16	26.4	0.2	50.5	16.7	281	1.02	0.081
3.27	17	126	0.04	9.5	0.27	38.3	8.5	187	2.62	0.176

3.4.4 Chemical Quality of Water

Chemical analyses taken from the groundwater surrounding the proposed sites are included Table 1-4 represents data taken in 2016.

SECTION 4: GROUNDWATER DISCHARGE CONTROL PLAN

The finisher farm is designed as a closed system and therefore with the exception of the Septic system which will be designed and approved through the local health department no wastewater will be discharged to the surrounding soil.

4.1 Finisher Waste Management Description

A diagram of the overall operation of the finisher facility is found in **Figure 4-1**

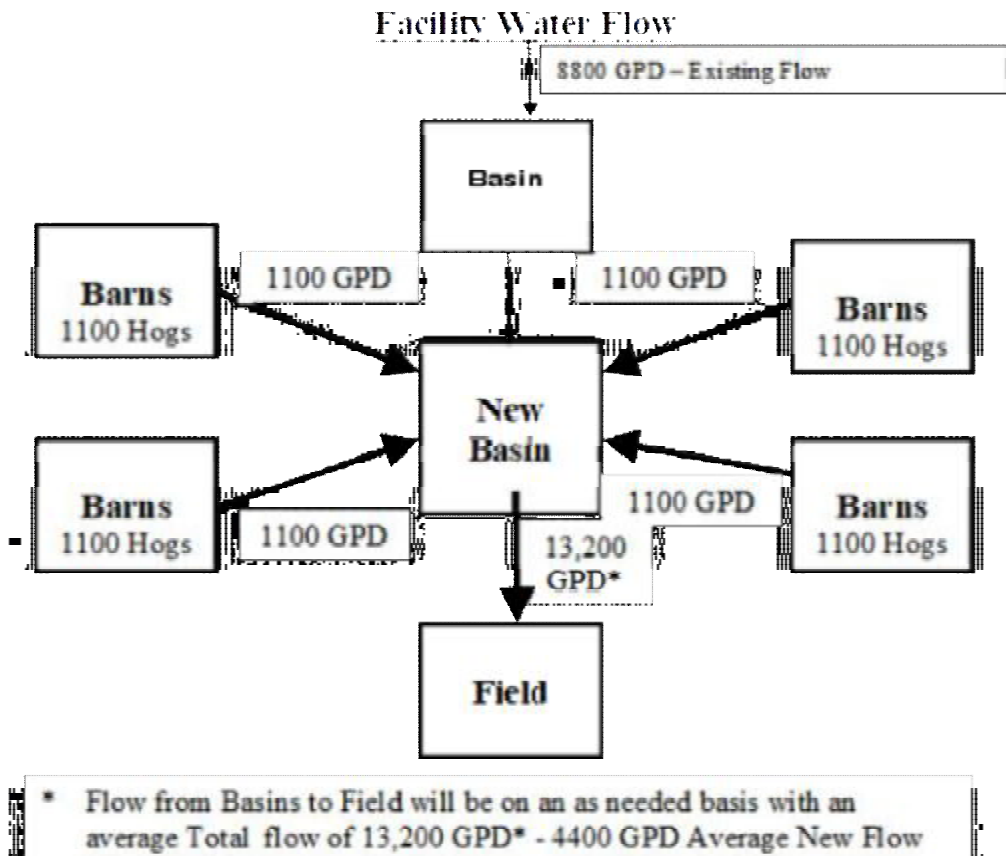


Figure 4-1 Finisher Flow Diagram

4.1.1 Waste Flow Description

The sewage collected from the individual finisher buildings will drain into the waste

containment basin. The waste will not be treated but will be pumped to agricultural fields at an agronomic rate to be utilized as fertilizer. The level of fluid in the containment will be strictly monitored and controlled. The basins are designed to hold approximately 425 days of waste produced by the hogs in the barn at full capacity.

4.1.2 Soil Information

The soil and water table around the site were investigated by reviewing the well logs for the wells which are near the facilities:

Well # 1 – WIN#: 22343: S 15 ft, W 660 ft from NE corner of Section 2,
T 31S, R 4W, SL B&M

Well # 2 – WIN#: 429786: N 28 ft, W 78 ft from SE corner of Section 35,
T 30S, R 4W, SL B&M

Soil logs for the locations listed above are located in **Attachment B**. Information was obtained from Utah Division of Water Rights.

The shallowest groundwater in the surrounding borings was located roughly 43 feet below existing grade at Well # 1 which is closest to the facility site. An excavation was made at the site and the groundwater is estimated to be about 60 feet below the ground surface.

In order to meet DEQ criteria for Containment Basin construction, the seasonal high water table elevation must be at least 2 feet below the floor of the containment basin in hydrogeologically stable soil strata. At the facility location, the seasonal high water table will be more than 2 feet below the bottom of the proposed containment basin. Also, the soil strata underlying the facility site appear to be hydrogeologically stable. It appears that the proposed site will meet this criteria.

4.1.3 Containment Basin Overview

The owner of this facility will follow the previously accepted design criteria in developing containment basins for this facility. No digestion of the waste is necessary, because the hog manure will be utilized as fertilizer on an agricultural field.

A plan view of the containment basin is shown in **Figure 4-3**

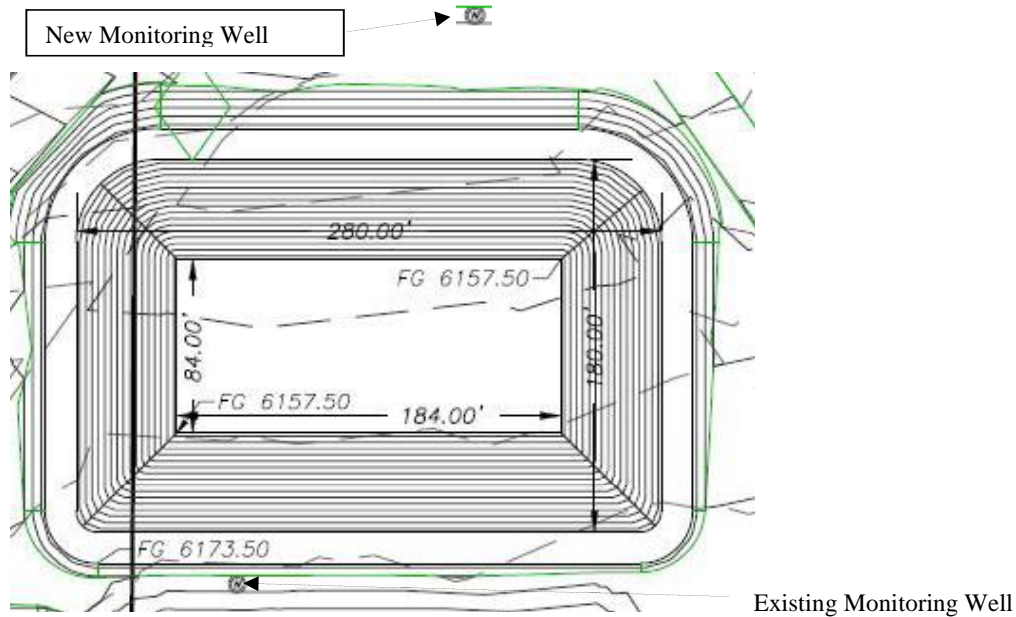


Figure 4-3 New Containment Basin Detail and Monitoring Well Location
(There is an existing containment basin than proceeds this basin)

4.1.4 Waste Conveyance System

Waste shall be conveyed from the farm sites to the containment basin through either HDPE SDR 35 or PVC Schedule 40 sewer pipe, as shown in the Composite Location & Plot Map included in **Attachment A**. The waste will gravity flow from the barns to the waste containment basins. The containment basin will be lined with a Flexible Membrane Liner (FML).

4.1.5 Containment Basin Management Plan

As previously described, the waste flows from the barn to containment basins and then is pumped to the fields at an agronomic rate. Should problems be encountered either in the liner or piping, the flow of sewage from the individual farm sites can be shut off and the contents of the basin(s) can be pumped to the existing field or containment basin so that repairs can be made and the containment basin be put back into use.

SECTION 5: COMPLIANCE MONITORING PLAN

5.1 Groundwater Monitoring

One additional down gradient monitoring well be drilled for compliance monitoring of the new containment basin site at the facility site. There is an existing upgradient well at the site which is currently the down gradient well for the existing basin. A Water Quality Sampling, Handling and Analysis Plan is included as **Attachment F**. All water samples taken from the monitoring wells will be processed according to the guidelines set forth in this plan. The installation guidelines and an outline of the proposed groundwater monitoring plan are as follows:

- 1.) A Downgradient monitor well will be constructed. The proposed locations of these wells are shown in **Figure 4-3**. The monitor wells will have a total depth of 10' below the first encountered water table and will be constructed and developed as per requirements of the State of Utah, Department of Environmental Quality. The monitor wells will typically be constructed as shown in **Figure 5-1**. The upgradient wells will provide background data for the downgradient monitoring wells. The wells will be constructed at the location shown in **Figure 4-3** and in **Attachment D** at the proposed site.
- 2.) The static water level in each well and the elevation of the water level will be determined at least 8 days after the well has been completed. The water levels at each well will be compared with existing data to confirm the direction of groundwater movement.
- 3.) Monitoring wells will be sampled and tested according to the procedures outlined in the Water Quality Handling and Analysis Plan (**Attachment F**). It is anticipated that the monitoring wells will be 70 to 80 feet in depth below the ground surface at the proposed farm site location.

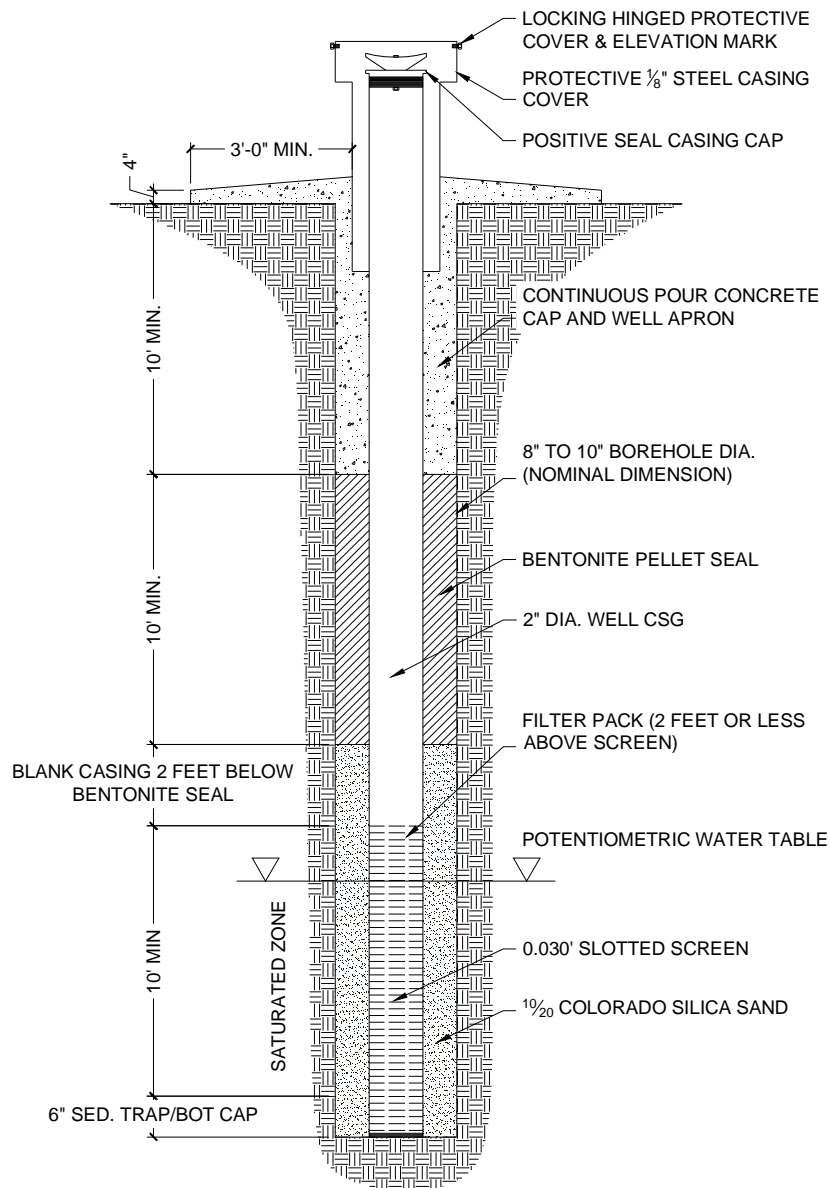


Figure 5-1 Typical Monitor Well Detail

5.1.1 Upgradient Groundwater Monitoring

The upgradient monitor well will be constructed and sampled prior to waste introduction to the Containment Basin at the site. The water in the upgradient well will be sampled and analyzed at least 8 days after the well is completed in order to determine the groundwater class protection levels and begin to establish background mean concentration levels. The groundwater protection levels of the upgradient well will be

determined according to UAC R317-6-4 from the analysis of eight independent samples taken at equal intervals during a period of one year. The accelerated background constituents that will be analyzed in a laboratory include: total dissolved solids, sulfate, calcium, magnesium, potassium, sodium, carbonate, bicarbonate, total phosphorous, chloride, nitrate-N/nitrite-N, and ammonia-N. The parameters that will be determined at the monitor well include: static water level, pH, temperature, and specific conductance.

The background mean concentration levels will be determined by averaging the upgradient monitor wells accelerated background data, then adding 2 standard deviations. The following parameters will constitute the quarterly monitoring from the upgradient well after all eight background analysis: static water level, pH, temperature and specific conductance. Also, the following constituents will be monitored quarterly: nitrate-N/nitrite-N, ammonia-N, total dissolved solids, bicarbonate, and chloride. After the groundwater properties have been well established the analysis frequency may be decreased to semi-annually.

5.1.2 Downgradient Groundwater Monitoring

If data from upgradient monitor wells indicate differing movement of groundwater than what is shown in this application, the locations for the downgradient monitor wells will be changed, sent to the DEQ for approval, and drilled at a different location than proposed in this application.

A first sample will be taken from the downgradient well at least eight days after it's construction and prior to waste flow to the digester system. Only the first sample from the downgradient well will be analyzed for the background parameters described in Section 5.1.1. After the first analysis, the well will be analyzed on a quarterly basis for the following constituents: nitrate-N/nitrite-N, ammonia-N, total dissolved solids, bicarbonate and chloride. The following field parameters will also be analyzed: static water level, pH, temperature, and specific conductance. After the groundwater properties have been well established the analysis frequency may be decreased to semi-annually.

5.1.3 Alternative & Additional Monitoring

In the event that the chemical quality proves that a common source comparison does not exist between the upgradient and downgradient well, a different background monitoring schedule may be proposed to the Department of Environmental Quality's Executive Secretary.

Additional Monitoring: Identification of the contaminants in the wastewater will be analyzed once a year. The analysis will identify the parameters required under the accelerated background monitoring at upgradient wells and also, the metals listed in Table 1 of the Groundwater Regulations, R317-6-6.3, (arsenic, barium, cadmium, chromium, copper, lead, mercury, selenium, silver and zinc).

SECTION 6: CONTAINMENT BASIN SYSTEM LOCATION AND DESIGN

6.1 Containment Basin Description

Containment basins will be used at this finisher site to store the swine manure produced at the finisher sites. Effluent will be collected from the production building in two Containment Basins where the effluent will be stored allowed to evaporate. The Containment Basins will be lined. The liners will consist of a Flexible Membrane Liner (FML). The waste contained in the containment basins will be pumped and utilized as fertilizer in the near by fields.

6.2 Containment Basins Site Soils Investigation

A soil and water table investigation will be performed near the proposed Containment Basin locations before construction. The soil investigations consisted of 2 backhoe trenches approximately 12 feet in depth near the proposed farm sites. The groundwater underlying the Containment Basin must be at least 8 feet below the existing ground level. In order to meet DEQ criteria for Containment Basin construction, the seasonal high water table elevation must be at least 2 feet below the floor of the Containment Basin in hydrogeologically stable soil strata. At the proposed farm site the seasonal high water table was more than 2 feet below the bottom of the proposed Containment Basin based on the hydrogeological information available. Also, the soil strata underlying the Containment Basins appear to be hydrogeologically stable. It is proposed that bottom of containment basin be placed approximately 15 feet below the ground surface at these farm site locations. It is estimated that the groundwater is approximately 60 feet below the ground surface at the proposed site location.

6.3 Containment Basins Design

The containment basins will be constructed with 60 mil HDPE liners as described in section 7 of this report and in accordance with the State of Utah Department of Environmental Quality regulations. A plan view and cross section of the containment basin can be found in Attachment D.

6.4 Waste Transfer System

Waste from the barns is transferred to the containment basin through either 12" SDR 32.5 HDPE

or 12” SDR 35 PVC sewer pipe, installed at a 0.5% minimum slope. The effluent pipe and Containment Basins elevations allow the waste to gravity flow from the pits to the Containment Basins. The waste will then be pumped to the agricultural field for use as fertilizer at an agronomic rate.

6.5 Containment Basins Safety System Considerations

Access to the Containment Basins by humans and animals will be controlled by fencing. The fences will help to prevent damage to the Flexible Membrane Liners (FMLs) in the instances where they are used. Only authorized personnel will have access to the Containment Basin areas to prevent damage to the FMLs. Additionally, safety-warning signs will be posted near the Containment Basins.

6.6 Containment Basins Management Plan

The Containment Basins will be managed as a fertilizer producing system. The Containment Basins are designed to contain all of the waste produced by the hogs for 425 days. The waste will be pumped to the fields at an agronomic rate. Since the prevailing climatological conditions result in more evaporation than precipitation no excess volume will be provided other than the free board of 1.5 feet as show on the lagoon cross section in Attachment D. However, should unforeseen precipitation events occur, excess effluent could be land applied at agronomic rates at any time. The effluent will be applied according to soil and plant nutrient uptake rates. In this case, the effluent will be applied in a manner such as to avoid any contamination of surface waters, drinking wells, springs or pipelines.

SECTION 7: LAGOON AND CONTAINMENT BASIN CONSTRUCTION

7.1 Containment Basins Construction

Construction of the Basins and Containment Basins shall be done in accordance with design drawings and specifications. Earthwork and liner construction shall be tested and inspected by qualified independent geotechnical and/or engineering firms. At the completion of construction, and prior to operation of the facility, an independent performance certification document will be completed by a qualified professional engineer licensed in the State of Utah containing test information and certification that basin and liner construction meets requirements of the project design documents and the requirements contained within this report.

7.1.1 General Earthwork Construction

Earthwork and dike construction for excavation of digesters and equalization basins shall be done as follows:

- A. The area scheduled for construction of basins and building pads shall be cleared and grubbed to remove topsoil and surface vegetation from the digester/basin areas.
- B. Soil shall be excavated from the basin area and be used to construct building pads or dikes.
- C. Basin dikes shall be constructed in 6-inch compacted lifts to obtain proper compaction. For building pad and digester dike construction, the soil shall be moistened and compacted to 90% of maximum dry density, as defined by AASHTO T-99. Moisture will be added to the soil during compaction to target 2% above the optimum moisture.
- D. The dikes will be constructed of relatively impermeable compacted native material.
- E. A qualified inspector will perform the moisture content and dry density testing per every two feet of lift at random locations once every 400 feet along the Containment Basin dikes.

7.2 Flexible Membrane Liner

Specifications for manufacture, delivery, subgrade preparation, installation, and testing for FML liner installation are included in **Attachment E**. The QA/QC plan is also included in this attachment. The specifications were adapted from requirements set forth in previous projects and permit applications. Moreover, an industry standard known as the GRI standard GM13 which covers smooth and textured geosynthetics has been developed with the intent of forming an industry standard for manufacture and testing of geosynthetic liner material. This standard was developed by the Geosynthetic Research Institute at Drexel University, Philadelphia, PA. As stated in the specifications, the requirements of latest revision of the GRI standard will be applicable.

If the basins are to remain empty for an extended period of time they shall be properly ballasted using ultraviolet ray resistant sand bags with nylon ties. The minimum specification for ballasting liner is 30-lb. sand bags spaced 5-feet apart along the entire toe of dike in containment basins. Sand-filled HDPE tube or pipe may also be used as long as an equivalent amount of ballasting per lineal foot (6 lbs./ln.-ft.) is maintained.

On occasion, repairs may have to be made to liners if damage occurs out of the norm, or modifications need to be made. All repairs made to liner seams, or incident holes found in the liner shall be vacuum/bubble tested, documented and sent to the State DEQ for informational purposes and approval of the repairs. Unless significant modifications to the liner are made, such repairs shall be made without any requirements for approval from the State DEQ.

7.2.1 Flexible Membrane Liner Installation

The Containment Basins at the finishing farms will be lined with a Flexible Membrane Liner (FML) constructed of a High Density Polyethylene (HPDE). The subgrade will conform to the FML specifications of the Manufacture and the previously stated most recent GRI standards. The installation of the FML will also comply with the Quality Assurance/Quality Control (QA/QC) found in Attachment E. In Addition to the FML specifications and QA/QC, detailed drawings of typical liner anchoring methods, pipe penetrations, air vents and water level markings of liners are found in attachment E. The

following procedures will be used for installation of liners for the Containment Basins at the farm sites.

The Subgrade will be constructed according to the specifications as detailed below:

1. The subgrade material will come from either on-site material or approved stockpiles.
2. The earthwork for the anaerobic Containment Basins will be free of any foreign material such as stones greater than 3/8 inch in diameter, vegetation, brush, roots or similar material which could damage the FML.
3. The subgrade material shall be classified as either CH, CL, CL-ML, ML, SM, SC, SW or SP by the USCS Classification System.
4. A Moisture density curve will be developed for the subgrade material.
5. The minimum compacted thickness of the subgrade layer shall be 8 inches.
6. The subgrade will be compacted and graded to meet the FML contractor's specifications so as to avoid any ruts, irregularities or soft areas. The subgrade will be thoroughly compacted to provide support for the FML.
7. The subgrade will be compacted to a minimum of 90% maximum dry density as defined by AASHTO T-99. For proper compaction, moisture will be added to the soil during compaction to target 2% above the optimum moisture.
8. Installed density shall be confirmed by field test methods at a frequency of one test per 100'x100' grid square at the surface of the subgrade.

A 60-mil HDPE will be installed over the compacted subgrade. The HDPE material will meet the specifications indicated in the most recent GRI standard and in the QA/QC references in Attachment E. The drawings in Attachment E show typical liner anchoring methods and pipe penetrations of the liner material.

The HDPE liner will be installed according to the following procedure:

1. The earthwork for the anaerobic Containment Basins will be constructed so the subgrade will be free of any foreign material such as stones greater than 3/8 inch in diameter, vegetation, brush, roots or other similar materials which could damage the FML.
2. The earthwork will be compacted and graded to meet the FML contractor's specifications so as to avoid any ruts, irregularities or soft areas. The subgrade will be thoroughly compacted to provide support for the FML.
3. An anchor trench will be constructed along the crest of the berms for the purpose of securing the FML.
4. The FML will be assembled, seamed, tested and installed by the methods specified by a liner material recognized by the NSF (National Sanitation Foundation, Standard 54).
5. The FML will be certified as "holiday free" by electrical potentiometric means (spark tested) during manufacture.
6. Adequate slack will be maintained in the liner material during assembly and installation to minimize stresses due to variations in ambient temperature and incident radiation.
7. Heavily creased or otherwise defective liner material must be rejected.
8. Testing of coupons (strips of material) before seaming, stress cracks and all seams must be done in accordance with the manufacture's requirements.
9. Installation of the FML will ideally take place in temperatures ranging from 40 degrees Fahrenheit to 110 degrees Fahrenheit. In the event that the FML is installed during colder conditions (between 20 degrees Fahrenheit and 40 degrees Fahrenheit) the cold weather seaming procedures detailed in FML QA/QC, Attachment E, shall be followed.
10. Air Vents will be installed on all four sides of the Containment Basin as detailed in Attachment E and Compaction of the anchor trench backfill will provide a firm unyielding surface to secure the FML along the berms.

SECTION 8: FACILITY CLOSURE AND POST CLOSURE

Should facility operation terminate the liquid and sludge will be removed and land applied at agronomic rates unless alternative technologies are developed. The sludge and Containment Basins liquid will be land applied in such a way as to avoid ground water pollution as well as contamination of surface waters, drinking wells, springs or pipelines. Additionally, the parameters and constituents of the water in the monitoring wells detailed in Sections 5.1.1 and 5.1.2 will be observed for 5 years thereafter. The actual duration of post operation monitoring may be less, if justified by long term operation and a history of compliance.

SECTION 9: CONTINGENCY AND CORRECTIVE ACTION

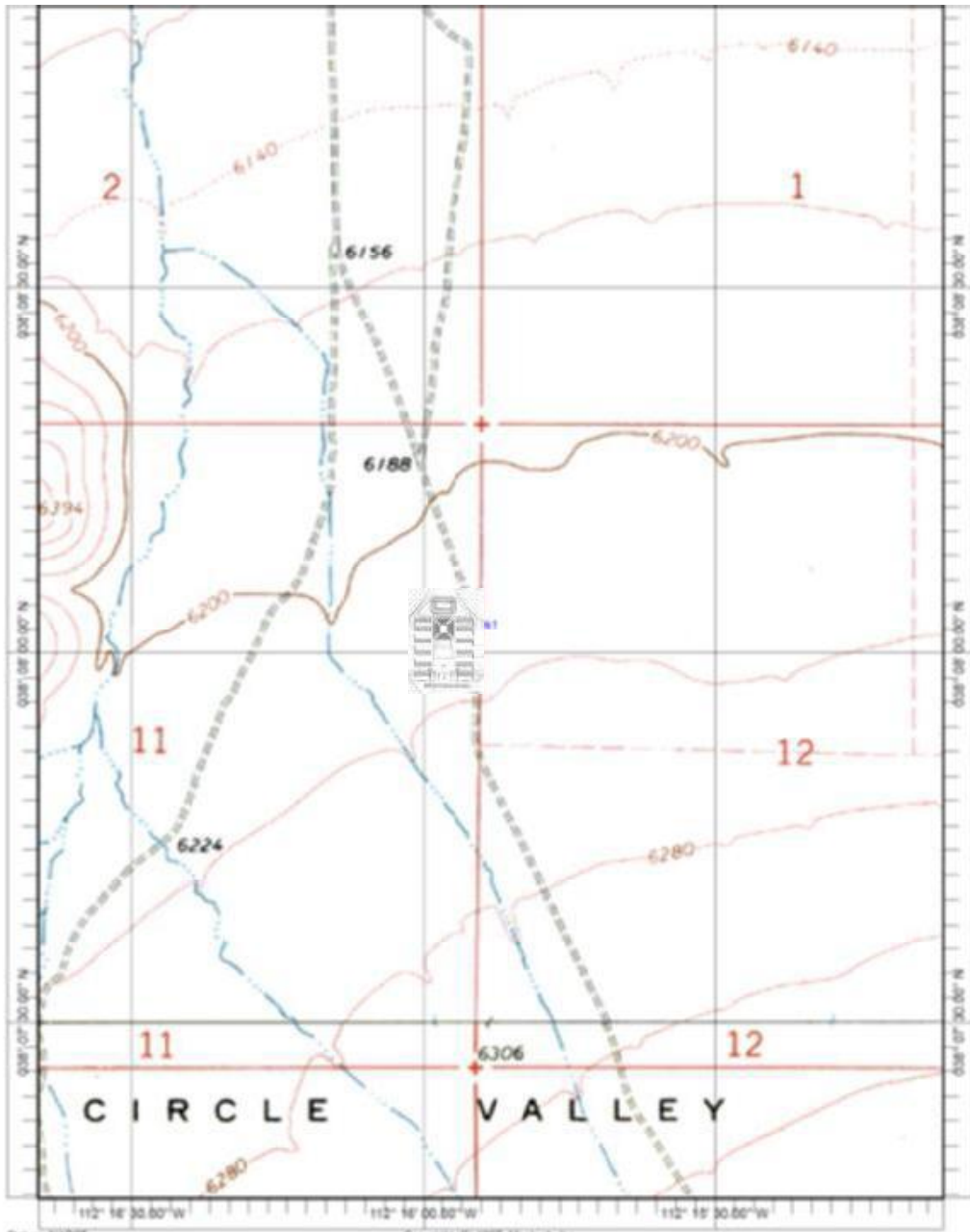
In addition to monitoring wells described in the previous section additional precautions will be implemented. The influent and effluent will be monitored on a regular basis to check for variations in the composition and quantity. The facility condition will also be checked on a daily basis to check for, among other things, damage to piping or liners and waste elevation in the containment basin. Should it become necessary to empty the containment basins for repairs, the liquid from the target basin will be transferred to one or more of the other existing Containment Basins or applied to the land at agronomic rates. Once any necessary repair work has been completed, the liner will be evaluated and re-certified prior to the reintroduction of liquid.

SECTION 10: ADJACENT PROPERTY OWNERS

The Dalton Hay Company, LLC own all of the land surrounding the proposed site.

ATTACHMENTS

Attachment A – Composite Location and Topographic Map



Attachment B –Well Logs and Water Quality Report

Utah Division of Water Rights



WELLPRT Well Log Information Listing

Version: 2003.09.18.00 Rupdate: 10/12/2003 12:39 PM

Utah Division of Water Rights

Water Well Log

LOCATION:

S 10 ft W 660 ft from NE CORNER of SECTION 2 T 31S R 4W BASE SL Elevation: feet

DRILLER ACTIVITIES:

ACTIVITY # 1 NEW WELL
 DRILLER: GRIMSHAW DRILLING LICENSE #: 240
 START DATE: 06/27/2000 COMPLETION DATE: 07/03/2000

BOREHOLE INFORMATION:

Depth(ft)	Diameter(in)	Drilling Method	Drilling Fluid
From To			
0 185	12.2	MUD ROTARY	BENTONITE

LITHOLOGY:

Depth(ft)	Lithologic Description	Color	Rock Type
From To			
0 43	SILT,SAND		
43 73	WATER-BEARING,HIGH-PERMEABILITY,SAND,GRAVEL,COBBLES,BOULDERS		
73 125	WATER-BEARING,HIGH-PERMEABILITY,SAND,GRAVEL		
125 155	WATER-BEARING,HIGH-PERMEABILITY,GRAVEL,COBBLES		
	PEA GRAVEL		
155 185	GRAVEL,COBBLES,BOULDERS		

WATER LEVEL DATA:

11/9/2017

<https://waterrights.utah.gov/docSys/v907/e907/e90705ue.htm>

Date	Time	Water Level (feet) (-)above ground	Status
07/03/2000		43.00	STATIC

CONSTRUCTION - CASING:

Depth(ft) From	Material	Gage(in)	Diameter(in)
185	A53 GRADE B	.188	8.62

CONSTRUCTION - SCREENS/PERFORATIONS:

Depth(ft) From	Screen(S) or Perforation(P)	Slot/Perf. siz	Screen Diam/Length Perf(in)	Screen Type/# Perf.
105	PERFORATION	.125	3	8 ROUND

CONSTRUCTION - FILTER PACK/ANNULAR SEALS

Depth(ft) From	Material	Amount	Density(pcf)
20	BENTONITE HOLE PLUG	10 BAGS	
195	3/8 PEA GRAVEL	6 YDS	

WELL TESTS:

Date	Test Method	Yield (CFS)	Drawdown (ft)	Time Pumped (hrs)
07/03/2000	AIR LIFT	1.114	2	2

GENERAL COMMENTS:

CONSTRUCTION INFORMATION
Well Head Configuration: Steel plate
Casing joint type: Butt weld
Perforator used: milled slot
Surface seal installed: yes
Depth of seal: 20 ft.
Drive shoe: no
Surface seal placement method: Tremie 20 ft back to surface
FILTER PACK
Grout density: 100%
Additional data not available.

WELL DRILLER'S REPORT

State of Utah
Division of Water Rights

For additional space, use "Additional Well Data Form" and attach

Well Identification

Change Application: a29728 (61-968)

WIN: 429786

Owner Note any changes

Dalton Brothers Farm
PO Box 326
Kingston, UT 84743

RECEIVED

38 JUN 22 2007

Contact Person/Engineer: _____

WATER RIGHTS
SALT LAKE

Well Location Note any changes

N 50 W 66 from the SE corner of section 35, Township 30S, Range 4W, SL B&M

Location Description: (address, proximity to buildings, landmarks, ground elevation, local well #)

Drillers Activity

Start Date: 4-25-07 Completion Date: 4-30-07

Check all that apply: New Repair Deepen Clean Replace Public Nature of Use: _____
If a replacement well, provide location of new well _____ feet north/south and _____ feet east/west of the existing well.

DEPTH (feet)		BOREHOLE DIAMETER (in)	DRILLING METHOD	DRILLING FLUID
FROM	TO			
0	202	17 1/2"	ROTARY	BENTONITE / POLYMER

Well Log		WATER		UNCONSOLIDATED					CONSOLIDATED		ROCK TYPE	COLOR	DESCRIPTION AND REMARKS (e.g., relative %, grain size, sorting, angularity, bedding, grain composition density, plasticity, shape, cementation, consistency, water bearing, odor, fracturing, mineralogy, texture, degree of weathering, hardness, water quality, etc.)
		High	Low	CLAY	SAND	GRAVEL	COBBLES	BOULDER					
DEPTH (feet)	FROM	TO											
0	27			X	X								800W
27	37			X		X							
37	62			X		X	X						
62	103	X	X			X	X						
103	107	X	X			X							
107	117	X	X			X	X						
117	123	X	X			X	X						
123	155	X	X			X	X						
155	164	X	X			X	X	X					
164	177	X	X			X	X	X					
177	202	X	X			X	X						

Static Water Level

Date 4-30-07 Water Level 72 feet Flowing? Yes No
 Method of Water Level Measurement FLOD If Flowing, Capped Pressure _____ PSI
 Point to Which Water Level Measurement was Referenced Top of Cassio Elevation _____
 Height of Water Level reference point above ground surface 2 feet Temperature _____ degrees C F

Well Log

Construction Information

DEPTH (feet)		CASING			DEPTH (feet)		SCREEN PERFORATIONS		OPEN BOTTOM
FROM	TO	CASING TYPE AND MATERIAL/GRADE	WALL THICK (in)	NOMINAL DIAM. (in)	FROM	TO	SCREEN SLOT SIZE OR PERF SIZE (in)	SCREEN DIAM. OR PERF LENGTH (in)	SCREEN TYPE OR NUMBER PERF (per foot/interval)
+2	202	A 53 GRADE B	.280	10 3/4	112	202	.125	2.50	10 PER FOOT 20 PER FOOT

Well Head Configuration: STEEL PLATE Access Port Provided? Yes No

Casing Joint Type: BUTT WELD Perforator Used: MILICO SLOT

Was a Surface Seal Installed? Yes No Depth of Surface Seal: 30 feet Drive Shoe? Yes No

Surface Seal Material Placement Method: TRENCH 30" TO TOP OF WELL

Was a temporary surface casing used? Yes No If yes, depth of casing: _____ feet diameter: _____ inches

DEPTH (feet)		SURFACE SEAL / INTERVAL SEAL / FILTER PACK / PACKER INFORMATION		
FROM	TO	SEAL MATERIAL, FILTER PACK and PACKER TYPE and DESCRIPTION	Quantity of Material Used (if applicable)	GROUT DENSITY (lbs./gal., # bag mix, gal./sack etc.)
0	30	2/8 BESTONITE HOLE PLUG	45 - 50 lb bags	100% WET
30	202	1/4" PEA GRAVEL	8.5 YARDS	100% WET

Well Development and Well Yield Test Information

DATE	METHOD	YIELD	Units Check One		DRAWDOWN (ft)	TIME PUMPED (hrs & min)
			GPM	CFS		
4-30-07	DIE LIFT	1200	X		18	4 hrs

Pump (Permanent)

Pump Description: _____ Horsepower: _____ Pump Intake Depth: _____ feet

Approximate Maximum Pumping Rate: _____ Well Disinfected upon Completion? Yes No

Comments Description of construction activity, additional materials used, problems encountered, extraordinary circumstances, abandonment procedures. Use additional well data form for more space.

Well Driller Statement

This well was drilled and constructed under my supervision, according to applicable rules and regulations, and this report is complete and correct to the best of my knowledge and belief.

Name: GRIMSHAW DRILLING License No. 240

Signature:  Date: 5-22-07

Central Sevier Valley

By Bradley A. Slauch

Central Sevier Valley, located in northern Piute, Sevier, and southern Sanpete Counties, in south-central Utah, is surrounded by the Sevier and Wasatch Plateaus to the east and the Tushar Mountains, Valley Mountains, and Pahvant Range to the west (fig. 20). Altitude ranges from 5,100 feet on the valley floor at the north end of the valley near Gunnison to more than 12,000 feet in the Tushar Mountains. Groundwater occurs in unconsolidated basin-fill deposits under both water-table and artesian conditions.

Total estimated withdrawal of water from wells in central Sevier Valley in 2015 was about 30,000 acre-feet, which is 1,000 acre-feet less than reported for 2014 and 5,000 acre-feet more than the average annual withdrawal for 2005–2014 (tables 2 and 3).

The location of 24 wells in central Sevier Valley in which the water level was measured during March 2016 is shown in figure 20. The relation of the water level in selected observation wells to annual discharge of the Sevier River at Hatch, Utah, to cumulative departure from average annual precipitation at Richfield Radio KVSC, to annual withdrawal from wells, and to concentration of dissolved solids in water from well (C-23-2)15deb-4 is shown in figure 21.

Discharge of the Sevier River at Hatch, Utah, in 2015 was about 47,300 acre-feet, which is about 32,400 acre-feet less than the 1940–2015 average annual discharge. Precipitation at

Richfield Radio KVSC was about 11.0 inches in 2015, which is about 2.9 inches more than the 1950–2015 average annual precipitation and 1.0 inch more than in 2014.

Water levels in central Sevier Valley generally declined in most areas from March 2015 to March 2016. Hydrographs for selected wells show that March water levels generally rose from about 1978 to 1985 and declined from 1985 to about 1993. Since 1993, water levels have fluctuated depending upon the amount and timing of precipitation and recharge to the basin-fill aquifer from snowmelt runoff.

The concentration of dissolved solids in water samples collected from well (C-23-2)15deb-4, located 0.1 mile south of Sevier River in Venice, from 1955 to 2015, is shown in figure 21. The concentration has ranged from 307 to 630 mg/L. There were substantial increases and decreases in dissolved-solids concentrations during the mid- to late 1960s and 1980s. Dissolved-solids concentrations in samples collected from 1990 through 2015 show little variability and are generally near the median value (410 mg/L) for all sample concentrations.

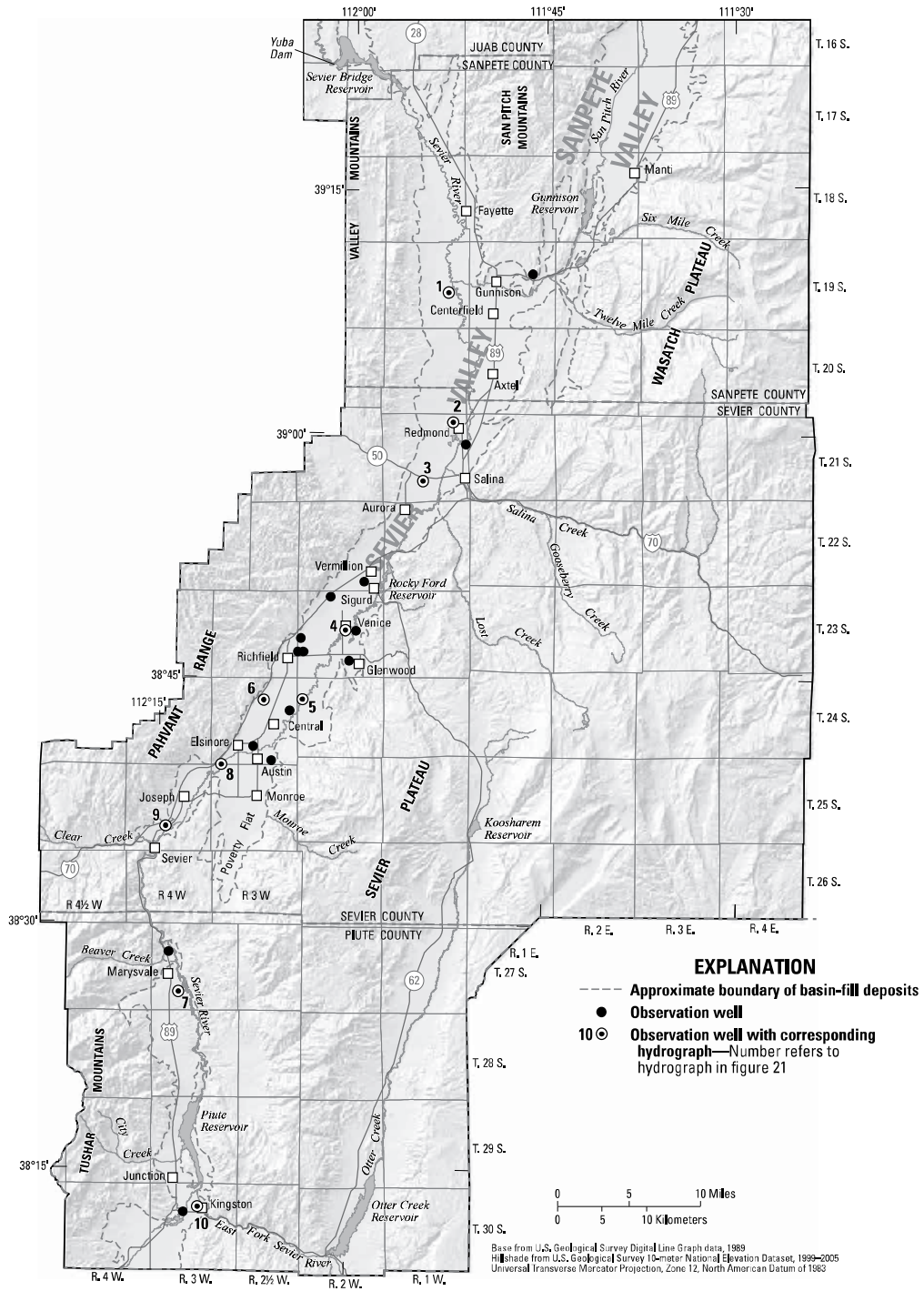


Figure 20. Location of wells in central Sevier Valley in which the water level was measured during March 2016.

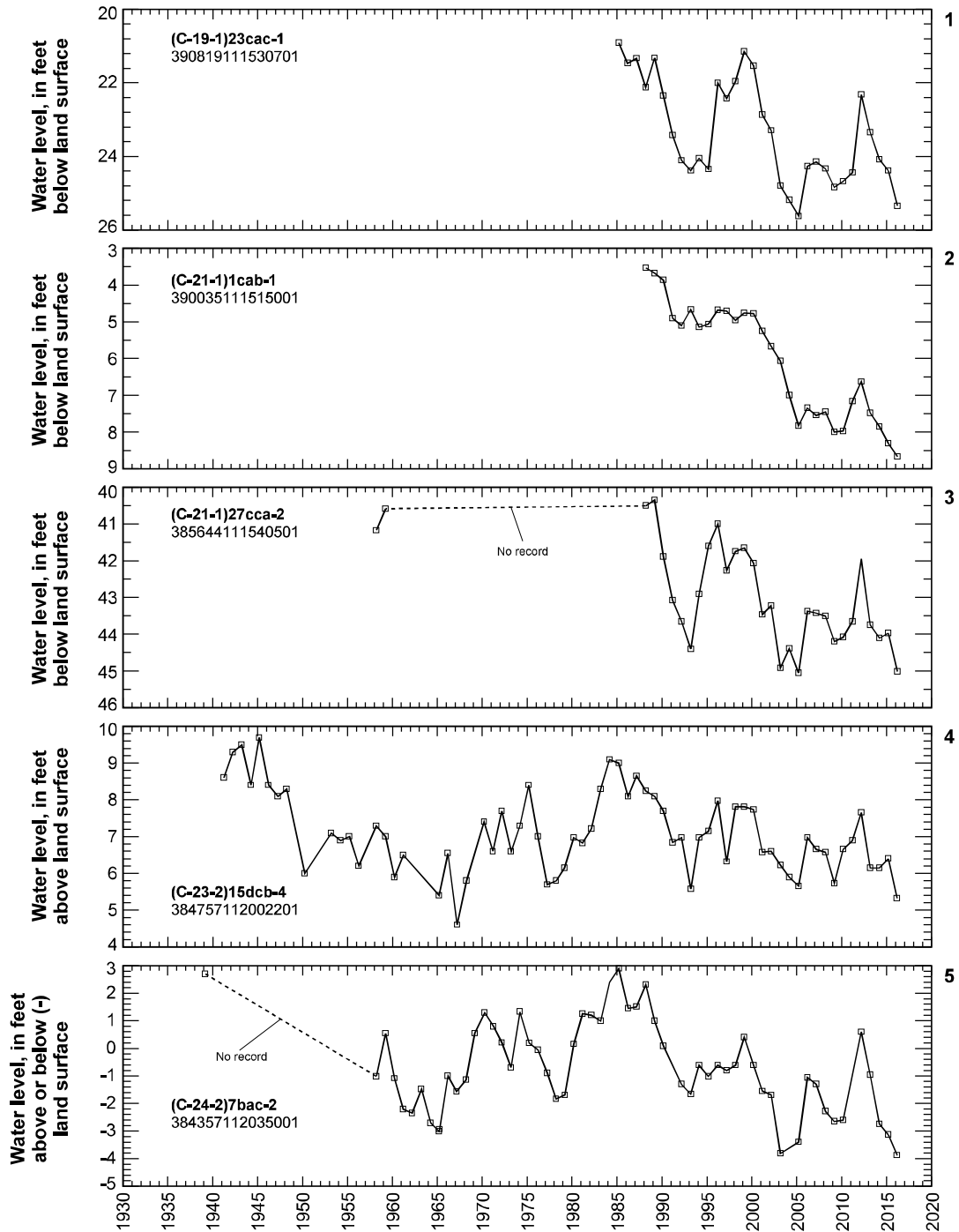


Figure 21. Relation of water level in selected wells in central Sevier Valley to annual discharge of the Sevier River at Hatch, Utah, to cumulative departure from average annual precipitation at Richfield Radio KVSC, to annual withdrawal from wells, and to concentration of dissolved solids in water from well (C-23-2)15dcb-4.

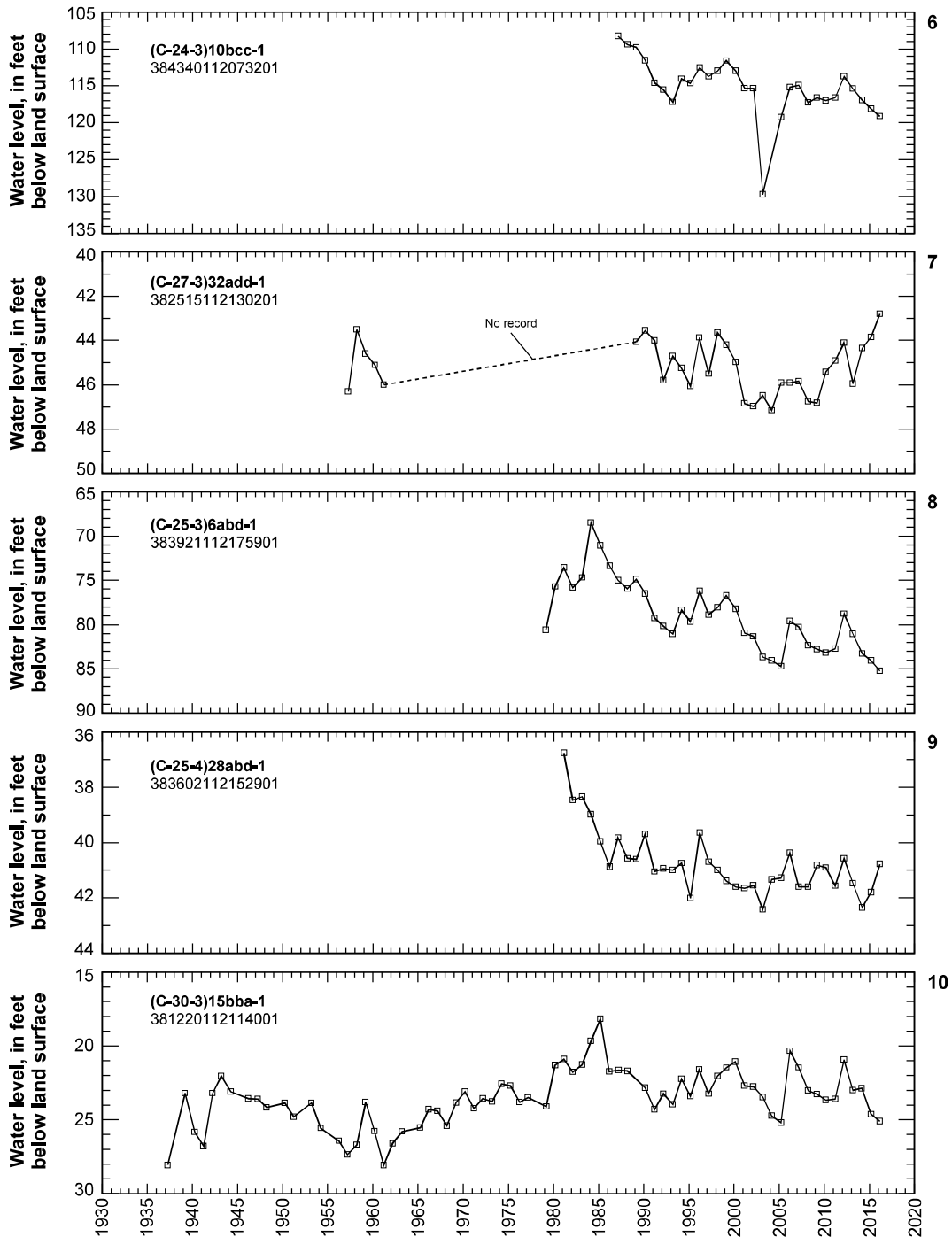


Figure 21. Relation of water level in selected wells in central Sevier Valley to annual discharge of the Sevier River at Hatch, Utah, to cumulative departure from average annual precipitation at Richfield Radio KVSC, to annual withdrawal from wells, and to concentration of dissolved solids in water from well (C-23-2)15dcb-4.—Continued

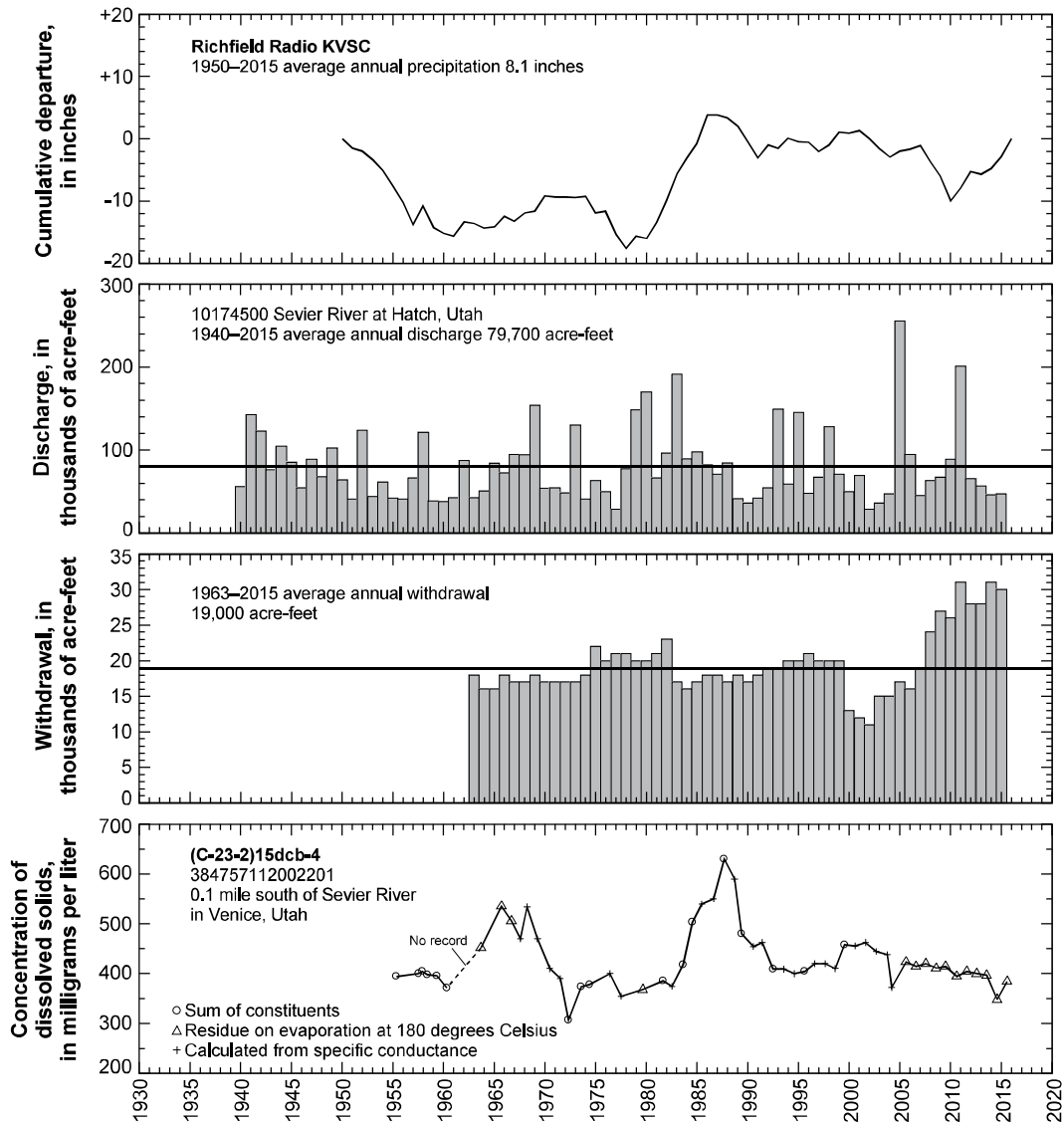
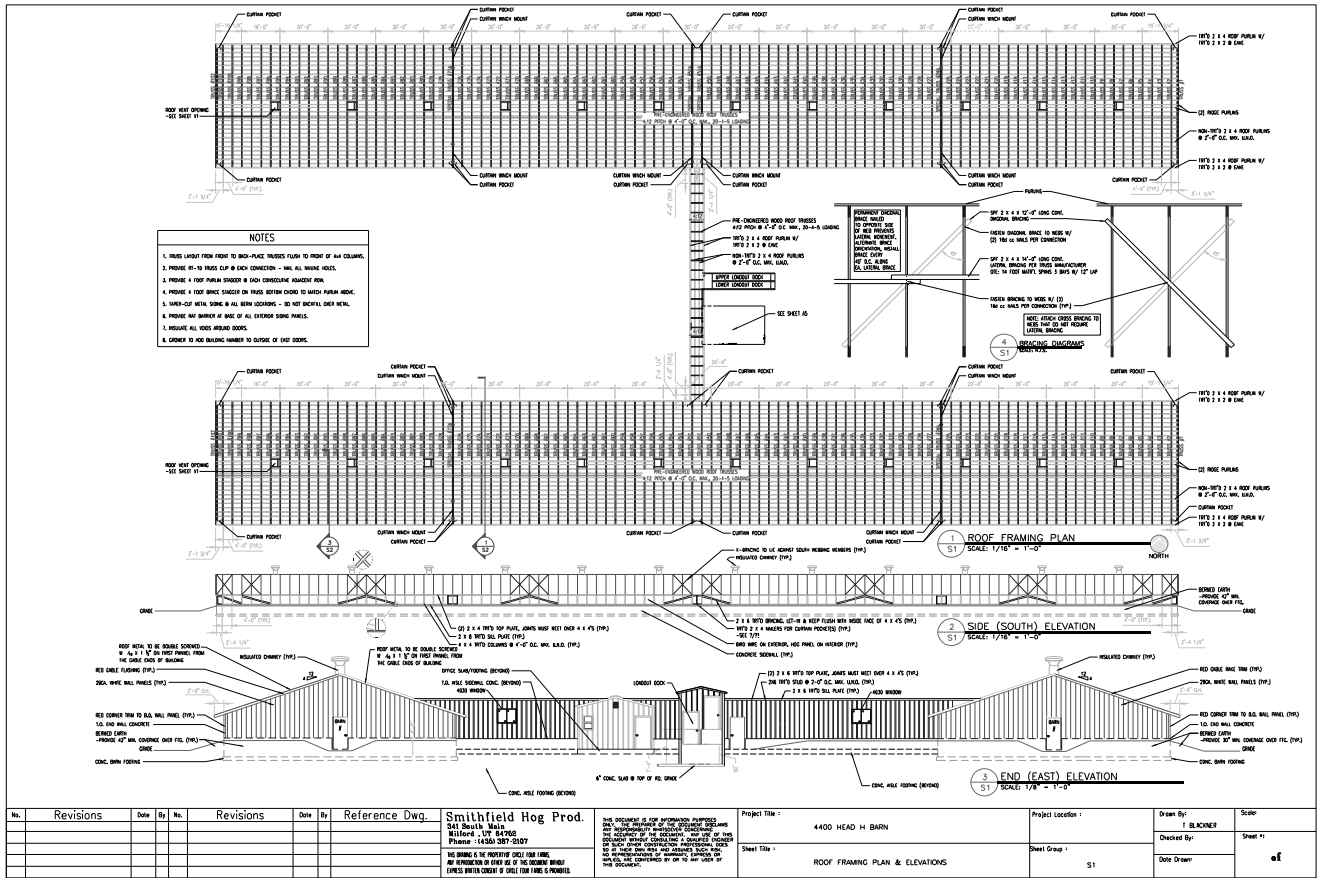


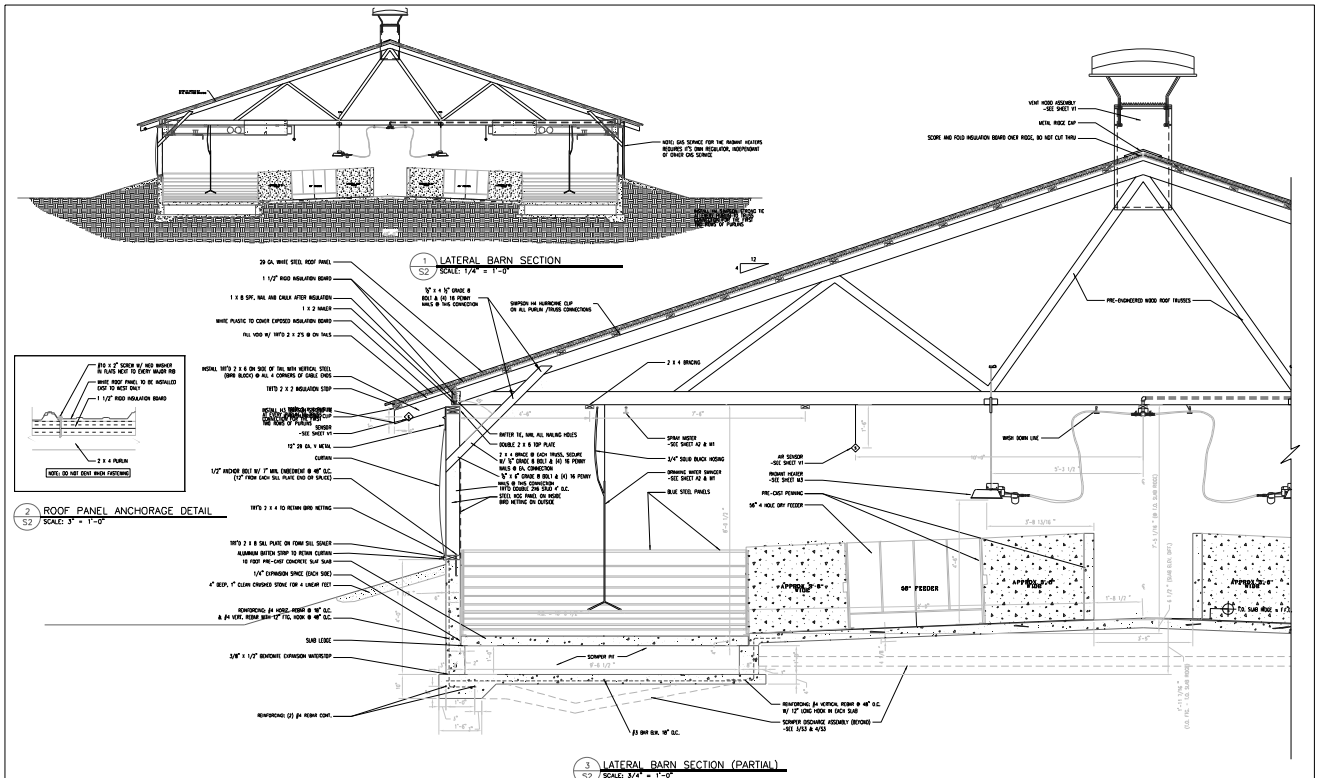
Figure 21. Relation of water level in selected wells in central Sevier Valley to annual discharge of the Sevier River at Hatch, Utah, to cumulative departure from average annual precipitation at Richfield Radio KVSC, to annual withdrawal from wells, and to concentration of dissolved solids in water from well (C-23-2)15dcb-4.—Continued

Attachment C – Project Documents



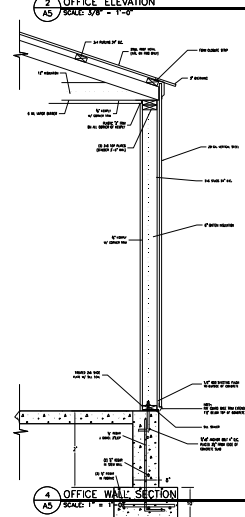
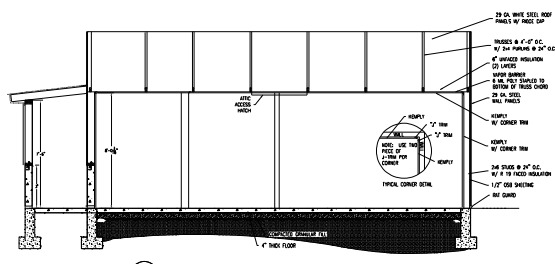
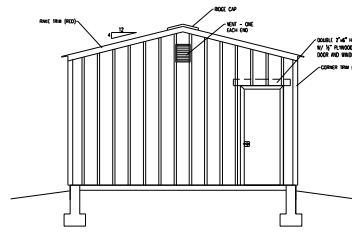
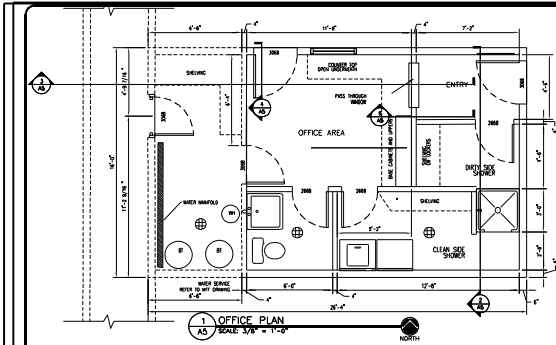
No.	Revisions	Date	By	No.	Revisions	Date	By	Reference Dwg.

Smithfield Hog Prod. 541 South Main Millfield, VA 24120 Phone: (434) 537-0227 FAX: (434) 537-0227 NO BRACING IN ROOF OR ON THE EXTERIOR BRACE UNLESS SPECIFICALLY NOTED OTHERWISE.	THIS DOCUMENT IS FOR INFORMATION PURPOSES ONLY. THE USER OF THIS DOCUMENT ASSUMES ALL LIABILITY OF THE OCCUPANT AND USER OF THE INFORMATION CONTAINED HEREIN. THE USER OF THIS DOCUMENT ASSUMES ALL LIABILITY OF THE OCCUPANT AND USER OF THE INFORMATION CONTAINED HEREIN. THE USER OF THIS DOCUMENT ASSUMES ALL LIABILITY OF THE OCCUPANT AND USER OF THE INFORMATION CONTAINED HEREIN.	Project Title : 4400 HEAD H BARN	Project Location : S1	Drawn By : E. SILKINER	Sheet #:
Sheet Title : ROOF FRAMING PLAN & ELEVATIONS	Sheet Group : S1	Checked By :	Date Drawn :	Scale :	of

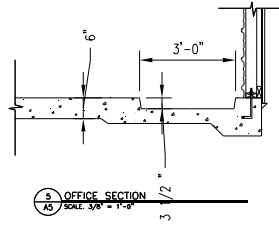


No.	Revisions	Date	By	No.	Revisions	Date	By	Reference Dwg.

Smithfield Hog Prod. 5455 South 10th Willard, UT 84202 Phone: (435) 337-3327 NO CHANGE IS TO BE MADE TO THIS DRAWING WITHOUT THE WRITTEN CONSENT OF THE ARCHITECT.		Project Title : 4400 HEAD H. BARN		Project Location : 52		Drawn By : I. BLANCHER		Sheet No : of	
SHEET TITLE : LATERAL SECTIONS & DETAILS		SHEET GROUP : 52		Checked By :		Date Drawn :			



- SPECIFICATIONS: CONCRETE CONTRACTOR**
- CONCRETE CONTRACTOR TO BE RESPONSIBLE FOR ALL CONCRETE WORK.
 - CONCRETE CONTRACTOR TO BE RESPONSIBLE FOR ALL FORMWORK AND SHORING.
 - CONCRETE CONTRACTOR TO BE RESPONSIBLE FOR ALL REINFORCEMENT.
 - CONCRETE CONTRACTOR TO BE RESPONSIBLE FOR ALL CURING.
 - CONCRETE CONTRACTOR TO BE RESPONSIBLE FOR ALL FINISHES.
 - CONCRETE CONTRACTOR TO BE RESPONSIBLE FOR ALL PROTECTION.
- SPECIFICATIONS: PLUMBING CONTRACTOR**
- PLUMBING CONTRACTOR TO BE RESPONSIBLE FOR ALL PLUMBING WORK.
 - PLUMBING CONTRACTOR TO BE RESPONSIBLE FOR ALL FIXTURES.
 - PLUMBING CONTRACTOR TO BE RESPONSIBLE FOR ALL PIPING.
 - PLUMBING CONTRACTOR TO BE RESPONSIBLE FOR ALL VENTING.
 - PLUMBING CONTRACTOR TO BE RESPONSIBLE FOR ALL TESTING.
 - PLUMBING CONTRACTOR TO BE RESPONSIBLE FOR ALL PROTECTION.
- SPECIFICATIONS: ELECTRICAL CONTRACTOR**
- ELECTRICAL CONTRACTOR TO BE RESPONSIBLE FOR ALL ELECTRICAL WORK.
 - ELECTRICAL CONTRACTOR TO BE RESPONSIBLE FOR ALL WIRING.
 - ELECTRICAL CONTRACTOR TO BE RESPONSIBLE FOR ALL PANELS.
 - ELECTRICAL CONTRACTOR TO BE RESPONSIBLE FOR ALL DEVICES.
 - ELECTRICAL CONTRACTOR TO BE RESPONSIBLE FOR ALL TESTING.
 - ELECTRICAL CONTRACTOR TO BE RESPONSIBLE FOR ALL PROTECTION.
- SPECIFICATIONS: FRAMING CONTRACTOR**
- FRAMING CONTRACTOR TO BE RESPONSIBLE FOR ALL FRAMING WORK.
 - FRAMING CONTRACTOR TO BE RESPONSIBLE FOR ALL JOISTS.
 - FRAMING CONTRACTOR TO BE RESPONSIBLE FOR ALL RAFTERS.
 - FRAMING CONTRACTOR TO BE RESPONSIBLE FOR ALL TRUSSES.
 - FRAMING CONTRACTOR TO BE RESPONSIBLE FOR ALL BRACING.
 - FRAMING CONTRACTOR TO BE RESPONSIBLE FOR ALL PROTECTION.



No.	Revisions	Date	By	No.	Revisions	Date	By	Reference Dwg.

Smithfield Hog Prod.
4400 HEAD H. BARN
SMITHFIELD, VT 05470
PHONE: 1-855-587-5397

Project Title: 4400 HEAD H. BARN
Sheet Title: OFFICE PLAN, ELEVATIONS, AND DETAILS

Project Location: A4
Drawn By: J. BLACKNER
Checked By: [Blank]
Scale: 3/8" = 1'-0"

Sheet #: 6 of 6

MATERIAL LIST			
NO.	SYMBOL	DESCRIPTION	APPROX QTY
1	CONV	ALL SYMBOLS ARE TO BE USED FOR SCHEDULES UNLESS OTHERWISE NOTED. ELECTRICAL CONTRACTOR SHALL FURNISH FOR CONTRACTOR'S USE. CONTRACTOR SHALL VERIFY ALL MATERIALS ARE AS SHOWN IN THE SCHEDULES AND APPROVED BY THE ARCHITECT. CONTRACTOR SHALL VERIFY ALL MATERIALS ARE AS SHOWN IN THE SCHEDULES AND APPROVED BY THE ARCHITECT. CONTRACTOR SHALL VERIFY ALL MATERIALS ARE AS SHOWN IN THE SCHEDULES AND APPROVED BY THE ARCHITECT.	
2	0	NO. 1000 SHALL BE USED FOR SCHEDULES UNLESS OTHERWISE NOTED. ELECTRICAL CONTRACTOR SHALL FURNISH FOR CONTRACTOR'S USE.	SCHEDULE 0 00
3	0	NO. 1000 SHALL BE USED FOR SCHEDULES UNLESS OTHERWISE NOTED. ELECTRICAL CONTRACTOR SHALL FURNISH FOR CONTRACTOR'S USE.	SCHEDULE 0 00
4	SP	SPRING ROCKET INSULATED CONDUIT, SINGLE POLE, SINGLE THROAT. SCHEDULE 0000 SHALL BE USED FOR SCHEDULES UNLESS OTHERWISE NOTED. ELECTRICAL CONTRACTOR SHALL FURNISH FOR CONTRACTOR'S USE.	CONV CONV CONV CONV
5	SS	SPRING ROCKET INSULATED CONDUIT, SINGLE POLE, SINGLE THROAT. SCHEDULE 0000 SHALL BE USED FOR SCHEDULES UNLESS OTHERWISE NOTED. ELECTRICAL CONTRACTOR SHALL FURNISH FOR CONTRACTOR'S USE.	CONV CONV CONV CONV
6	0	NO. 1000 SHALL BE USED FOR SCHEDULES UNLESS OTHERWISE NOTED. ELECTRICAL CONTRACTOR SHALL FURNISH FOR CONTRACTOR'S USE.	SCHEDULE 0 00
7	0	NO. 1000 SHALL BE USED FOR SCHEDULES UNLESS OTHERWISE NOTED. ELECTRICAL CONTRACTOR SHALL FURNISH FOR CONTRACTOR'S USE.	SCHEDULE 0 00
8	0	NO. 1000 SHALL BE USED FOR SCHEDULES UNLESS OTHERWISE NOTED. ELECTRICAL CONTRACTOR SHALL FURNISH FOR CONTRACTOR'S USE.	SCHEDULE 0 00
9	0	NO. 1000 SHALL BE USED FOR SCHEDULES UNLESS OTHERWISE NOTED. ELECTRICAL CONTRACTOR SHALL FURNISH FOR CONTRACTOR'S USE.	SCHEDULE 0 00
10	0	NO. 1000 SHALL BE USED FOR SCHEDULES UNLESS OTHERWISE NOTED. ELECTRICAL CONTRACTOR SHALL FURNISH FOR CONTRACTOR'S USE.	SCHEDULE 0 00
11	0	NO. 1000 SHALL BE USED FOR SCHEDULES UNLESS OTHERWISE NOTED. ELECTRICAL CONTRACTOR SHALL FURNISH FOR CONTRACTOR'S USE.	SCHEDULE 0 00
12	0	NO. 1000 SHALL BE USED FOR SCHEDULES UNLESS OTHERWISE NOTED. ELECTRICAL CONTRACTOR SHALL FURNISH FOR CONTRACTOR'S USE.	SCHEDULE 0 00
13	0	NO. 1000 SHALL BE USED FOR SCHEDULES UNLESS OTHERWISE NOTED. ELECTRICAL CONTRACTOR SHALL FURNISH FOR CONTRACTOR'S USE.	SCHEDULE 0 00
14	0	NO. 1000 SHALL BE USED FOR SCHEDULES UNLESS OTHERWISE NOTED. ELECTRICAL CONTRACTOR SHALL FURNISH FOR CONTRACTOR'S USE.	SCHEDULE 0 00
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18	0	NO. 1000 SHALL BE USED FOR SCHEDULES UNLESS OTHERWISE NOTED. ELECTRICAL CONTRACTOR SHALL FURNISH FOR CONTRACTOR'S USE.	SCHEDULE 0 00
19	0	NO. 1000 SHALL BE USED FOR SCHEDULES UNLESS OTHERWISE NOTED. ELECTRICAL CONTRACTOR SHALL FURNISH FOR CONTRACTOR'S USE.	SCHEDULE 0 00
20	0	NO. 1000 SHALL BE USED FOR SCHEDULES UNLESS OTHERWISE NOTED. ELECTRICAL CONTRACTOR SHALL FURNISH FOR CONTRACTOR'S USE.	SCHEDULE 0 00
21	0	NO. 1000 SHALL BE USED FOR SCHEDULES UNLESS OTHERWISE NOTED. ELECTRICAL CONTRACTOR SHALL FURNISH FOR CONTRACTOR'S USE.	SCHEDULE 0 00

ELECTRICAL INSTALLATION KEY	
1.	1/4" (Ø) INDICATES DETAIL NUMBER/SHEET NUMBER.
2.	1/4" (Ø) INDICATES DETAIL NUMBER/SHEET NUMBER.
3.	1/4" (Ø) INDICATES DETAIL NUMBER/SHEET NUMBER.
4.	1/4" (Ø) INDICATES DETAIL NUMBER/SHEET NUMBER.
5.	1/4" (Ø) INDICATES DETAIL NUMBER/SHEET NUMBER.
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16.	1/4" (Ø) INDICATES DETAIL NUMBER/SHEET NUMBER.
17.	1/4" (Ø) INDICATES DETAIL NUMBER/SHEET NUMBER.
18.	1/4" (Ø) INDICATES DETAIL NUMBER/SHEET NUMBER.
19.	1/4" (Ø) INDICATES DETAIL NUMBER/SHEET NUMBER.
20.	1/4" (Ø) INDICATES DETAIL NUMBER/SHEET NUMBER.
21.	1/4" (Ø) INDICATES DETAIL NUMBER/SHEET NUMBER.

SPECIFICATIONS - ELECTRICAL	
1.	PROVIDE ALL MATERIALS AND LABOR AS REQUIRED FOR A COMPLETE INSTALLATION.
2.	PROVIDE UNDERGROUND SERVICE FROM STREET TO SERVICE AT 200 AMP PANEL.
3.	PROVIDE ALL LEADS INCLUDING CABLES, TRAYS, TUBES, CONDUITS, AND ALL OTHER ITEMS NECESSARY FOR A COMPLETE INSTALLATION.
4.	PROVIDE ALL LEADS INCLUDING CABLES, TRAYS, TUBES, CONDUITS, AND ALL OTHER ITEMS NECESSARY FOR A COMPLETE INSTALLATION.
5.	PROVIDE ALL LEADS INCLUDING CABLES, TRAYS, TUBES, CONDUITS, AND ALL OTHER ITEMS NECESSARY FOR A COMPLETE INSTALLATION.
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9.	PROVIDE ALL LEADS INCLUDING CABLES, TRAYS, TUBES, CONDUITS, AND ALL OTHER ITEMS NECESSARY FOR A COMPLETE INSTALLATION.
10.	PROVIDE ALL LEADS INCLUDING CABLES, TRAYS, TUBES, CONDUITS, AND ALL OTHER ITEMS NECESSARY FOR A COMPLETE INSTALLATION.

LIGHTING FIXTURE SCHEDULE			
SYMBOL	DESCRIPTION	QTY	REMARKS
1	INDOOR	100	COMPLETE WITH DIMMER
2	OUTDOOR	50	COMPLETE WITH DIMMER

ELECTRICAL INSTALLATION NOTES	
1.	ELECTRICAL CONTRACTOR SHALL VERIFY ALL MATERIALS ARE AS SHOWN IN THE SCHEDULES AND APPROVED BY THE ARCHITECT.
2.	ELECTRICAL CONTRACTOR SHALL VERIFY ALL MATERIALS ARE AS SHOWN IN THE SCHEDULES AND APPROVED BY THE ARCHITECT.
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No.	Revisions	Date	By	No.	Revisions	Date	By	Reference Dwg.
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20				20				
21				21				

Smithfield Hog Prod.
4400 HEAD H BARN

Project Title
4400 HEAD H BARN

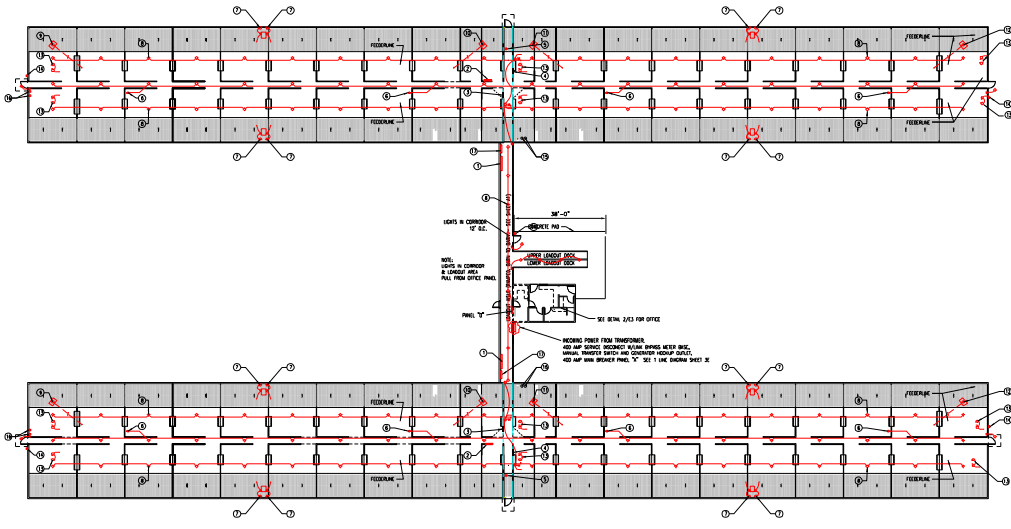
Project Location
Sheet No. 1 of 1

Drawn By
T. BLACKNER

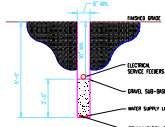
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Date Drawn

Sheet No. 1 of 1

SCHEDULES & NOTES



1 ELECTRICAL FLOOR PLAN
SCALE: 1/16" = 1'-0"



2 UTILITY TRENCH DRAIN (TYPICAL)
SCALE: 1/2" = 1'-0"

- #### PLAN DETAIL NOTES
- 1. USE CONTRACTOR DRAWING PANEL # 407/7. SEE SHEET #1.
 - 2. USE CONDUIT PANEL.
 - 3. HOOK UNDER RACEWAY FOR CONDUIT SWITCH. USE 1/2" X 1/2" CONDUIT OVER THE TOP OF RACEWAY FOR 1'-0" CLEARANCE FROM THE FULL CODE UNDER RACEWAY. THESE MUST BE 0-6 SECONDS.
 - 4. USE CONDUIT UNDER FOR CONDUIT SWITCH. USE 1/2" X 1/2" CONDUIT OVER THE TOP OF RACEWAY FOR 1'-0" CLEARANCE FROM THE FULL CODE UNDER RACEWAY. THESE MUST BE 0-6 SECONDS.
 - 5. CONDUIT MUST BE 1/2" ABOVE RACEWAY.
 - 6. 1/2" X 1/2" CONDUIT UNDER RACEWAY.
 - 7. CONDUIT MUST BE 1/2" ABOVE RACEWAY.
 - 8. CONDUIT MUST BE 1/2" ABOVE RACEWAY.
 - 9. CONDUIT MUST BE 1/2" ABOVE RACEWAY.
 - 10. CONDUIT MUST BE 1/2" ABOVE RACEWAY.
 - 11. CONDUIT MUST BE 1/2" ABOVE RACEWAY.
 - 12. CONDUIT MUST BE 1/2" ABOVE RACEWAY.
 - 13. CONDUIT MUST BE 1/2" ABOVE RACEWAY.

- #### ELECTRICAL SHEET
1. ALL SERVICES ARE TO BE LOCATED ALONG THE CENTER LINE OF THE MAIN & BARN TRAILS IN FROM THE MAIN TO THE BARN. SEE SHEET #1 FOR DETAILS. THE SERVICE TRAILS ARE BEHIND THE SERVICES THAT BE INSTALLED 3" FROM LEADING FEATURES.
 2. SERVICES ARE TO BE MADE BY THE MAIN. SERVICE TRAILS AND FEEDERS ARE TO BE MADE BY THE MAIN. SERVICE TRAILS AND FEEDERS ARE TO BE MADE BY THE MAIN. SERVICE TRAILS AND FEEDERS ARE TO BE MADE BY THE MAIN.
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- #### SITE ELECTRICAL NOTES
1. NORTH ORIENTATION OF SHEET AND BUILDING NUMBERING ARE SHOWN.
 2. SEE SHEET #1 FOR THE SITE REQUIREMENTS.
 3. SEE SHEET #1 FOR THE SITE REQUIREMENTS.
 4. ALL SERVICE TRAILS ARE TO BE MADE BY THE MAIN. SERVICE TRAILS AND FEEDERS ARE TO BE MADE BY THE MAIN. SERVICE TRAILS AND FEEDERS ARE TO BE MADE BY THE MAIN.
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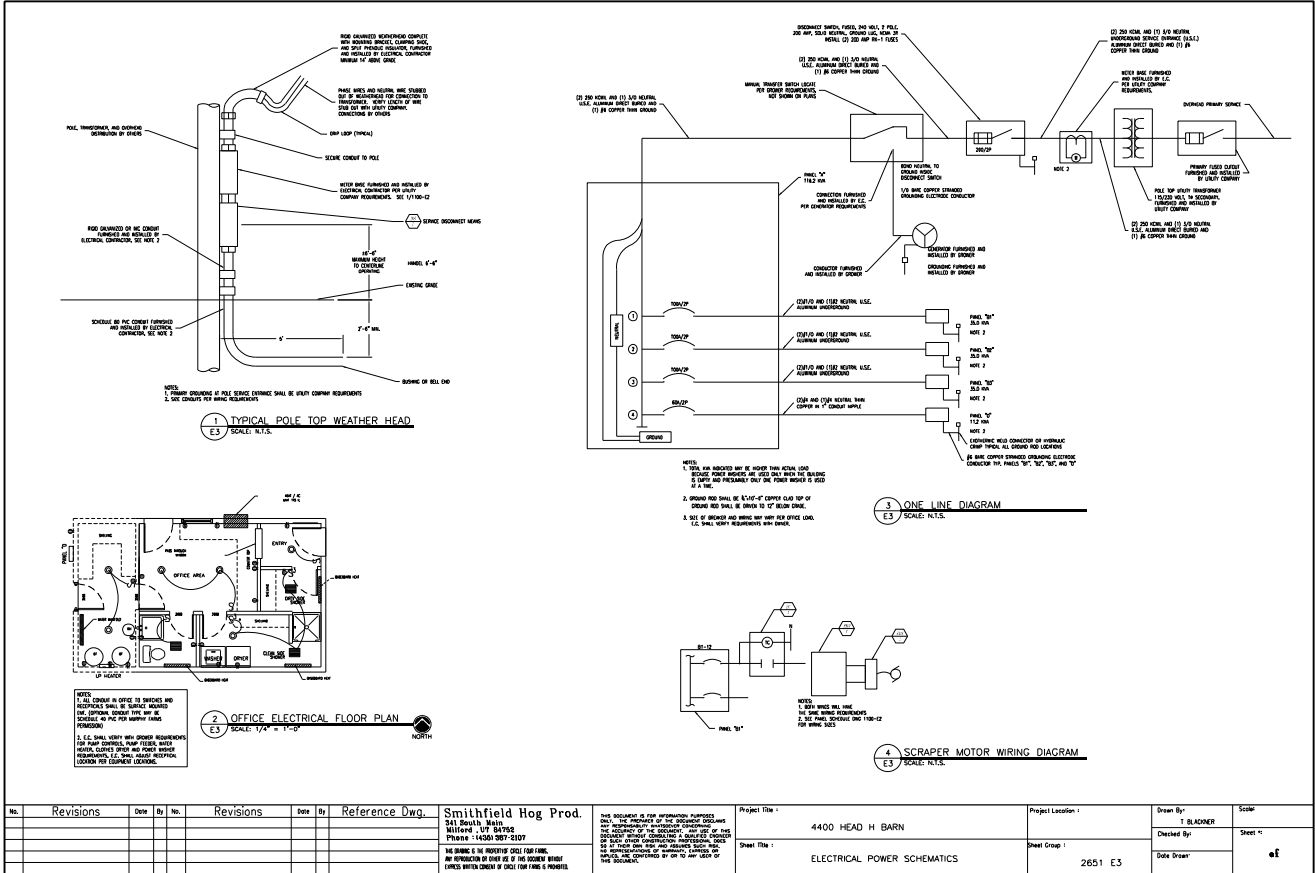
No.	Revisions	Date	By	No.	Revisions	Date	By	Reference Dwg.

Smithfield Hog Prod.
341 South Main
Hilltown, VT 05745
Phone: (435) 887-2107

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Project Title: 4400 HEAD H BARN
Sheet Title: ELECTRICAL FLOOR PLAN
Project Location: [Blank]Sheet Group: E2

Drawn By: T BLACKNER
Checked By: [Blank]
Date Drawn: [Blank]
Scale: 1/16" = 1'-0"



No.	Revisions	Date	By	No.	Revisions	Date	By	Reference Dwg.

Smithfield Hog Prod. 341 South Main Milford, VT 05546 Phone - (435) 387-3107 THE DRAWING IS THE PROPERTY OF THE DRAWER. ANY REPRODUCTION OR USE AS IS OF THIS DRAWING WITHOUT EXPRESS WRITTEN CONSENT OF THE DRAWER IS PROHIBITED.	THIS DRAWING IS FOR INFORMATION PURPOSES ONLY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM ALL AFFECTED AGENCIES AND AUTHORITIES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM ALL AFFECTED AGENCIES AND AUTHORITIES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM ALL AFFECTED AGENCIES AND AUTHORITIES.	Project File : 4400 HEAD H BARN Sheet Title : ELECTRICAL POWER SCHEMATICS	Project Location : Sheet Group : 2651 E3	Drawn By : J BLANMER Checked By : Date Drawn :	Scale : Sheet 11 of 11
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1 to Name: Pincan 1200 Head Size 8.1 XL7

Expansion box #

Barn 1

Note: Cards in box will likely need to be moved to match this layout

Type of expansion box:

6 slots

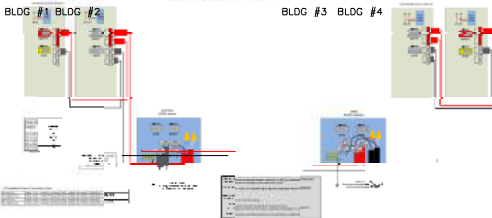
6 slots

6 slots

6 slots

Slot #1		Slot #2		Slot #3		Slot #4		Slot #5		Slot #6	
4 inputs, 8 relay (ALLSAFE) (Type #1)		4 inputs, 8 relay (ALLSAFE) (Type #1)		4 inputs, 8 relay (ALLSAFE) (Type #1)		4 inputs, 8 relay (Type #1)		4 inputs, 8 relay (Type #1)		4 inputs, 8 relay (Type #1)	
R 1 NC	Room Probe 1 (N1)	R 1 NC	Room Probe 2 (N1)	R 1 NC	Room Probe 3 (N1)	R 1 NC	Room Probe 4 (N1)	R 1 NC	Room Probe 5 (N1)	R 1 NC	Room Probe 6 (N1)
R 1 NO	Room Probe 1 (N1)	R 1 NO	Room Probe 2 (N1)	R 1 NO	Room Probe 3 (N1)	R 1 NO	Room Probe 4 (N1)	R 1 NO	Room Probe 5 (N1)	R 1 NO	Room Probe 6 (N1)
R 1 COM	Room Probe 1 (N1)	R 1 COM	Room Probe 2 (N1)	R 1 COM	Room Probe 3 (N1)	R 1 COM	Room Probe 4 (N1)	R 1 COM	Room Probe 5 (N1)	R 1 COM	Room Probe 6 (N1)
R 2 NC	Room Probe 1 (N1)	R 2 NC	Room Probe 2 (N1)	R 2 NC	Room Probe 3 (N1)	R 2 NC	Room Probe 4 (N1)	R 2 NC	Room Probe 5 (N1)	R 2 NC	Room Probe 6 (N1)
R 2 NO	Room Probe 1 (N1)	R 2 NO	Room Probe 2 (N1)	R 2 NO	Room Probe 3 (N1)	R 2 NO	Room Probe 4 (N1)	R 2 NO	Room Probe 5 (N1)	R 2 NO	Room Probe 6 (N1)
R 2 COM	Room Probe 1 (N1)	R 2 COM	Room Probe 2 (N1)	R 2 COM	Room Probe 3 (N1)	R 2 COM	Room Probe 4 (N1)	R 2 COM	Room Probe 5 (N1)	R 2 COM	Room Probe 6 (N1)
R 3 NC	Room Probe 1 (N1)	R 3 NC	Room Probe 2 (N1)	R 3 NC	Room Probe 3 (N1)	R 3 NC	Room Probe 4 (N1)	R 3 NC	Room Probe 5 (N1)	R 3 NC	Room Probe 6 (N1)
R 3 NO	Room Probe 1 (N1)	R 3 NO	Room Probe 2 (N1)	R 3 NO	Room Probe 3 (N1)	R 3 NO	Room Probe 4 (N1)	R 3 NO	Room Probe 5 (N1)	R 3 NO	Room Probe 6 (N1)
R 3 COM	Room Probe 1 (N1)	R 3 COM	Room Probe 2 (N1)	R 3 COM	Room Probe 3 (N1)	R 3 COM	Room Probe 4 (N1)	R 3 COM	Room Probe 5 (N1)	R 3 COM	Room Probe 6 (N1)
R 4 NC	Room Probe 1 (N1)	R 4 NC	Room Probe 2 (N1)	R 4 NC	Room Probe 3 (N1)	R 4 NC	Room Probe 4 (N1)	R 4 NC	Room Probe 5 (N1)	R 4 NC	Room Probe 6 (N1)
R 4 NO	Room Probe 1 (N1)	R 4 NO	Room Probe 2 (N1)	R 4 NO	Room Probe 3 (N1)	R 4 NO	Room Probe 4 (N1)	R 4 NO	Room Probe 5 (N1)	R 4 NO	Room Probe 6 (N1)
R 4 COM	Room Probe 1 (N1)	R 4 COM	Room Probe 2 (N1)	R 4 COM	Room Probe 3 (N1)	R 4 COM	Room Probe 4 (N1)	R 4 COM	Room Probe 5 (N1)	R 4 COM	Room Probe 6 (N1)
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R 6 COM	Room Probe 1 (N1)	R 6 COM	Room Probe 2 (N1)	R 6 COM	Room Probe 3 (N1)	R 6 COM	Room Probe 4 (N1)	R 6 COM	Room Probe 5 (N1)	R 6 COM	Room Probe 6 (N1)

1 EXPANSION BOX SCH. (TYP. EACH BARN)
E4 SCALE: N.T.S.

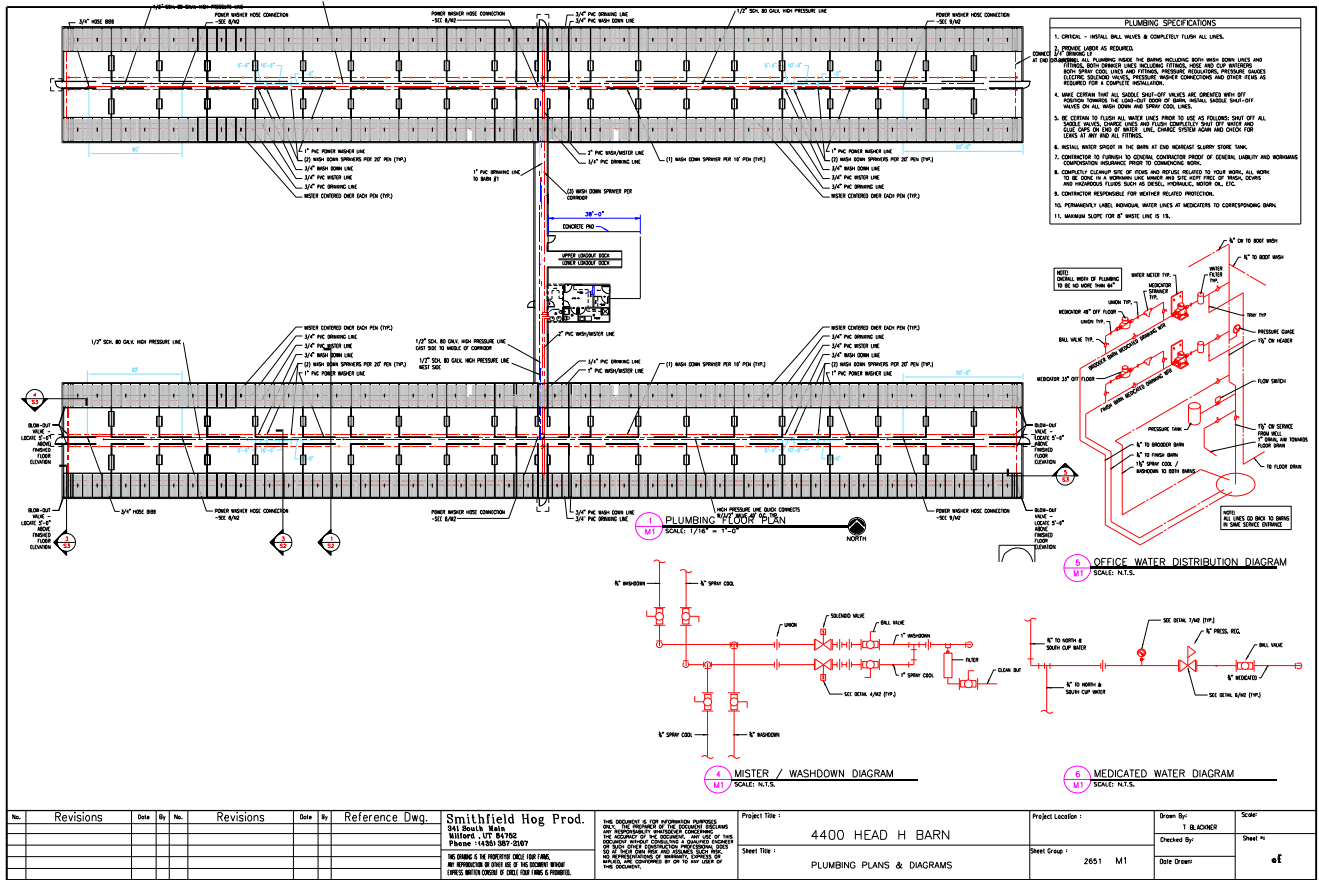


3 RS 42 CABLE WIRE DIAGRAM
E4 SCALE: N.T.S.

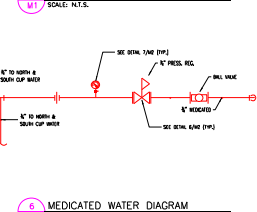
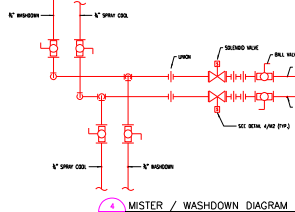
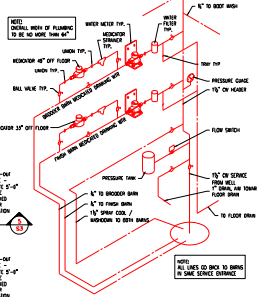
PANEL "B1" TYP.		120/208 VAC 3PH 4W.												
SURFACE MOUNT		225A - M												
CKT NO.	LOAD V.A. TYPE	LOAD DESCRIPTION	WIRE SIZE	PH	AMP SIZE	P	AMP SIZE	P	AMP SIZE	P	LOAD DESCRIPTION	LOAD TYPE	LOAD V.A.	CKT NO.
3	450 LTS	FEED MOTORS	12 1 20	A	20	1	12	1	12	1	LIGHTS ROOM #2	LTS	450	4
5	500	FEED MOTORS	12 1 15	C	-	-	-	-	-	-	SPARE	-	-	6
7	850 MTR	SOUTH LINE	12 2 15	B	15	2	12	2	12	2	2-24" FANS	MTR	850	8
9	850	FEED MOTORS	-	-	-	-	-	-	-	-	ROOM #2	-	-	10
11	850 MTR	INCOMING	12 2 15	C	15	2	12	2	12	2	ROOM #2	MTR	850	12
12	850	SW & SM CURTAIN MACHINE	-	-	-	-	-	-	-	-	ROOM #2	-	-	14
15	700 MTR	2-18" FANS	12 2 15	B	15	2	12	2	12	2	2-18" FANS	MTR	700	16
17	700	ROOM #1	-	-	-	-	-	-	-	-	ROOM #1 & #2	-	-	18
19	1000 MTR	HEATER ROOM #1	12 1 20	A	20	1	12	1	12	1	HEATER ROOM #2	MTR	1000	20
21	1000 MTR	PRE-HEAT HEATERS #1	12 1 15	B	20	1	12	1	12	1	HALLWAY CONV. RECEPT.	RECP	1000	22
23	1000 MTR	PRE-HEAT HEATER #2	12 1 15	C	20	1	12	1	12	1	PRE-HEAT LIGHTS	LTS	1000	24
25	-	SPARE	1 20	A	20	1	12	1	12	1	SPARE	-	-	26
27	1100 LTS	HEAT LAMPS RM-#1	12 1 20	B	20	1	12	1	12	1	HEAT LAMPS RM-#1	LTS	1100	28
29	1100 LTS	HEAT LAMPS RM-#2	12 1 20	C	20	1	12	1	12	1	HEAT LAMPS RM-#2	LTS	1100	30
31	1100 LTS	HEAT LAMPS RM-#3	12 1 20	A	20	1	12	1	12	1	HEAT LAMPS RM-#3	LTS	1100	32
33	1100 LTS	HEAT LAMPS RM-#4	12 1 20	B	20	1	12	1	12	1	HEAT LAMPS RM-#4	LTS	1100	34
35	1100 LTS	HEAT LAMPS RM-#5	12 1 20	C	20	1	12	1	12	1	HEAT LAMPS RM-#5	LTS	1100	36
37	1100 LTS	HEAT LAMPS RM-#6	12 1 20	A	20	1	12	1	12	1	HEAT LAMPS RM-#6	LTS	1100	38
39	-	SPARE	1 20	B	20	1	-	-	-	-	SPARE	-	-	40
41	-	SPARE	1 20	C	20	1	-	-	-	-	SPARE	-	-	42

2 ELEC. BREAKER PANEL (TYP.)
E4 SCALE: N.T.S.

No.	Revisions	Date	By	No.	Revisions	Date	By	Reference Dwg.	Smithfield Hog Prod. 341 South Main Bluffton, UT 84303 Phone (435) 587-2187 NO DRAWING IS TO BE REPRODUCED OR COPIED WITHOUT THE WRITTEN CONSENT OF THE DESIGNER. DESIGN CHANGES SHALL BE MADE BY THE DESIGNER.	Project Title: 4400 HEAD H BARN	Project Location: 2651 E4	Drawn By: 1 BLANCHER	Scale: Sheet # of
									NO DRAWING IS TO BE REPRODUCED OR COPIED WITHOUT THE WRITTEN CONSENT OF THE DESIGNER. DESIGN CHANGES SHALL BE MADE BY THE DESIGNER.	Sheet Title: PANEL SCH & RELAY ASSIGN.	Sheet Group: 2651 E4	Date Drawn:	



- PLUMBING SPECIFICATIONS**
1. CRITICAL - METAL BALL VALVES & COMPLETELY FLUSH ALL LINES.
 2. REMOVE AIRS AS REQUIRED.
 3. VERIFY ALL PLUMBING WORK IS BEING INSTALLED WITH CORRECT JOINTS AND FITTINGS. ALL PLUMBING WORK SHALL BE INSTALLED TO THE APPROVED PLUMBING SPECIFICATIONS. ALL PLUMBING WORK SHALL BE INSTALLED TO THE APPROVED PLUMBING SPECIFICATIONS. ALL PLUMBING WORK SHALL BE INSTALLED TO THE APPROVED PLUMBING SPECIFICATIONS.
 4. MAKE CERTAIN THAT ALL SCHEDULE 40S-45S VALVES ARE OPERATED WITH OFF-POSITION TOWARD THE DOWN-SIDE OF BARN. METAL BALL VALVE SHUT-OFF MUST BE ON THE DOWN-SIDE OF BARN.
 5. BE CERTAIN TO FLUSH ALL WATER LINES PRIOR TO USE AS FOLLOWS: SHUT OFF ALL WATER SUPPLY, OPEN LINES AND FLUSH COMPLETELY SHUT OFF WATER AND LEAVE OPEN FOR 24 HOURS. LINE CHANGE SYSTEM AGAIN AND CHECK FOR LEAKS AT ALL JOINTS AND FITTINGS.
 6. METAL WELDS SHALL BE IN THE BARN AT THE WELDED JOINTS.
 7. CONTRACTOR TO PROVIDE TO COVER CONNECTIONS OF GENERAL LIABILITY AND WORKING COMPENSATION INSURANCE PRIOR TO COMMENCING WORK.
 8. CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND INSURANCE COVERAGE PRIOR TO COMMENCING WORK.
 9. CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND INSURANCE COVERAGE PRIOR TO COMMENCING WORK.
 10. CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND INSURANCE COVERAGE PRIOR TO COMMENCING WORK.
 11. MAINTAIN SLOPE FOR 1/8" WATER LINE & 1/4"



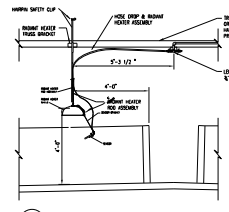
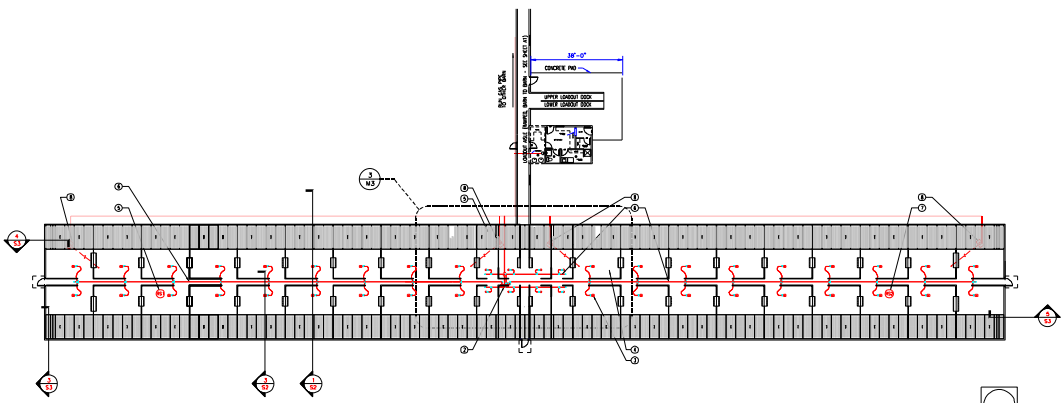
No.	Revisions	Date	By	No.	Revisions	Date	By	Reference Dwg.

Smithfield Hog Prod. 341 South Main Williston, VT 05495 PHONE: 1-800-557-2207 THE DRAWING IS THE PROPERTY OF THE DRAWER. ANY REPRODUCTION OR USE OF THIS DRAWING WITHOUT THE WRITTEN CONSENT OF THE DRAWER IS PROHIBITED.	Project Title: 4400 HEAD H BARN Sheet Title: PLUMBING PLANS & DIAGRAMS	Project Location: Sheet Group: 2651 M1	Drawn By: T. BLADNER Checked By: Date Drawn: 01	Scale: as shown
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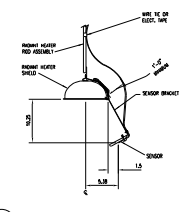
- PLAN DETAIL NOTES**
1. GAS HEATER REGULATOR PANEL, GAS PANEL SHALL ALWAYS BE PLACED OUT TO THE EXTERIOR CORNER PANEL. - SEE SHEET M1
 2. HEATER HEATER (ELDER) SHALL HAVE MINIMUM 2" (4" FOR 2" PIPING) TO THE CEILING JAMB.
 3. HEATER HEATER (ELDER) SHALL BE SUPPORTED BY SINGLE AUTO/WALLMOUNT REGULATOR ON GAS PANEL, 2" (4" FOR 2" PIPING).
 4. HEATER HEATER GAS SERVICE ENTRANCE, 3/4" BLACK IRON PIPE, HAS TO BE SERVICE ENTRANCE & REGULATOR SHALL BE MOUNTED FROM THE REGULATOR FOR ALL SHIP GAS IMPURITIES WITH THE REGULATOR. - SEE DETAIL V-02
 5. USE ONE HEATER PIPE, 3/4" BLACK IRON PIPE.
 6. HEATER BRACKET (HEATER) SHALL BE MOUNTED 4" AWAY FROM PIPING TO THE CEILING JAMB.
 7. HEATER HEATER
 8. HEATER HEATER

ATTENTION

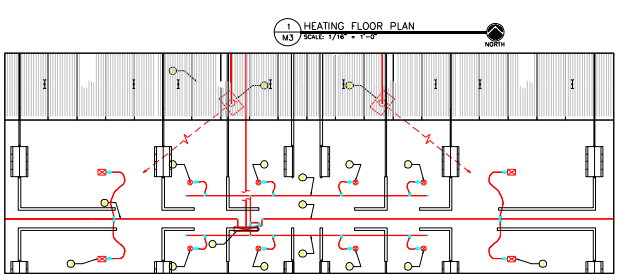
THE DRAWING IS PROVIDED AS AN EXAMPLE OF HOW TO CONSTRUCT THE GAS HEATER SYSTEM. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY THE EXISTING CONDITIONS AND TO OBTAIN ALL NECESSARY PERMITS AND APPROVALS FROM THE LOCAL AUTHORITY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CORRECTNESS OF THE INFORMATION PROVIDED IN THIS DRAWING. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CORRECTNESS OF THE INFORMATION PROVIDED IN THIS DRAWING.



2 RADIANT HEATER ASSEMBLY
SCALE: N.T.S.



3 RADIANT HEATER SENSOR ASSEMBLY
SCALE: N.T.S.



1 BROODER BARN HEATING PLAN
SCALE: 3/16" = 1'-0"
NORTH

No.	Revisions	Date	By	No.	Revisions	Date	By	Reference Dwg.

Smithfield Hog Prod. 5415 South 54th Bluffton, UT 84715 Phone: 435-587-0327	This document is for informational purposes only. The contractor shall verify the accuracy of the information provided and shall be responsible for the accuracy of the information provided. The contractor shall be responsible for the accuracy of the information provided.	Project Title : 4400 HEAD H BARN	Project Location :	Drawn By : T. BLACKNER	Scale :
HIS DRAWING IS THE PROPERTY OF THE DRAWING ENGINEER. NO REVISIONS OR CHANGES TO THIS DRAWING SHALL BE MADE WITHOUT THE WRITTEN CONSENT OF THE DRAWING ENGINEER.	Sheet Title : HEATING FLOOR PLANS & DIAGRAMS	Sheet Group : M3	Date Drawn :	Checked By :	Sheet # : of

Attachment D – Specifications and QA / QC for HDPE Liners

Specifications and QA / QC for HDPE Liners

1.0 SCOPE

1.1 These specifications describe High Density Polyethylene (HDPE) Lining Membranes. The supply and installation of these materials shall be in strict accordance with the Engineer's specifications and engineering drawings and be subject to the terms and conditions of the contract. The subgrade and the HDPE material will meet the specifications contained herein and in the GRI Test Method GM13.

2.0 MATERIAL

2.1 Physical Properties:

- A. The HDPE liner material used in this project shall be a minimum of 60 mil in thickness and have the properties as called out in Table 1(a) of GRI Test Method GM13 (Attachment G).
- B. Raw material shall be first quality polyethylene resin containing no more than 2% clean recycled polymer by weight.
- C. Melt Index (ASTM D1238 Condition 190/2.16): ≤ 1.0 g / 10 min.
- D. Dimensional stability in each direction at +/- 2% max (ASTM D 1204 – 100°C 1 hr).
- E. Environmental stress crack resistance of 1500 hrs min (ASTM D 1693 Condition B).
- F. The new membrane liner shall comprise HDPE material manufactured of new, first-quality products designed and manufactured specifically for the purpose of liquid containment in hydraulic structures.
- G. The lining material shall be manufactured a minimum of 22.5 feet seamless widths. Labels on the roll shall identify the thickness, length and manufacturer's roll number. There shall be no factory seams.
- H. The liner material shall be so produced as to be free of holes, blisters, undispersed raw materials, or any sign of contamination by foreign matter. Any such defect shall be repaired using the extrusion fusion welding technique in accordance with the manufacturer's recommendations.
- I. The contractor shall, at the time of bidding, submit a certification from the manufacturer of the sheeting, stating that the sheeting meets physical property requirements for the intended application. FML rolls will not be installed, if any tested property is below the National Sanitation Foundation (NSF 54) minimum standard.

2.2 Handling:

- A. Delivery: Transportation of the geomembrane shall be performed by the geomembrane manufacturer through an independent trucking firm or other party as agreed by the owner.

Specifications and QA / QC for HDPE Liners

- B. **Offloading:** Geomembrane, when off-loaded, shall be placed on a smooth well drained surface, free of rocks or any other protrusions which may damage the material. No special covering is necessary for geomembrane. The following should be verified prior to off-loading the geomembrane:
 - 1. Handling equipment used on the site is adequate and does not pose any risk of damage to the geomembrane.
 - 2. Personnel informed of proper handling techniques and will do so with care.
- C. Any welding rod delivered to the site prior to the geomembrane installation contractor's arrival should be kept covered and dry or placed in a storage facility.
- D. Upon arrival at the site the geomembrane installation contractor shall conduct a surface observation of all rolls for defects and for damage. This inspection shall be conducted without unrolling rolls unless defects are found or suspected. The geomembrane installation contractor shall indicate any damage to the Project Manager / Owner.
- E. **Storage:** The Project Manager / Owner shall provide storage space in a location(s) such that on-site transportation and handling are minimized. Storage space should be protected from theft, vandalism, passage of vehicles, and be adjacent to the area to be lined.

3.0 MANUFACTURER

3.1 Experience: The manufacturer of the lining material specified in the previous section shall have previously demonstrated the ability to produce this membrane by having successfully manufactured a minimum of ten million square feet of similar liner material for hydraulic lining installations. The liner material provided by the manufacturer must be listed by the NSF (National Sanitation Foundation) Standard 54.

3.2 Factory Quality Assurance and Control

- A. Quality Assurance testing shall be carried out by the geomembrane manufacturer to demonstrate that the product meets this specification.
- B. **Raw Material:** All compound ingredients of the HDPE materials shall be randomly sampled on delivery to the HDPE manufacturing plant to ensure compliance with specifications. Tests to be carried out shall include Density ASTM D1505 and Melt Index ASTM D1238, Condition E.
- C. **Manufactured Roll Goods:** Samples of the production run shall be taken and tested according to ASTM D638 to ensure that tensile strength at yield and break, elongation at yield and break meet the minimum specifications. A quality control certificate shall be issued with the material.
- D. All welding material shall be of a type supplied by the manufacturer.

Specifications and QA / QC for HDPE Liners

- E. All FML material shall be certified as “holiday free” by electrical potentiometric means (spark tested) or other equivalent approved means, during manufacture.

3.3 Submittals: The geomembrane manufacturer shall submit the following information to the Project Manager / Owner:

- A. The origin (resin supplier’s name, resin production plant), identification (brand name, number) and production date of resin.
- B. A copy of the quality control certificates issued by the resin supplier noting results of density and melt index.
- C. Reports on the tests conducted by the geomembrane manufacturer to verify the quality of the resin used to manufacture the geomembrane rolls assigned to the considered facility (these tests should include specific gravity [ASTM D792 Method A or ASTM 1505 and melt index ASTM D1238 Condition 1902.16]).
- D. Reports on these tests conducted by the geomembrane manufacturer to verify the quality of the sheet.
- E. A properties sheet including, at a minimum, all specified properties, measured using test methods indicated in the specifications or equivalent.
- F. After receipt of material, the geomembrane manufacturer shall provide the Project Manager / Owner with one quality control certificate for every roll of FML provided. The quality control certificate shall be signed by a responsible party. The quality control certificate shall include: roll numbers, identification and results of quality control tests. As a minimum, the quality control certificates shall include the results of the geomembrane properties tested by the method and at the frequency shown in the table below.

Property	Test Method	Frequency
Thickness	ASTM D 751	Every Roll
Density	ASTM D 792/1505	Every 5 th Roll
Tensile Yield Strength	ASTM D 638	Every Roll
Yield Elongation	ASTM D 638	Every Roll
Tensile Break Strength	ASTM D 638	Every Roll
Break Elongation	ASTM D 638	Every Roll
Dimensional Stability	ASTM 1204	Every Roll
Tear Resistance	ASTM D 1004	Every Roll
Puncture Resistance	FRMS 101C-2065	Every Roll
Environmental Stress Crack Resistance	ASTM D 1693B	Every Roll
Carbon Black Content	ASTM D-1603	Every 5 th Roll
Carbon Black Dispersion	ASTM D-3015	Every Resin Lot

Specifications and QA / QC for HDPE Liners

4.0 INSTALLATION

4.1 Area Subgrade Preparation: The earthwork contractor shall be responsible for preparing the subgrade according to the basin's design and in accordance with the following specifications. If there is a discrepancy between the project design drawings and the following specifications the more stringent requirements shall apply.

- A. The earthwork shall be smooth and free of all rocks, stones, sticks roots, sharp objects, or debris of any kind. No stones or other hard objects that will not pass through a 3/8" screen shall be present in the top 1" of the surfaces to be covered. No vegetation, brush roots or other foreign material shall be present on the surfaces to be lined.
- B. The surface should be compacted so as to provide a firm, unyielding foundation for the membrane with no sudden, sharp or abrupt changes or break in grade. No ruts, irregularities or soft areas will be present on the surfaces to be lined. The subgrade shall be thoroughly compacted.
- C. No standing water or excessive moisture shall be allowed.
- D. An anchor trench shall be constructed in a square in accordance with detail DF3 / C.DF3 to secure the FML along the berm of the containment structure to be covered. See attached drawings at end of this specification for anchor and cover details.
- E. The installation contractor shall certify in writing that the surface on which the membrane is to be installed is acceptable before commencing work. The FML will be assembled, seamed, tested and installed by the methods specified by a manufacturer recognized by the National Sanitation Foundation, Standard 54.
- F. The subgrade shall be constructed so as to meet the following:
 - 1. The subgrade material will come from either on-site or from approved stockpiles.
 - 2. The earthwork for the anaerobic digesters and the equalization basins will be constructed so the subgrade will be free of any foreign material such as stones greater than 3/8 inch in diameter, vegetation, brush, roots or similar material which could damage the FML.
 - 3. The subgrade material will be classified as CH, CL, CL-ML, ML, SM, SC, SW or SP by the USCS Classification System.
 - 4. A moisture/density curve will be developed for the subgrade material.
 - 5. The minimum compacted thickness of the subgrade layer shall be 8".
 - 6. The subgrade will be compacted and graded to meet the installation contractor's specifications so as to avoid any ruts, irregularities and soft areas. The subgrade will be thoroughly compacted to provide support for the FML.

Specifications and QA / QC for HDPE Liners

7. The subgrade will be compacted to a minimum of 90% dry density. For proper compaction, moisture will be added to the soil in quantities comparable to the OMC.
8. Installed density shall be confirmed by field test methods at a frequency of one test per 200' x 200' grid square.
9. A written statement by an independent professional engineer regarding the subgrade's structural integrity, along with supporting data will be submitted with the liner certification packet.

4.2 Dike Construction: The earthwork contractor shall be responsible for constructing dikes according to the following specifications:

- A. The dike will be constructed of relatively impermeable material.
- B. Each lift shall not exceed 6 inches in depth.
- C. A geotechnical inspector will conduct compaction testing for each two vertical foot intervals at a frequency of 1 per every 400 linear feet.
- D. A written statement by an independent professional engineer regarding the dike's structural integrity, along with supporting data will be submitted with the liner certification packet.

4.3 Anchor Trench:

- A. The attached schematic detail DF3 / C.DF3 at the end of this specification indicates the anchor trench installation. Deviations from this design must be approved by the design engineer prior to use.
- B. Compaction of the anchor trench backfilling will be done promptly after installation of the FML.
- C. Compaction of the trench backfill shall include moisture added to the top 6 inches, with compaction done by a vibratory roller or tamper to firm unyielding surface.
- D. Final grading will be implemented to produce a smooth uniform finish that slopes away from the digester and basins.
- E. A client approved quality control technician shall inspect the anchor trench upon completion. Any portion of the anchor trench inadequately constructed will be re-dug and repaired in accordance with the specifications above.

4.4 Geomembrane Placement:

- A. The installation of the HDPE must be done by the manufacturer, or a manufacturer's authorized distributor, using the manufacturer's extrusion or hot wedge welding equipment and installation methods. All supervisors overseeing the liner installation must

Specifications and QA / QC for HDPE Liners

have five million square feet of supervisory liner experience. All field technicians must have one million square feet of seaming experience.

B. Field Panel identification: A field panel is the unit area of polyethylene which is to be seamed in the field, i.e., a field panel may be a complete roll or partial roll cut in the field. Smaller units used in the lining systems such as repairs, tabs, extensions, etc. need not be documented in the same manner as a field panel.

1. The installer will be responsible for marking each panel with the identification number and the appropriate manufacturer's roll number. It is suggested that the panel number be marked on each end of the panel, after each panel is placed, for ease of reference.

C. Field Panel Placement:

1. Placement Plan: Panel placement should take into account: site drainage (including sump or low point considerations), prevailing wind direction, subgrade construction, access to the site and the production schedule of the project. Adequate slack will be maintained in the liner material during assembly and after installation to minimize stress due to variations in ambient temperature and incident radiation.

2. Installation Sequence: Field deployed panels should be seamed as soon as possible after deployment to minimize the risk of wind or water damage.

3. Weather Conditions: Geomembrane panel deployment shall not proceed when ambient air temperature or adverse weather conditions exist which will jeopardize the integrity of the liner installation. Typically, installation shall not proceed when the ambient temperature is below 20°F or above 110°F. Special low temperature welding techniques may be required in conditions of ambient temperatures between 20°F and 40°F.

4. Geomembrane panel deployment shall not proceed if subgrade conditions have deteriorated due to moisture, or in the presence of high winds which might cause damage to the liner material. Deployed panels should be adequately ballasted at all times to limit the risk of wind damage.

5. Method of Deployment: The FML installation contractor shall proceed with deployment provided the following conditions are met. If the conditions below are not met the FML installation contractor shall cease deployment and resolve the problems with the Project Manager / Owner.

- Any equipment used does not damage the subgrade.
- The subgrade conditions have not deteriorated.
- The subgrade is free of loose rocks, debris, ruts, etc.
- The personnel who are in contact with the liner do not smoke wear damaging shoes or engage in other activities which risk damage to the liner.

Specifications and QA / QC for HDPE Liners

- Adequate sandbags are present to weight the edges of the liner to avoid wind uplifting.
 - Excessive traffic across the liner is avoided.
6. Damage: The FML installation manager and quality assurance technical shall visually inspect each panel, as soon as possible after deployment, for damage or areas needing repair. Appropriate marks indicating a need for repairs shall be done during the inspection. Heavily creased or otherwise defective material shall be rejected.

4.5 Field Seaming & Layout:

- A. Individual panels of liner material shall be laid out and overlapped by a maximum of four inches (101 millimeters) for extrusion weld prior to welding or five inches (127 millimeters) for hot wedge weld prior to welding. Extreme care shall be taken by the installer in the preparation of the areas to be welded.

All sheeting shall be welded together by means of integration of the extrudate bead with the lining material. The composition of the extrudate shall be identical to the lining material, or all sheeting shall be welded together using the hot wedge welding system.

- B. Seam Layout: In general, seams shall be oriented parallel to the plane of maximum slope, i.e., oriented along, not across the slope. In corners and odd shaped geometric locations the number of seams should be minimized. No horizontal seams should occur on a panel less than 5 lineal feet from the top of the slope. On slopes of less than 10% (6:1) this rule shall not apply. Seams will be installed at least four feet into the anchor trench.
1. A seam is considered a separate entity if it joins two panels. Repairs are not considered seams in this context.
 2. A seam numbering system can be used to identify the seams. It is suggested that a simple numerical system be used or adjacent panel numbers can be utilized to identify the seam.
 3. Seams will be welded to at least four feet into the anchor trench.
- C. Seaming Equipment and Products: Approved processes for field seaming and repairing are extrusion welding and fusion welding. All welding equipment should have accurate temperature monitoring devices installed and working to ensure proper measurement of the fusion welding wedge temperature or the extrusion barrel temperature.
- D. Extrusion Welding Process: This process shall be used primarily for repairs, patching and special detail fabrication and can also be used for seaming.
1. The extrusion welding apparatus (Handwelder) shall be equipped with gauges or other temperature monitoring devices to indicate temperature of the extrudate (resin) as well as the applicable pre-heat settings.
 2. The FML installation contractor shall verify the following:

Specifications and QA / QC for HDPE Liners

- a. Equipment in use is functioning properly.
 - b. Welding personnel are purging the machine of heat-degraded extrudate prior to actual use.
 - c. All work by the personnel is performed on clean surfaces and done in a professional manner.
 - d. No seaming is done in adverse weather conditions.
- E. Fusion Welding Process: This process shall be used for seaming panels together and is not generally used for patching or detail work.
1. The apparatus may be of a hot wedge type and shall be equipped with a “split wedge”, used for pressure type seam testing.
 2. Fusion welding equipment shall be self-propelled devices and shall be equipped with functioning speed controllers and monitors to assure proper control by the welding technician. The welding equipment used shall be capable of continuously monitoring and controlling the temperatures in the zone of contact where the machine is actually fusing the lining material so as to ensure that changes in environmental conditions will not affect the integrity of the weld.
 3. The FML installation contractor shall verify the following:
 - a. Equipment in use is functioning properly.
 - b. Welding personnel are performing seaming in a professional manner and are attentive to their duties.
 4. **Figure F-1** below is a schematic detail which indicates acceptable fusion weld. Deviations from these must be approved by the design engineer prior to use.

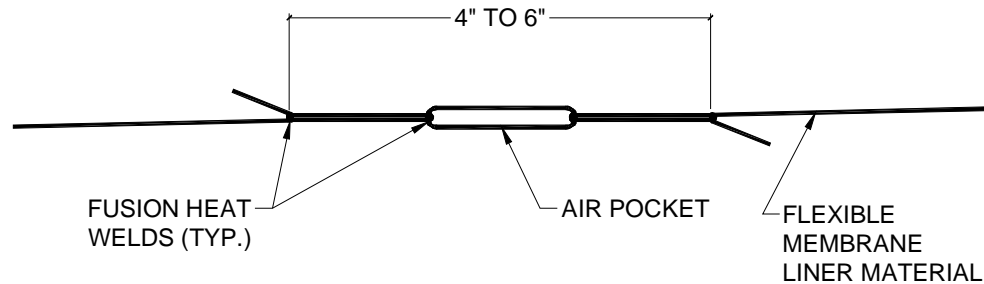


Figure F-1 – Typical Fusion Weld

Specifications and QA / QC for HDPE Liners

- F. Seam Preparation: The area to be welded shall be cleaned and prepared in accordance with this specification and the recommendations of the material manufacturer. The welding technician shall verify the following:
1. Prior to seaming the seam area shall be free of moisture, dust, dirt, sand or debris of any nature.
 2. Seam is overlapped for fusion welding.
 3. Seam is overlapped or extended beyond damaged areas at least 4" when extrusion welding.
 4. Seam is properly heat tacked and abraded when the extrusion welding is done.
 5. Seams are performed with the fewest number of unmatched wrinkles or "fish mouths".
- G. Fish Mouths: No "fish mouths" shall be allowed within the seam area. Where "fish mouths" occur the material shall be cut, overlapped and an overlap extrusion weld shall be applied.
- H. Slack: Adequate slack will be maintained in the liner during assembly and after installation to minimize stresses due to variations in ambient temperature and incident radiation.
- I. Defective Material: Heavily creased or otherwise defective liner material will be rejected.
- J. Weather Conditions for Seaming: No seaming shall be performed in ambient air temperatures or adverse weather conditions which will jeopardize the integrity of the liner installation. Ambient air temperatures shall not exceed 110°F nor be below 20°F during seaming. Additionally, seaming shall not proceed in conditions in which the liner is subject to dew or other condensation, rain, snow, frost or frozen subgrade.
- K. Low Temperature Welding Procedures: The most important criteria for performing welding when the ambient temperature is between 20°F to 40°F is the condition of the trial weld. All trial welds should be made in conditions duplicating the actual welding environment. The following procedures should be used to maintain the quality of the weld in low temperature ambient conditions (20°F to 40°F).
1. Conduct additional trial welds when a welding machine has been shut off, or after a major change in ambient conditions. A major change in ambient conditions would include but is not limited to the following:
 - a. Change in temperature of more than 20°F
 - b. Change in wind speed of more than 10 mph.
 - c. Change in the amount of sunshine on the liner.

Specifications and QA / QC for HDPE Liners

2. The geomembrane and extrudate material must be dry and free from frost, dew, condensation or other moisture.
 3. Hot wedge set temperatures may be increased up to 700°F in 10°F increments as necessary.
 4. The hot wedge rate of travel should be slowed as necessary.
 5. Length of trial weld seams should be increased to 5 ft for extrusion welds and 24 ft for fusion welds.
 6. Clean the seam area immediately in front of the welding apparatus with a clean dry cloth.
 7. Destructively test one specimen, no greater than 6" from the end of each seam to confirm the quality of the seam.
 8. Increase handwelder (extrusion welder) pre-heat temperature up to 600°F in 20°F increments as necessary.
 9. Increase handwelder extrudate temperature up to 530°F in 10°F increments as necessary.
 10. If additional measures are needed to produce acceptable welds the following additional measures may be implemented:
 - a. Install an insulating material such as a geotextile cushion beneath the seam being welded.
 - b. Use hot air pre-heat (additional pre-heat for extrusion welding) 6" to 12" in front of the welding apparatus (both fusion and extrusion welders). Verify weld quality by means of a trial weld.
 11. If trial welds still indicate that a quality weld cannot be produced by the above steps, a wind shield or an enclosure may be placed over the area to be welded. In the case of an enclosure, the enclosed area shall be heated by forced air or radiant means to an air temperature at or above 40°F.
 12. All trial welds will be documented with samples (failures and approved) recorded, retained with samples attached to completion submittal records.
- L. Temporary Bonding: The FML installation contractor shall verify that no solvents or adhesives are used in the seaming area. Tape or heat tacking is permissible for temporarily holding patches but is not a substitute for welding.
- M. Trial seams / Welds: Trial seams / welds shall be made on appropriate sized pieces of geomembrane material to verify that seaming conditions are adequate.

Specifications and QA / QC for HDPE Liners

1. Trial seams / welds shall be performed for each welder to be used and by each operator of extrusion welders, and by the primary operator of the fusion welder.
2. A passing trial seam / weld shall be made prior to seaming each day. If the apparatus is cooled down after use and additional trial seam may be required.
3. Fusion welded trial seams shall be approximately 5 foot long by 1 foot wide with the seam centered lengthwise. For extrusion welding the trial seam sample size shall be approximately 3 feet long by 1 foot wide with the seam centered lengthwise.
4. Test welds shall be marked with date, ambient temperature and welding machine number. All test weld samples will be retained and submitted with approved inspection reports.
5. Samples of weld ¼” to ½” wide shall be cut from the test weld and pulled by hand in peel. The weld should not peel.
6. Refer to Quality Assurance and Quality Control Section 5.2.B for testing requirements.
7. The geomembrane installation contractor shall assign each trial seam / weld sample a number and record the test results in the appropriate log.
8. Upon passing, unless otherwise specified, all trial seam / weld specimens must be retained and submitted with approval inspection reports.

4.6 Defects and Repairs

- A. Once defective or areas requiring repair are identified as called out in Section 5.3. Each area shall be repaired in accordance with this section and non-destructively tested.
- B. Repair Procedures: Any portion of the polyethylene lining system exhibiting a defect which has been marked for repair shall be repaired with one or more of the following appropriate procedures:
 1. Repair Methods:
 - Patching: Used to repair holes, tears, un-dispersed raw materials in the sheet.
 - Grind and Re-Weld: Used to repair small section of extruded seams.
 - Spot Welding: Used to repair small, minor, localized flaws.
 - Flap Welding: Used to extrusion weld the flap of fusion weld in lieu of a full cap.
 - Capping: Used to repair failed seams.
 - Topping: Application of extrudate bead directly to existing seams.
 2. The following conditions shall apply to all of the above methods:
 - a. Surfaces of the polyethylene which are to be repaired shall be abraded.

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- b. All surfaces must be clean and dry at the time of the repair.
 - c. All seaming equipment and personnel used in repairing procedures shall be qualified and documented by the client's third party inspector.
 - d. All patches and caps shall extend at least 4" beyond the edge of the defect and all patches shall have rounded corners.
- C. Large Wrinkles: Large wrinkles that remain in the sheet as a result of temperature expansion or uneven surface preparation may need removal in consideration of applied dead loads on the wrinkle, etc. Should the wrinkle need removing, the lower down slope edge of the wrinkle shall be cut, overlapped and repaired as described above. Both ends of the wrinkle repair shall be patched. Caution must be taken in removing any wrinkles. Wrinkles are needed to allow for future contraction of the geomembrane, especially in cold weather.

4.7 Liner Vents

- A. The attached schematic detail DF4A / C.DF4 depicts a typical vent. Vents shall be installed in accordance with manufacturer's recommendations as well as requirements and recommendations indicated on project design drawings.

4.8 Pipe Penetrations

- A. The attached schematic detail DF4B / C.DF4 depicts a pipe penetration. Pipe penetrations shall be installed in accordance with manufacturer's recommendations as well as requirements and recommendations indicated on project design drawings.

4.9 Final Earthwork, Backfilling and Equipment

- A. Backfilling of Anchor Trench: Promptly after installation of the FML, the anchor trench shall be backfilled by the earthwork contractor or the installer, as specified in the contract. Backfilling should occur when the geomembrane is in its most contracted (taut) state. Care must be taken when backfilling to avoid damage to the FML.
- B. Construction Equipment: Construction equipment or vehicles with steel tracks shall not be permitted directly on the geomembrane liner. Vehicles with rubber tires, without a tugged tread and with a loading of less than 6.0 lbs / in² weight are allowed, provided proper care is taken when operating the vehicle to avoid stressing the geomembrane. Other equipment such as portable generators shall be permitted if the support apparatus for the equipment protects the liner from being damaged.

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5.0 QUALITY ASSURANCE AND QUALITY CONTROL

5.1 Materials:

- A. The FML installation contractor or quality control technician shall verify that the property values certified by the geomembrane manufacturer meet all of the specifications; that the measurements of properties by the geomembrane manufacturer are properly documented; and that the test methods used are acceptable.

5.2 Field Seam Testing / Quality Control

- A. The end user company, or their designated representative, reserves the right of access for inspection of any or all phases of this installation at their expense.
- B. Qualifications of personnel: All personnel performing seaming operations shall be qualified by experience. At least one welder (Master Welder) shall be on site at all times during the seal welding process and have experience seaming a minimum of 5,000,000 ft² of geomembrane. The “Master Welder” shall provide supervision of the less experienced welding technicians during seaming, patching and testing operations.
- C. Testing of coupons (strips of material) before seaming, stress cracks and all seams must be done in accordance with the FML manufacture’s requirements.
- D. Trial Welds / Seams:
 - 1. Four specimens, each 1” wide and 6” apart from each other shall be cut from the trial seam. Two of the specimens shall be tested in shear and two specimens tested in peel. Both shear and peel tests shall be conducted to the yield point of the geomembrane. When testing a fusion welded seam the outside (top) weld of a split-wedge weld should be considered the primary weld and shall be the weld tested in peel. The specimen must exhibit the following properties to pass:
 - a. Shear Test: Both specimens must meet or exceed the bonded seam strength values in shear of both specimens shall exhibit a bonded seam strength in shear that is greater than 90% of the minimum yield tensile strength of the parent material.
 - b. Peel Test: Both specimens must exhibit failure of the parent material or meet or exceed the bonded seam strength values in peel, or strength values shall be greater than 70% of the minimum yield tensile strength of the parent material.
 - 2. General seaming operations may proceed prior to the test being complete. Should a trial seam fail, a sample shall be removed 3 lineal feet from the start of the seaming operations and tested per the above. This procedure will be repeated and followed until a passing sample is located. All work preceding the passing sample shall be repaired.

Specifications and QA / QC for HDPE Liners

E. Non-Destructive Seam Continuity Testing

1. Concept: The FML installation contractor shall non-destructively test and document all field seams over their full length using an air pressure test or vacuum test. The purpose of non-destructive tests is to check the continuity of the seams.
2. The FML installation contractor shall:
 - a. Schedule all non-destructive testing operations.
 - b. Instruct the testing personnel regarding marking of repairs needed, leaks and sign-off marks on seams and repairs.
 - c. Monitor the operations of testing personnel to ensure that procedures for testing are followed.
3. On seams that cannot be non-destructively tested by vacuum or air-pressure methods due to physical constraints, (i.e. a boot detail) the seam shall be tested using other approved methods.
4. Vacuum Testing:
 - a. Equipment:
 - Vacuum box assembly consisting of a rigid housing, a transparent viewing window, a soft gasketing material attached to the bottom, a valve assembly and a certified vacuum gauge.
 - Vacuum pumping device. Including back-up device
 - Foaming agent in solution.
 - Equipment suitable for applying the foaming agent.
 - b. Procedure:
 - Wet the section of the seam with foaming agent.
 - Place the vacuum box over the wetted area.
 - Energize the pumping apparatus.
 - Obtain a minimum pressure of -5.0 psi.
 - For a period of approximately 10 seconds, observe, through the viewing window, for the presence of soap bubbles.
 - If no bubbles are observed, reposition the box on the next area for testing.
 - If bubbles are detected, mark and document location of the leak so repairs can be made.
5. Air Pressure Testing: The following procedures are applicable for seams produced by a double-fusion welding apparatus.

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a. Equipment:

- Air pump or air tank equipped with pressure gage and capable of producing pressures between 25 to 30 psi.
- Sharp hollow needle to insert the air into the air chamber of the seam.

b. Testing Procedure:

- Installer will provide for approval a detailed seam testing map prior to the starting of seal tests.
- Seal both ends of the air channel in the seam to be tested.
- Insert the hollow needle into the air chamber at either end of the seam to be tested.
- Energize the air pump to a pressure between 25 and 30 psi and read pressure inserted into the chamber. Allow the pressure to stabilize and if necessary, re-pressurize to between 25 and 30 psi. Then record the pressure.
- Wait for a minimum of 5 minutes and then record the air pressure again.
- If the difference between the initial and the final pressure is greater than 4 psi the seam failed. Documentation required on all failed tests.
- Upon completion of all readings, open the opposite end of the seam with a needle. The escaping air will confirm that the entire length of the seam was pressurized and therefore tested.
- Upon passing the air pressure test, the seam shall be marked and documented.
- All Seam tests shall be witnessed by client or clients inspector.

c. Procedure for Air Pressure Test Failure:

- While the seam air-channel is under pressure, traverse the length of the seam and listen for the leak. Once the area of the leak has been narrowed down, apply a soapy solution to the seam edge (do not trim excess material from edge of seam) and observe for bubbles formed by escaping air.
- As an alternative to the step above the seam may be re-tested in progressively smaller increments, until the area of leakage is identified.
- Repair the identified leaking area by extrusion welding the excess material at the edge of the seam and then vacuum test.
- In areas where the air channel is closed and the integrity of the weld is not suspect, vacuum testing is acceptable.

F. Destructive Seam Testing

1. Concept: Destructive seam tests shall be performed at locations selected by client's inspectors. The purpose of these tests is to evaluate bonded seam strength. Seam strength testing shall be performed and documented as work progresses.
2. Location and Frequency: The minimum frequency of sample removal shall be one sample per 750 ft of seam. The location of the test sample will be taken no greater than 6" from the end of the seam. Additional test samples removal as requested by the client or client's inspector.

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3. Size of Samples: The size of the sample for independent testing shall be 12" by minimal length with the seam centered lengthwise. The sample shall be cut into the following segments and distributed as follows:
 - a. The first segment cut shall be 12" x 12" marked with the appropriate D/S number and given to the AQ technician for testing.
 - b. The second segment, 12" x requested length (18" max) shall be marked with the appropriate D/S number and transmitted at the contractors cost to the independent testing laboratory or the quality assurance technician personnel for their dispersal.
4. Field Testing: The segments given to the quality assurance technician shall be tested in peel and in shear using the following criteria:
 - a. Ten specimens of 1" width shall be cut from the segment.
 - b. Five of the specimens shall be tested in a peel configuration. The outside (top) weld of a split wedge weld shall be considered the primary weld and shall be the weld tested in peel.
 - c. Five of the specimens shall be tested in a shear configuration.
 - d. The geomembrane manufacturer shall supply a field tensiometer equipped with a drive / pull apparatus adjusted to a pull rate of 2"/min to 20"/min and a means of measuring the strength of the sample.
 - e. Pass Fail Criteria: The installers sample will pass when:
 - The peel specimens exhibit failure of the parent material.
 - The bonded strength peel values shall be greater than or equal to 70% of the minimum yield tensile strength of the parent material.
 - The shear specimens display parent material failure.
 - If the bonded seam strength in shear values is not listed, the shear values shall be greater than or equal to 90% of the minimum yield tensile strength of the parent material.

Note: Locus of break determinations is to be in accordance with ANSI/NSF 54

 - Four out of five specimens meeting the above criteria constitute a passing test.
 - f. Procedure for Failing Tests:
 - Two samples of the same size shall be removed from the failed seam. The first sample shall be removed 10 lineal feet in front of the failed sample and the second shall be removed from behind the failed sample.
 - Label the samples A and B and test in accordance with procedures listed above.

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- If both samples A and B pass, seam between the location of samples A and B shall have the flap extrusion welded or be capped and non-destructively tested accordingly.
- If either sample A or B fails, additional samples shall be taken a minimum distance of 10 feet away from the failed test location. Testing shall continue as outlined above until the area of incorrect seam is isolated.
- In lieu of taking an excessive number of samples, the installer may opt at their cost to extrusion weld the flap or cap for the entire length of the seam then non-destructively test the seam.
- All failing tests shall be documented and forwarded to the client or client's representative within 24 hours, along with recommendation of correction

5.3 Defects and Repairs

- A. Identification: All seams and non-seam areas of the polyethylene lining system shall be examined for defects in the seam or sheet.
- B. Identification of the defect may be made by marking on the sheet/seam with paint or other marks. The following procedure shall be followed:
 1. For any defect in the seam or sheet that is an actual breach (hole) larger than ¼” in the liner system, the installer personnel shall circle the defect and mark the letter “P” inside the circle. The letter “P” indicates that a patch is required.
 2. For any defect in the seam or sheet that is less than a ¼” hole, the installer personnel shall only circle the defect indicating that the repair method may be only an extruded bead and a patch may not necessarily be required. Repair methods will be at the sole discretion of the client and the client's qualified inspection representative.
- C. Unless otherwise specified, only the geomembrane installation contractor or quality assurance technician shall be permitted to mark on the liner system. The quality assurance technician shall use markings that are distinguishable from the geomembrane installation contractor markings.
- D. Verification of Repairs: Each repair shall be non-destructively tested in accordance with requirements of these specifications and manufacturer's recommendations. Once passing tests are achieved a marking shall be placed on the repair, indicating the test is complete and the area has passed the test. If defects remain, appropriate markings shall be made to clearly indicate that additional repairs are required.

5.4 Final Approval

- A. A final inspection of the completed liner will be conducted by the FML installation contractor, quality assurance technician and project manager / owner. This careful evaluation will occur before the Division of Water Quality is asked to approve the use of the lined lagoon. The purpose of the inspections is to verify the following:
 1. All repairs have been appropriately performed.

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2. All test results are positive.
 3. Area is free of scrap, trash and debris.
 4. Anchor trench has been properly backfilled.
 5. Liner has been installed according to the requirements of these specifications, the project documents and the manufacturer's recommendations.
 6. Four (4) copies in three ring binders of all installation record documents will be required prior to final acceptance.
- B. Each liner material test, construction inspection checklist, data sheet, or narrative report will be preserved for inspection by the Division of Water Quality. Waste shall not be discharged into the digesters or equalization basins prior to the approval of the Division of Water Quality.

6.0 Warranty and Guarantee

- 6.1** The manufacture / Installer shall provide a written warranty in accordance with the requirements specified by the owner and / or design engineer.

Attachment E

State of Utah Geological Survey Paper #1836 entitled "Ground-Water Conditions and
Geologic Reconnaissance of the Upper Sevier River Basin, Utah"

Ground-Water Conditions
and Geologic Reconnaissance
of the Upper Sevier
River Basin, Utah

A. C. W. CARLISLE, G. D. LUTHER, JR., AND E. J. DORLAND

GEOLOGICAL SURVEY WATER-SUPPLY PAPER 1836

*Prepared in cooperation with the
Utah State Engineer*



UNITED STATES DEPARTMENT OF THE INTERIOR

STEWART L. CHAFFE, *Secretary*

GEOLOGICAL SURVEY

William T. Pecora, *Director*

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GROUND-WATER CONDITIONS AND GEOLOGIC RECON-
NAISSANCE IN THE UPPER SEVIER RIVER BASIN, UTAH

By O. H. CARPENTER, G. B. ROBERTSON, JR.,
and L. J. RYAN, JR.

ABSTRACT

The upper Sevier River basin is in south-central Utah and includes an area of about 2,400 square miles in high plateau and valleys. It comprises the entire Sevier River drainage basin above Kingman, including the East Fork Sevier River and its tributaries. The study was investigated to determine general ground-water conditions, the interrelation of ground water and surface water, the effects of increasing the pumping of ground water, and the amount of ground water in storage.

The basin includes four main valleys—Panguitch Valley, Clatsa Valley, East Park Valley, and Gross Valley—which are drained by the Sevier River, the East Fork Sevier River, and Other Creek. The plateaus surrounding the valleys consist of sedimentary and igneous rocks that range in age from Triassic to Quaternary. The valley fill, which is predominantly alluvial gravel, sand, silt, and clay, has a maximum thickness of more than 500 feet.

The four main valleys constitute separate ground-water basins. East Park Valley basin is divided into Bryce Valley, Johns Valley, and Antelope subbasin, and Gross Valley basin is divided into Escalante and Angle subbasins. Ground-water occurs under both unconsolidated water-table conditions in all the basins and subbasins except John Valley, Bryce Valley, and Angle subbasins, where water is only under water-table conditions. The water is under artesian pressure in both of gravel and sand confined by underlying beds of silt and clay in the Escalante part of Panguitch Valley basin, Clatsa Valley basin, and Antelope subbasin, and in most of Hatchman to Hatch. Along the sides and upstream ends of these basins, water is usually under water-table conditions.

About 1 million acre-feet of ground water that is readily available to wells is stored in the gravel and sand of the upper 250 feet of saturated valley fill. About 270,000 acre-feet is stored in the silt and clay of saturated valley fill in Clatsa Valley basin, about 67,000 in Bryce Valley subbasin, about 80,000 in Johns Valley subbasin, about 26,000 in Antelope subbasin, about 60,000 in Escalante subbasin, and about 50,000 in Angle subbasin. Additional water, although it is not readily available to wells, is stored in beds of silt and clay. Some ground water also is available in the bedrock underlying and surrounding the basins, although the bedrock formations generally are poor aquifers.

The principal source of recharge to the water table in the upper Sevier River basin is infiltration from stream channels, and is directed to a. Some ground water also moves into the valley fill from the high rock surrounding the basins.

The thickness of the valley fill varies from 200 to 1,000 feet, and is composed of coarse sand and gravel, silt, and clay. The thickness of the valley fill varies from 200 to 1,000 feet, and is composed of coarse sand and gravel, silt, and clay. The thickness of the valley fill varies from 200 to 1,000 feet, and is composed of coarse sand and gravel, silt, and clay.

Approximately 1,000 acre-feet of ground water was discharged in 1907 from the valley fill. Springs discharged about 38,000 acre-feet, wells about 5,000, and domestic about 4,000; and evaporation from green surface areas about 42,000 acre-feet. Springs in basins discharged an average of 17,000 acre-feet. Most of the water discharged by springs, wells, and during was used for irrigation.

The ground water in the basin generally is of good chemical quality. The water is excellent for irrigation and stock, but is not as desirable for domestic use as that of the consolidated basins. The dissolved solids content of the water is generally less than that of the consolidated basins, and is generally of good quality. The water is excellent for irrigation and stock, but is not as desirable for domestic use as that of the consolidated basins.

Surface water and ground water in the upper Sevier River basin are interconnected, and the base flow of streams is affected by changes in ground-water levels. Increased pumping of ground water would result in (1) a decrease in the quantity of water available from surface water sources or (2) a decrease in the storage capacity of streams, springs, flowing wells, and areas of infiltration or other combination of these.

About 1,000 acre-feet of ground water is now discharged annually by irrigation systems from phosphate mines, and perhaps one-third of this loss, or about 300 acre-feet, could be saved by eliminating wet areas and phosphate mines. The water which would be saved could be used for irrigation in the consolidated basins of the Sevier River basin, and for stock and domestic use. The loss of the 1,000 acre-feet of water could be pumped from the phosphate mines or discharged by properly designed drains without greatly affecting stream flow, and with only moderate effect on spring discharge. If the wells were properly located, the pumping would lower water levels and dry up wet areas where phosphates grow. Continued use of ground water and surface water would increase the available use of it some richness in the basin.

INTRODUCTION

PURPOSE AND SCOPE OF THE INVESTIGATION

The U. S. Geological Survey, in cooperation with the Utah State Engineer, investigated ground-water and geologic conditions in the upper Sevier River basin to determine the following: the availability of water in the unconsolidated valley fill and the consolidated rocks in the basin; the amount of water in storage in the valley fill; the relation of ground water and surface water; and the effect of pumping additional quantities of ground water. The investigation was part of a cooperative program of ground-water investigation in the upper Sevier River basin, which began with a study of ground-

water conditions in the central Sevier Valley in 1966 (Young and Carpenter, 1965).

The investigation in the upper Sevier River basin included determination of the relation of geology to ground water; sources, occurrence, recharge, and discharge of ground water; present ground water development; fluctuation of water levels; chemical quality of ground and surface waters; relation between ground water and surface water; inflow-outflow analysis of several sub-basins; the amount of ground water stored in the valley fill; and an evaluation of potential deep aquifer and its effect on hydrologic conditions in the area.

LOCATION AND EXTENT OF THE AREA

The upper Sevier River basin occupies about 2,400 square miles in south-central Utah, and it includes parts of Garfield, Iron, Kane, Uinta, and Sevier Counties (fig. 1). It comprises the Sevier River drainage basin above Kingston, including the Sevier River, the East Fork Sevier River, and their tributaries. The geologic reconnaissance covered the entire drainage basin, but the details of hydrologic study were concentrated in the valleys of an area of about 900 square miles.

PREVIOUS WORK

Previous hydrologic studies in the upper Sevier River basin by the U.S. Geological Survey resulted in reports on the surface-water resources of the Sevier Lake basin (Wadley, 1927), the ground-water resources of the Bryce Canyon National Park area (Keefer, 1952), and the hydrology and hydrogeology of Navajo Lake (Wilson and Huxon, 1951). The Geological Survey has collected streamflow records in the basin since 1911 and has measured ground-water levels in the basin since 1955. These data have been published annually or at intervals of 6 years in U.S. Geological Survey Water Supply Papers. The Sevier River water diversions have measured and compiled records of diversions for irrigation for most years since 1917.

Investigations of the geology and geography of parts of the upper Sevier River basin and adjacent areas have been made by Averitt (1932), Callaghan (1938, 1960), Callaghan and Packer (1951, 1952a, b), Gregory (1911, 1918, 1918, 1952a, b, 1953), Gregory and Moore (1951), and Willard and Callaghan (1962).

Honorable LaRoy H. Cox (1936), judge of the First Judicial District of the State of Utah, compiled water rights in the upper Sevier River basin in a court decree adjudicating the Sevier River system.

4 GROUND WATER, UPPER SEVIER RIVER BASIN, UTAH

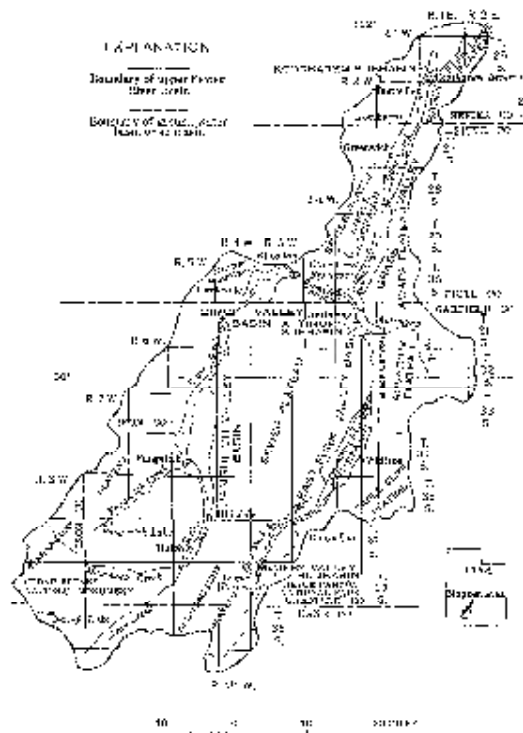


FIGURE 1. Map of the upper Sevier River basin showing physiography and ground-water basins and subbasins.

PERSONNEL AND METHODS OF INVESTIGATION

R. A. Young, project chief, and C. H. Carpenter began the investigation in July 1961. Mr. Young was transferred in December 1961, and L. J. Bjorklund was assigned as project chief. Mr. Bjorklund was assigned to another investigation in the Sevier River basin in September 1962, and Mr. Carpenter was designated project chief. G. B. Robinson, Jr., was assigned to the project in February 1963. R. D. Bullis supervised the test-drilling program during the summer of 1962, assisted by G. B. Robinson, Jr., and they prepared a report on the test-drilling (Bull and Robinson, 1963.)

Many types of basic data were collected and analyzed during the investigation. Much of the data, including well and spring records, water-level measurements, well logs, and chemical analyses, are included in a separate report (Carpenter, Robinson, and Bjorklund, 1964).

More than 500 wells and 50 springs were recorded; periodic water-level measurements were made in 58 observation wells and water-level recording gages were maintained on 4 wells. Estimates of ground-water discharge from wells, springs, and seeps were made using periodic discharge measurements at selected locations and single measurements at other locations. Aquifer tests were made using selected wells to determine well performance and the hydraulic properties of the aquifers. Chemical analyses were made for 10 samples collected from surface-water sources and 50 samples collected from ground-water sources.

Many drilled logs were studied to provide information about the thickness and composition of the valley fill, and in addition 21 test holes were drilled during 1962. The test-drilling program was financed by the U.S. Geological Survey in cooperation with Garfield, Piute, Sevier, Sanpete, and Mohave Counties, many of the irrigation companies in those counties, and the Utah State Engineer. The test holes were drilled by the rotary method, and composite samples were obtained for 10-foot intervals. The samples were examined microscopically to determine their mineral and fossil content, and electric and gamma-ray logs of several of the holes were made to help indicate the character and thickness of the material penetrated. Seven of the test holes were cased and used as observation wells.

A geologic map was compiled mainly from field reconnaissance and photogeologic data and partly from data from available reports.

Stream-gaging stations were installed at Panguitch Creek near Panguitch, East Fork Sevier River near Aullimony, and Otter Creek near Aullimony. Streamflow data from these and other stations and records of diversions for irrigation were studied and measured with

ground-water levels and precipitation data to determine the relation between ground water and stream flow.

The amount of ground water consumed by evapotranspiration was estimated from data on transpiration and by phreatophytes in other areas in Utah and in other Western States. These data were applied to areas of xerophytic growth in the upper Sevier River basin. Evapotranspiration was estimated from areas of surface evaporation and rates of evaporation assumed to be those observed in the central Sevier Valley (Young and Carpenter, 1964).

Inflow-outflow studies were made for all basins and subbasins, using all available data for ground water, surface water, canal water, precipitation, geology, and climatology. These studies accounted for all water entering and leaving each area.

ACKNOWLEDGMENTS

Officials of Garfield, Kane, Sevier, Sumpter, and Millard Counties and of some irrigation companies assisted in financing and organizing the test-drilling program.

Information on streamflow was furnished by the Sevier River water commission, K. B. Christensen. Personnel of the Soil Conservation Service, U.S. Department of Agriculture, helped map and classify phreatophytes and provided information on irrigation. Personnel of the National Park Service helped with the collection of water level data in the Bryce Canyon National Park area. Many individuals contributed information about their wells and permitted the measurement of water levels in the wells.

WELL AND SPRING NUMBERING SYSTEM

The well and spring numbers used in this report indicate the location by land subdivision according to a numbering system that was devised comparatively by the Utah State Engineer and the Geological Survey about 1955. The system is illustrated in figure 2. The complete number comprises letters and numbers that designate consecutively the quadrant and township (shown together in parentheses as a capital letter designating the quadrant in relation to the base point of the South Base and Meridian, and numbers representing the township and range); the number of the section; the quarter section (designated by a letter); the quarter of the quarter section; the corner of the quarter-section or section (with, finally, the section or well within the figure that (designated by a number). If a spring is indicated, the first number is omitted. By this system the letters A, B, C, and D designate respectively East, west, and West, southwest, and south, and east, west, of the standard base and meridian system of the Bureau of Land Management, and the letters a, b, c, and d designate respectively the

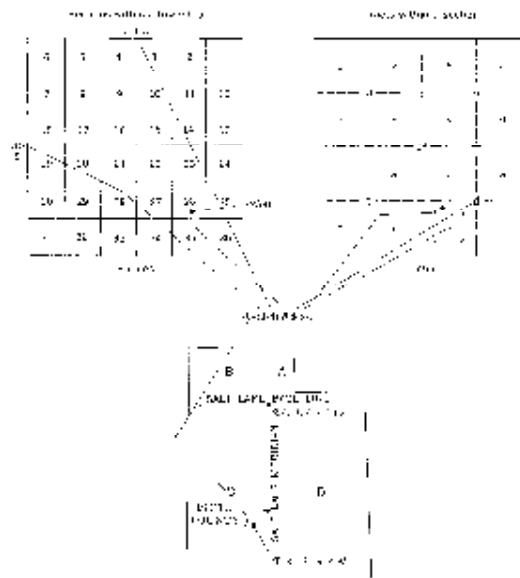


FIGURE 2. Well and spring locations in section 10-31-1726.

northeast, north-west, southeast, and southwest quarters of the section, of the quarter section, and of the quarter-quarter section. Thus, the number (C-30-2)2046-1 designates well 1 in the NW¼ SW¼ SE¼ of sec. 20, T.30 S., R. 4 W., the letter C showing that the township is south of the Salt Lake Base Line and the range is west of the Salt Lake Meridian.

GEOGRAPHY

PHYSIOGRAPHY

The upper Snake River basin is in the High Plateaus of Utah south of the Colorado Plateau physiographic province (Fenneman, 1931, p. 283). The basin comprises four arid valleys—Panguitch Valley, Circle Valley, East Fork Valley, and Grass Valley—which are

surrounded by high plateaus and mountains (fig. 1). Panguitch Valley and Circle Valley combined locally are called South Fork Valley.

Panguitch Valley is approximately 40 miles long and is as much as 8 miles wide in the area north of Panguitch. The altitude of the valley ranges from about 8,500 feet at the north end to about 7,500 feet at the south end. The valley is bordered on the west by the Markagunt Plateau, which reaches an altitude of more than 11,000 feet above mean sea level, and on the east by the Panosaugunt and Sevier Plateaus, which reach altitudes of more than 9,500 and 11,000 feet, respectively.

Circle Valley is about 5 miles long and is more than 6 miles wide at Circle Pt. The altitude of the valley floor ranges from about 8,000 feet at the north end to about 6,200 feet at the south end. The valley is bordered on the west by the Tushet Mountains, which reach an altitude of more than 11,000 feet, and on the east by the Sevier Plateau.

East Fork Valley is approximately 75 miles long and is more than 5 miles wide near Wildsee. The altitude of the valley floor ranges from more than 6,300 feet at the head of Kingston Canyon to more than 8,200 feet south of Temple Reservoir. The valley is bordered on the west by the Panosaugunt and Sevier Plateaus, and on the east by the Table Cliff and Aqueduct Plateaus, which reach altitudes exceeding 10,000 and 11,000 feet, respectively.

Grass Valley is approximately 20 miles long and ranges in width from half a mile in the area south of Greenwich to about 4 miles at Greenway. The altitude of the valley floor ranges from about 6,400 feet at Otter Creek Reservoir to about 7,200 feet north of Koohtatun Reservoir. The valley is bordered on the west by the Sevier Plateau and on the east by the Awapit and Fish Lake Plateaus, which reach altitudes exceeding 8,000 and 11,500 feet, respectively.

Each of the valleys consists of three parts: (1) a valley floor, the flood plain of the main stream in the valley, (2) a valley basin, those areas that are underlain by unconsolidated deposits, and (3) the valley sides—quartzite underlain by bedrock. These features are shown on the geologic cross sections (pl. 1), and a more detailed description of the structure of the basins is given in the section on geology (p. 11).

The discussion of ground-water conditions in this report is by valley basins. These basins are Panguitch Valley basin, Circle Valley basin, East Fork Valley basin, and Grass Valley basin. East Fork Valley and Grass Valley basins are further divided into subbasins. East Fork Valley basin includes Emery Valley subbasin, John's Valley subbasin, and Antelope subbasin; and Grass Valley basin includes Koohtatun subbasin and Angle subbasin (fig. 1).

CLIMATE

The climate in the upper Sevier River basin ranges from semiarid in the valleys to humid on the plateau. The climatological data recorded at Panguitch are regarded as typical of the valleys in the region.

Large daily ranges in temperature are usual in the valleys. The temperature rarely exceeds 80°F in the daytime and is usually between 40° and 50°F during summer evenings. Winters are usually cold in the valleys, and temperatures below 0°F are common. The average annual temperature at Panguitch is 43°F. The frost-free, or growing, season ranges from 2 to 34½ months in the valleys, and below-freezing temperatures have been recorded in every month of the year. The lowest temperature recorded at Panguitch was -32°F in January 1877, and the highest was 86°F in June 1931. The average frost-free period at Panguitch is from June 16 to September 9.

The principal precipitation in the valleys is during July, August, and September when warm moist air moves from the sea from the Gulf of Mexico. The annual precipitation in the valleys ranges from about 7 to 16 inches; November and June usually are the driest months and July and August the wettest. The area is still benefited by storms, however, from both the northern and southern Pacific coasts between September and May. Most of the precipitation from these storms falls on the surrounding high plateaus in the form of snow. This precipitation has an annual range from about 20 to 45 inches, and the same accumulates in places to depths of more than 10 feet and often has a water content of as much as 40 inches.

Annual precipitation at Panguitch ranged from a minimum of 5.41 inches in 1912 to a maximum of 18.62 inches in 1910 and averaged 9.12 inches for 36 years of record (1881-93). The trend in precipitation between 1923 and 1965 is illustrated by a graph of the cumulative departure from the mean annual precipitation at Panguitch (fig. 3). The wettest years are shown by the most steeply rising parts of the graph and the driest years by the steepest descents. The late 1920's and the 1940's were wetter than average, and the climate has been relatively dry since 1950 except during 1953, 1957, and 1961.

Winds in the area usually are light to moderate in all seasons. The only strong winds usually are associated with thunderstorms and squalls.

Evaporation in the valleys greatly exceeds annual precipitation. Mean annual evaporation at Pate Reservoir, 3 miles north of Kington, is 56.9 inches (U.S. Weather Bureau, written communication, 1958) and is considered to be representative of potential evaporation in the valleys of the upper Sevier River basin.

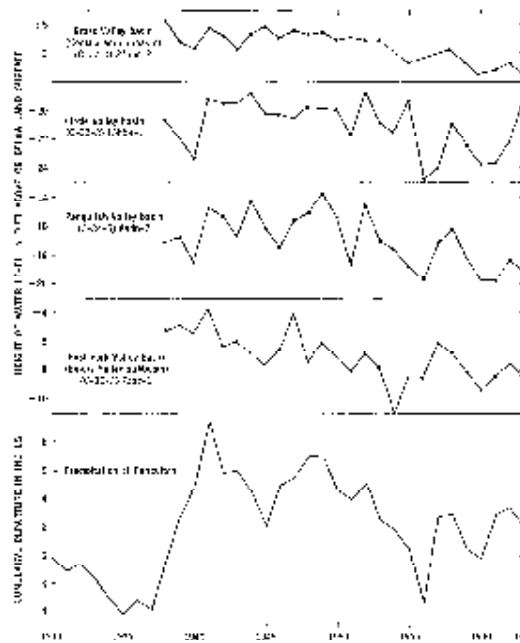


FIG. 8.—Hydrographs of seasonal water for the period 1928-44 and cumulative deficits from the 1931-81 period for precipitation at Panguitch.

VEGETATION

Native vegetation in the upper Severn River basin ranges in type from desert to alpine. Sagebrush (*Artemisia tridentata*), rabbitbrush (*Chrysothamnus nauseosus*), greasewood (*Goodyeropsis monanthoides*), willow (*Salix* sp.), and sagebrush (*Artemisia tridentata*) grow in the unalluviated lands of the valleys. The vegetation on the alluvial fans and lower hills up to an altitude of about 7,000 feet is mainly amaranth, juniper (*Juniperus* sp.), scrub oak (*Quercus* sp.), mountain-sagebrush (*Chrysothamnus* sp.), and piñon pine (*Pinus edulis*).

At an altitude of about 7,000 feet, aspen (*Piceus tremuloides* Mill.), ponderosa or yellow pine (*Pinus ponderosa*), spruce (*Picea* sp.), and Douglas fir (*Pseudotsuga densifolia*) predominate. These genera are most dense on the plateau and mountain slopes having a northern exposure. Along all stream channels in the valley, willows and cottonwoods (*Populus* sp.) are the principal vegetation.

POPULATION, AGRICULTURE, AND INDUSTRY

The total population in the upper Sevier River basin is about 3,000. Panguitch, the largest community, has a population of about 1,400. Most of the local residents are engaged in agriculture and related activities and live in towns near their farms. The principal crops are alfalfa, native hay, small grains, and potatoes. Sheep and cattle raising is an important part of the agricultural economy. Next to agriculture, lumbering and service are the most important sources of income.

A large part of the area is administered by the U.S. Forest Service (Dixie and Fish Lake National Forests), the Bureau of Land Management, and the National Park Service (Bryce Canyon National Park and Cedar Breaks National Monument).

GEOLOGY

The geologic map of the upper Sevier River basin was compiled partly from maps in previous geologic reports and partly from photogeologic and geologic field studies conducted during this investigation. The previous maps are primarily the work of Gregory (1922), Olson (1951) and Macine (1952). (See pl. 1.)

During this investigation the geology of approximately 1,600 square miles of the basin was mapped. This mapping was done in a single field season and, hence, is considered a reconnaissance. The valley and mountain areas comprising sedimentary basins were studied in greatest detail; these areas were mapped on aerial photographs, primarily in the field, but some were not checked in the field. Areas containing only volcanic rocks were mapped by photogeologic methods, with but slight field checking.

The geology obtained from maps in previous reports was adopted with only a few changes: the valley floor was subdivided into several fault blocks, some outcrops were recognized on the ground with these shown on aerial photographs, outcrops were redefined along map boundaries to conform with mapping done during this study, and faults and small outcrops were added in places to show slightly greater detail. These changes are primarily in the valley areas, in the area around and north of Panguitch Lake, and in the northern Panguitch Plateau.

GEOLOGIC FORMATIONS AND THEIR WATER-BEARING CHARACTERISTICS

The geologic formations exposed in the upper Sevier River basin include rocks of Tertiary, Cretaceous, Cretaceous, Tertiary, and Quaternary age. Rocks older than Late Cretaceous age, however, although widely exposed in surrounding areas, are limited to an exposure of less than 10 square miles near the head of Antimony Creek; elsewhere in the basin they are deeply buried. Rocks of Late Cretaceous age are exposed principally on the Panamint Plateau, and rocks of Tertiary age are exposed almost everywhere in the area except where covered with valley fill. Unconsolidated deposits of Quaternary age fill the valley basins, and form the reservoir for most of the ground water in the project area.

The areal distribution and structure of the various formations are shown on the geologic map (pl. 1). The structure and some of the important physiographic elements in the area are shown on the geologic sections (pl. 1). The age, thickness, lithology, surface expression, and water-bearing characteristics of the formations are summarized in table 1 and described in detail in the pages that follow.

MESOZOIC FORMATIONS

The oldest rocks exposed in the upper Sevier River basin are in the uppermost block of the Panamint fault on the north-west edge of the Apurimac Plateau in Antimony Creek canyon and Dry Wash (pl. 1). The outcrops include six formations and one additional formation (member of Late Triassic and Jurassic age) and two formations of Late Cretaceous age (Gregory, 1914, p. 582-583). These formations, individually listed in table 1, have only small areal exposure and elsewhere in the basin lie at great depths; hence, they are not important as sources of ground water. They are shown on the geologic map (pl. 1) in preliminary colors.

CRETACEOUS SYSTEM

UPPER CRETACEOUS SERIES

LATE CRETACEOUS FORMATIONS

General description

Upper Cretaceous formations include the Straight Cliffs and Wahweap Sandstones and the Kaiparowits Formation. The Straight Cliffs and Wahweap Sandstones are lithologically and hydrologically similar; are exposed along the sides of the Panamint Plateau, along the east side of Juniper Valley subbasin and bordering the southern part of Julius Valley subbasin (pl. 1); and are probably continuous

Table 1. - Researcher's profile section in the paper 'Stier et al. 2008'

Researcher	Researcher's name	Researcher's title	Researcher's affiliation	Researcher's address	Researcher's phone	Researcher's email	Researcher's website
Stier, M.	Stier, M.	PhD	University of California, Berkeley	1000 University Avenue, Berkeley, CA 94720-1500	415-848-3200	mstier@berkeley.edu	http://www.stierlab.org
	Stier, M.	PhD	University of California, Berkeley	1000 University Avenue, Berkeley, CA 94720-1500	415-848-3200	mstier@berkeley.edu	http://www.stierlab.org
	Stier, M.	PhD	University of California, Berkeley	1000 University Avenue, Berkeley, CA 94720-1500	415-848-3200	mstier@berkeley.edu	http://www.stierlab.org
Stier, M.	Stier, M.	PhD	University of California, Berkeley	1000 University Avenue, Berkeley, CA 94720-1500	415-848-3200	mstier@berkeley.edu	http://www.stierlab.org
	Stier, M.	PhD	University of California, Berkeley	1000 University Avenue, Berkeley, CA 94720-1500	415-848-3200	mstier@berkeley.edu	http://www.stierlab.org
	Stier, M.	PhD	University of California, Berkeley	1000 University Avenue, Berkeley, CA 94720-1500	415-848-3200	mstier@berkeley.edu	http://www.stierlab.org

14 GROUND WATER, UPPER SEVEN RIVER BASIN, ILLINOIS

TABLE 1.—Generalized geologic sections in the major basins, Upper Basin, Illinois.

Section	Units	Subgroup and formation	Thickness feet	Description of rocks	Character of soil and underdrains	Water-bearing capacity
Upper Basin	Tipton, etc. Plymouth	Lower Tipton	1,400	Upper 1,000 feet, sandstone, shale, and siltstone; lower 400 feet, sandstone, shale, and siltstone, with thin layers of limestone.	Very light to medium; very porous; water-bearing capacity is high.	Highly permeable; water-bearing capacity is high.
		Upper Tipton	1,400	Upper 1,000 feet, sandstone, shale, and siltstone; lower 400 feet, sandstone, shale, and siltstone, with thin layers of limestone.	Very light to medium; very porous; water-bearing capacity is high.	Highly permeable; water-bearing capacity is high.
		Lower Tipton	1,400	Upper 1,000 feet, sandstone, shale, and siltstone; lower 400 feet, sandstone, shale, and siltstone, with thin layers of limestone.	Very light to medium; very porous; water-bearing capacity is high.	Highly permeable; water-bearing capacity is high.
Middle Basin	Plymouth	Upper Plymouth	1,400	Upper 1,000 feet, sandstone, shale, and siltstone; lower 400 feet, sandstone, shale, and siltstone, with thin layers of limestone.	Very light to medium; very porous; water-bearing capacity is high.	Highly permeable; water-bearing capacity is high.
		Lower Plymouth	1,400	Upper 1,000 feet, sandstone, shale, and siltstone; lower 400 feet, sandstone, shale, and siltstone, with thin layers of limestone.	Very light to medium; very porous; water-bearing capacity is high.	Highly permeable; water-bearing capacity is high.
Lower Basin	Plymouth	Upper Plymouth	1,400	Upper 1,000 feet, sandstone, shale, and siltstone; lower 400 feet, sandstone, shale, and siltstone, with thin layers of limestone.	Very light to medium; very porous; water-bearing capacity is high.	Highly permeable; water-bearing capacity is high.
		Lower Plymouth	1,400	Upper 1,000 feet, sandstone, shale, and siltstone; lower 400 feet, sandstone, shale, and siltstone, with thin layers of limestone.	Very light to medium; very porous; water-bearing capacity is high.	Highly permeable; water-bearing capacity is high.

Grade	Year-Sub-Grade	Component	Col. #	Component Description	Component Description	Component Description	Component Description
		Workshop Period	0.10	Workshop Period	Workshop Period	Workshop Period	Workshop Period
		Lecture	0.10	Lecture	Lecture	Lecture	Lecture
		Assignment/Exam	0.10	Assignment/Exam	Assignment/Exam	Assignment/Exam	Assignment/Exam
		Community	0.10	Community	Community	Community	Community
		Workshop/Exam	0.10	Workshop/Exam	Workshop/Exam	Workshop/Exam	Workshop/Exam
		Collaborative work	0.10	Collaborative work	Collaborative work	Collaborative work	Collaborative work

Workshop Period: This component involves hands-on learning activities, including group projects, case studies, and practical exercises. It is designed to reinforce theoretical concepts and develop problem-solving skills.

Lecture: This component consists of traditional classroom-style instruction. It covers fundamental concepts, theories, and models related to the course objectives. Lectures are supported by visual aids and interactive discussions.

Assignment/Exam: This component involves individual or small group assignments and examinations. It assesses students' understanding of the course material and their ability to apply concepts to new situations. Assignments are designed to be challenging and thought-provoking.

Community: This component focuses on building a sense of community and collaboration among students. It includes group discussions, peer reviews, and collaborative learning activities. The goal is to create a supportive and inclusive learning environment.

Workshop/Exam: This component combines hands-on learning with assessment. It involves practical exercises, group projects, and individual or group examinations. This component is designed to evaluate students' practical skills and their ability to work effectively in a team.

Collaborative work: This component emphasizes teamwork and collaboration. It involves group projects, case studies, and peer reviews. Students are encouraged to share their ideas, provide feedback, and learn from each other. This component is designed to develop students' communication and teamwork skills.

Workshop Period: This component involves hands-on learning activities, including group projects, case studies, and practical exercises. It is designed to reinforce theoretical concepts and develop problem-solving skills.

Lecture: This component consists of traditional classroom-style instruction. It covers fundamental concepts, theories, and models related to the course objectives. Lectures are supported by visual aids and interactive discussions.

Assignment/Exam: This component involves individual or small group assignments and examinations. It assesses students' understanding of the course material and their ability to apply concepts to new situations. Assignments are designed to be challenging and thought-provoking.

in the subsurface throughout most of the area. The Kaiparowits Formation is exposed around the east, south, and west sides of the Paunsaugunt Plateau, but thins rapidly in a northerly direction, extending only about to the middle of the upper Sevier River basin. The combined thickness of these formations ranges from about 500 to 2,300 feet (Gregory, 1951, p. 25; Marinc, 1963, p. 456-457).

The following lithologic description was derived largely from Gregory and Moore (1951), Gregory (1951), and Marinc (1963). The combined Straight Cliffs and Wahweap Sandstones consist mostly of massive to thin bedded sandstone which intergrades and laterally merges unconformably with massive but ranges from shale to shaly sandstone. The pre-Cambrian sandstone of the unit is tan to yellow tan and buff brown, fine to coarse grained, cemented initially by calcite and some iron oxides, and is mostly massive bedded, beds thicker than 10 feet predominating. The shale and shaly sandstone is tan to gray, usually argillaceous, carbonaceous, or siliceous, and thin bedded. In addition, irregular beds and lenses of conglomerate occur in the low formations. Coal is also present in the Straight Cliffs Sandstone, as described by Velt's and Robinson (1962, p. 24-26), Carpenter, Robinson, and Bjorndal (1963, p. 23-24), and Marinc (1963, p. 457, pl. 9C).

The Straight Cliffs Sandstone forms prominent steep-sided ridges and cliffs; the Wahweap Sandstone forms a group of steeply cliffs which are distinguishable from the cliffs of the Straight Cliffs Sandstone in some places but in other places combine with them to form a single slope interrupted by ledges.

The Kaiparowits Formation consists of dark gray, gray-green, yellow, and tan arkose sandstone which is medium to coarse grained and weakly cemented by calcium carbonate. The sandstone is highly variable, both horizontally and vertically, in bedding, texture, and composition. Beds range in thickness from several inches to less than 8 feet. The unit forms predominantly dark-gray weathering slopes interrupted by shaly benches.

Water-bearing characteristics

The best water bearing zones in the Upper Cretaceous formations are in the Straight Cliffs and Wahweap Sandstones. These zones contain the more permeable sandstone beds and also fractures in the sandstone beds. The Upper Cretaceous formations on the north end of the Paunsaugunt Plateau yield small quantities of water, generally less than 10 gpm (gallons per minute), to wells that range from about 100 to 310 feet in depth. Well (C-37-1)11111-1 in Bryce Canyon National Park, however, is 2,300 feet deep and yields about 250 gpm from the Straight Cliffs and Wahweap Sandstones (Marinc, 1963, p.

450). The depth to the Cretaceous formations in most of the upper Sevier River basin is too great for economical well construction.

Many small springs and seeps around the Painsaugunt Plateau yield water from the Upper Cretaceous formations. Only a few of these springs, however, discharge into the upper Sevier River basin. Maxine (1963, table 6, p. 461-466) listed 13 springs in the Bryce Canyon area which discharge from the Upper Cretaceous formations southeastward into the Paria River drainage. Recharge to the formations, however, is from the upper Sevier River basin. Of these 13 springs, 13 discharge from about 2 to 185 gpm from the Straight Cliffs and Wahweap Sandstone and 3 springs discharge unmeasured amounts from the Kaiparowits Formation. Several springs on the plateau, usually yielding 10 gpm or less, issue from the Kaiparowits Formation south of Tropic Reservoir, but the source of the water is probably from the local conglomerate of the overlying Wasatch Formation (Maxine, 1963, p. 492).

TERTIARY SYSTEM

WASATCH AND BRIAN HEAD FORMATIONS

WASATCH AND BRIAN HEAD FORMATIONS

General description

The Wasatch and Brian Head Formations are well exposed throughout much of southern Utah. Although both formations are distinct in appearance, a gradational zone between them makes separation difficult; hence, they have been mapped as an undifferentiated unit in previous reports (Gregory, 1913, 1956a).

The Wasatch Formation is one of the most widely exposed formations in the upper Sevier River basin. It forms prominent pink cliffs on the Martinsburg, Painsaugunt, and Table Cliff Plateaus and on the south ends of the Sevier and Aquarius Plateaus; in Bryce Canyon National Park it forms cliffs, spires, and columns. The formation thins rapidly to the north, ranging in thickness from 400 to 1,100 feet on the Painsaugunt and Table Cliff Plateaus to practically zero north of John's Valley subbasin (Gregory, 1914, p. 603-604; 1951, p. 44; Maxine, 1963, p. 456). It consists of thick-bedded pink to red fresh-water limestone which contains irregularly interbedded pink to yellow shaly limestone, shale, siltstone, sandstone, and conglomerate. At many localities the lowest part of the formation is a red massive calcareous basal conglomerate which is lenticular and discontinuous.

Gregory (1915, p. 107) described the Brian Head Formation as containing a lower unit of coarsely stratified fine-grained material and an upper unit of coarse agglomerate. The lower unit generally is

exposed in the same areas as is the Washock Formation, except that the lower unit generally has been "stripped" from the Permian-age plateau. In addition, the lower unit is well exposed on the northern end of the Mackaugert Plateau, north-west of Panguitch, where section has not yet exposed, and also the underlying Wasatch Formation. The thickness of the lower unit in the area reportedly ranges from 0 to nearly 1,000 feet (Gregory, 1944, p. 201; 1945, p. 111; 1949, p. 697-699; 1951, p. 36). It is composed of well-stratified siliceous limestone, impure sand, calcareous silt, shale, calcareous and shaly sandstone, calcareous sandstone and conglomerate, and magnesian, pyroclastic material of various types. The material is shades of white, gray, green, tan, and black. There is some evidence that the deposits included in the unit in the northern Mackaugert Plateau are a part of the Brian Head Formation. Determining the exact age of these deposits was beyond the scope of this investigation, however, and they were mapped as part of the Washach and Weiva Head Formations.

A typical form of the lower unit is an irregular, from scale to nearly the amount of volcanic debris and of grain size. In general, the unit forms cliffs, ledges, and steep slopes, or a cap on parts of the major plateaus. It also forms rounded hills along valley edges and often weathers into boulders.

The upper part of the Brian Head Formation is exposed on the west side and the lower end of Panguitch Valley and near Arroyo in both sides of Red Rock gorge River between the head of Black Canyon and the head of Klappa Canyon (p. 11) (Gregory, 1944, p. 201). Much of the unit is described in these zones, because they later prove to be part of the Bullion Canyon volcanic sequence. Part of the unit mapped as Series Brier Formation on the eastern margin of the Mackaugert Plateau between Panguitch and Tropic (p. 11) may be part of the upper unit of the Brian Head Formation. The upper part of the Brian Head thins to the north, reaching an estimated maximum of about 500 feet.

The upper unit is indistinctly upper and lower boundaries and is sometimes difficult to distinguish from the volcanic rocks with which it intergrades. It was described by Gregory (1945, p. 103; 1949, p. 693) as "dark gray, remarkably coarse agglomerate." The conglomerate in Black Canyon was further described by Gregory (1944, p. 205) as being "roughly angular, but very poorly sorted and including many thin lenses of thin bedded, medium grained sandstone." Except for the outcrop in Black Canyon, much of the dark-gray agglomerate assigned by Gregory to the upper unit or several plates (Gregory, 1944, p. 204; 1949, p. 103; 1949, p. 693) probably is part of the Henry volcanic rock series. Most of these intergrade volcanic sandstone and are probably pyroclastic debris belonging to the

Doger Park Basaltic Breccia. In this report the upper unit of the Brian Head Formation is considered to include primarily only sandified and apparently water-laid tuff's sands, conglomerate and sandstone deposits of volcanic origin. The upper unit is believed to crop out north of Kingston Canyon but is interbedded with volcanic rocks of Tertiary age; therefore, it is not differentiated on the geologic map (Pl. 15). The upper unit is expressed topographically in narrow, unfanned, and unconsolidated conglomerate ledges and weathering sandstone slopes.

Water-bearing characteristics

The Wasatch and Brian Head Formations both contain water-bearing zones that consist mainly of fractures and joints in otherwise impermeable slates or thin, porous strata within the sandstone and conglomerate beds. In addition, the Wasatch Formation contains large quantities of water in solution cavities in limestone beds, and the upper unit of the Brian Head Formation in Black Canyon near 4,000 feet large quantities of water from fractures and joints in all the contact between the conglomerate and interformational volcanic flows.

The Wasatch and Brian Head Formations yield small quantities of water to wells, chiefly in Glen Fork Valley and so on. These wells mainly produce less than 25 gpm, mostly from the Wasatch Formation, and are generally less than 150 feet deep. Wells penetrating the upper unit of the Brian Head Formation near Antimony produce from about 4 to 25 gpm.

These formations also are the sources of many springs. Springs in the Wasatch Formation in the eastern Markagunt Plateau normally discharge from 25 to 1,500 gpm from solution channels in limestone; Mazonoth Springs (10-38-2781.1ac) has discharged as much as 191,300 gpm (Wilson and Thomas, 1904, fig. 15). The Wasatch Formation elsewhere in the area generally yields less than 100 gpm to springs.

The lower unit of the Brian Head Formation yields several quantities of water (generally less than 25 gpm) to a few springs and seeps. The upper unit yields water to a few springs in Black Canyon and near Antimony. Most of the springs in Black Canyon occur in the contact of fractured intraformational volcanic rocks and underlying relatively impermeable conglomerate and sandstone. These springs discharge from 50 to more than 1,000 gpm. Large quantities of mineral issue from contact zones between volcanic rocks and relatively impermeable conglomerate in the upper unit at the head of Antimony Creek. These springs are largely responsible for the consistent base flow of the creek, about 10 cfs (cubic feet per second).

MIOCENE(?) AND PLEISTOCENE(?) SERIES

VOLCANIC ROCKS

General description

Volcanic rocks of Miocene(?) and Pliocene(?) age compose the bulk of the Fish Lake, Awapa, Aquatics, and Sevier Plateaus, the southern Thacher Mountains, and the highlands of the northern Markagunt Plateau between Panguitch and Circleville Canyon (pl. 1). These rocks include two separate formations: the Bullion Canyon Volcanics of Miocene(?) age, exposed mainly north of Circleville and Kingston Canyons, and the Tiger Park Basaltic Breccia of Pliocene(?) age, exposed in the remainder of the area. The Bullion Canyon Volcanics overlap and interfingers with the upper unit of the Basin Head Formation north of Kingston Canyon and is possibly conformable to it. In fact, much of the upper Basin Head unit described in Black Canyon and near Antimony may later prove to be part of the Bullion Canyon volcanic sequence. Elsewhere the Bullion Canyon Volcanics and the Tiger Park Basaltic Breccia are probably unconformable on the underlying rocks. The combined volcanic rocks in the northern part of the upper Sevier River basin are more than 4,000 feet thick (Willard and Callaghan, 1962) and are estimated to range in thickness from 2 to a few hundred feet in the southern part of the basin.

According to Willard and Callaghan (1962), the Bullion Canyon Volcanics "consists of a thick series of lentic breccias, tuffs, and thin flows; at the base, a succession of lava and quartzite flows with thin intervening beds of volcanic breccia, and more silica flows and breccias at the top." The Tiger Park Basaltic Breccia is described (Callaghan and Parker, 1962) as "a breccia composed of fragments and matrix of basaltic andesite."

Topographically, the volcanic rocks form caps, jagged cliffs, ledges, and rocky slopes on most of the major plateaus and mountains and underlie foothills on the valley sides.

Water-bearing characteristics

Water-bearing zones consist of fractures and joints which occur irregularly; therefore, ground-water conditions in the volcanic rocks in any single locality are unpredictable. In most places these rocks impede the movement of ground water. Only one well, (C-37 J) shown 1, is known to produce water from the volcanic rocks; this well reportedly yields 90 gpm.

The volcanic rocks yield water to many springs, the largest being Blue Springs, (C-28-1) (pl. 2), which produced about 1,400 gpm in July 1962. Other springs issuing from volcanic rocks of Tertiary age yield from less than 1 to more than 900 gpm.

INTRUSIVE ROCKS

General description.

Intrusive rocks of Tertiary age are exposed at the north end of Panguitch Valley basin near the head of Circleville Canyon (pl. 1). Because they appear to have intruded the upper unit of the Blinn Head(?) Formation of Miocene (?) age and are overlain by the Roger Park Headline Formation of Pliocene (?) age, the intrusive rocks probably are of Miocene (?) age.

The intrusive rocks consist of quartz monzonite and quartz monzonite porphyry, are light to medium gray, and are finely to coarsely crystalline. They form steep-sided, rably to smooth slopes, cliffs, and highly jointed ridges.

Water-bearing characteristics.

The intrusive rocks are compact and homogeneous and are not fractured; therefore, they are poor aquifers. In fact, they form a barrier to ground-water movement at the lower end of Panguitch Valley basin and are largely responsible for the marshy conditions there.

TERTIARY OR QUATERNARY SYSTEMS

UPPER Eocene OR LOWER PLIOCENE DEPOSITS

SEVIER RIVER FORMATION

General description.

The Sevier River Formation of late Pliocene or early Pleistocene age is exposed at the upper Sevier River basin only as relatively small, isolated to semiconnected deposits in the south end and on both sides of Panguitch Valley basin (pl. 1). It is a valley fill deposit, consisting primarily of old alluvial fans, and, therefore, it is similar in most respects to, and is usually difficult to distinguish from, deposits of lower alluvium. (The phrase "valley fill," as used in this report, includes all alluvium, fans(?) or mesas(?) deposits, boulder deposits, and the Sevier River Formation.) The Sevier River Formation can be differentiated by outcrop, however, by (1) topographic form, (2) excessive dolomite, or reversed dip of bedding planes (Willard and Callaghan, 1963), (3) a generally poorer degree of sorting and stratification, (4) a generally greater degree of consolidation, (5) bedding within the formation (Callaghan and Parler, 1963b), and (6) the presence of lacustrine deposits similar to those in the type area of the formation near Sevier, Utah (Callaghan, 1968, p. 101, and Callaghan and Parler, 1968).

The Sevier River Formation is believed to underlie much of the surficial Quaternary and Recent alluvium in the southern part of Panguitch Valley basin (pl. 1, section D-2'). Gregory (1949, p. 687 and

pl. 1) mapped exposures of the formation on the west side of Pangulch Valley between Pangulch and Darsh, but each of these may be the remnant of the Brian Head Formation. No attempt was made in this study to alter Gregory's (1919) mapping of the Sevier River Formation.

The Sevier River Formation is generally unsorted and mostly buried by younger alluvium; therefore, the thickness of the formation cannot be determined from outcrops. Study of outcrops, dated logs, and logs of test holes in Pangulch Valley basin, however, indicates thicknesses ranging from 3 to more than 450 feet. The formation generally is poorly sorted and poorly stratified valley fill which consists of unconsolidated to partly consolidated cobbles, pebbles, sand, silt, and clay deposited as alluvial fans. It also contains lacustrine deposits of silty clay, and argillaceous fine-grained beds which are thin lens, gastropods, shaly nodules, and mammalian (?)

fragments. In the Sevier River Formation there are rounded hills, isolated to semi-isolated bluffs, fans, and terraces in some, and long "truncated" deposits which were dissected from old alluvial fans by recent streams.

Water-bearing characteristics

The lack of sorting and stratification and the abundance of silt in the Sevier River Formation generally results in low permeability. The best water-bearing zones are lenses of well-sorted sand and gravel that contain little silt. These lenses do not yield much moderate amounts of water to domestic and stock wells. Reported yields from wells generally range between 10 and 50 gpm.

The formation yields water to gardens and crops in an area about 2½ miles west of Red Crown, the largest of which, Meyer Springs, (C 36 3125-6), flows about 450 gpm. Yields of other springs range from less than 1 to about 10 gpm.

QUATERNARY SYSTEM DEPOSITIONAL AND SEDIMENT SERIES

ALLUVIUM

General description

Basal flows of Quaternary age cover large areas of the Markagunt Plateau and more or less isolated flows along the east side of Pangulch Valley and on the northern Barnsavage Plateau near the entrance to Red Canyon (pl. 1). The estimated thickness of the basal on the Markagunt Plateau ranges from 3 to 1,200 feet (Gregory, 1906a, p. 26). The flow on the Barnsavage Plateau was estimated to be less than 100 feet thick.

The basal flows of the Markagunt Plateau probably may consist in part of massive crystalline and porphyritic olivine or hornblende basalts (Gregory, 1940, p. 395). The basalt is black, dense, often vesicular and sometimes in many places it displays well-developed flow structures. The basalt flows exhibit a strong columnar jointing.

Water-bearing characteristics

The basal flows of the Markagunt Plateau were described as being "so permeable enough that they can absorb the water of a recent clean hard freeze or maximum snowmelt without runoff" (p. 397) but "so impermeable to have effect on location of water movement along low permeable valleys" (p. 397) since the drainage is now achieved by channels in the Quaternary (Wasatch) beneath the basalt (Wilson and Thomas, 1934, p. 19). The basalt is not known to yield water in wells or large springs on the Markagunt Plateau, but it does yield water to many seeps and small springs.

The basal flows of the Markagunt Plateau have also arrested the flow of many surface streams, and thereby formed channels and new drainages. Many of these streams have by-passed the damming effect by the oblique solution channels in the underlying Wasatch Formation. These solution channels yield large quantities of water to streams that swirl in ascending the base flow of the Sevier River above Hatch (Wilson and Thomas, 1934, p. 25).

Quaternary deposits

Quaternary gravels

Gregory (1940, p. 381, 391) made a total picture of units and layers of igneous gravel on the northern edge of the Markagunt Plateau. He mapped this material in two areas (1940, pl. 1) as "Quaternary igneous gravel." Similar deposits on the north end of the Markagunt Plateau and on the south end of the Sevier Plateau (pl. 1) were mapped during this study. These deposits and one of the deposits mapped by Gregory are shown on the geologic map as "Quaternary igneous rubble" because most of the material is angular. The second deposit mapped by Gregory as igneous gravel near the north boundary of the map is believed to be part of the Tertiary volcanic rocks. The deposits mapped during this study cover the slopes, top, most of the ridges, and some high extended deposits in the vicinity of Castle Cliff. This material is not considered to be part of the valley fill.

The type of the igneous rubble is unknown, but the rubble probably was deposited during an outburst of erosion and deposition similar to that described by Wilbur and Callaghan (1927). It thus may be as old as or older than the Sevier River formation, which is of late Pleistocene or early Pliocene age. However, instructions have to be

evidence that the rubble is of Pliocene age, the formation is here assumed to be of Quaternary age, although parts of it may be older.

The thickness of the igneous rubble ranges from 0 to more than 100 feet and averages about 25-50 feet. The outcrop of the formation are quite uniform and consist of poorly sorted and poorly stratified boulders, cobbles, pebbles, sand, and silt. The larger fragments are generally angular to subangular. The rubble is composed almost entirely of volcanic rock fragments similar to the Roger Park Basaltic Breccia. In many areas the rubble is about 5 percent box-shaped to oblate boulders and cobbles of white and narrow banded quartzite. This quartzite is foreign to the upper Sevier River basin, and its source is unknown.

The Quaternary igneous rubble forms hummocky and rubbly masses which occupy interstream benches and slopes of the drainage of the Sevier and Panamint Plateaus.

Water-bearing characteristics

The Quaternary igneous rubble probably is not a good water-bearing formation because it contains abundant silt and coarse sorting. It is not known to yield water to wells or large springs in the area.

LANDSLIDE DEPOSITS

Two small landslides are shown on the geologic map (pl. 1). One slide is several miles east of Otter Creek Reservoir and the other is about 8 miles southeast of Greenwich. The slides have a combined area of less than 3 square miles and are composed of a heterogeneous mass of material that has moved downslope from the face of the Awapa Plateau. The maximum thickness of these deposits probably is more than 300 feet. The landslides are not important water-bearing units because of their small areal extent and poor sorting.

ALLUVIUM

The alluvium in the upper Sevier River basin was subdivided into three mappable units—old alluvium, young alluvium, and flood-plain deposits. The old alluvium, which is exposed only in Panguitch Valley basin, generally is distinguishable from the young alluvium only on the basis of topographic expression. It consists of old dissected alluvial fan remnants which are topographically higher than present young alluvial fans. All alluvium elsewhere in the basin other than flood-plain deposits is shown on the geologic map as young alluvium, even though much of this alluvium may be equivalent to the old alluvium. "Flood-plain deposits," as used in this report, refers to sediments deposited in the present flood plains of the Sevier River, the East Park Sevier River, and Otter Creek. The old alluvium is

similar to the young alluvium in water-bearing properties. The flood-plain deposits constitute the best aquifers in the alluvium, but they are lenticular and discontinuous and interfingery with the other alluvial deposits in the subsurface. Therefore, although the three units are shown separately on the geologic map, they are discussed as a single hydrogeologic unit in this report.

Old and young alluvium

The old alluvium (pl. 1) is exposed only in Panguitch Valley basin as isolated bluffs and terraces like forms or outliers 75-100 feet high on the valley sides as large scalds and small fans whose apices are being stripped away by the Sevier River, and within side canyons as small remnant burging terraces. Its topographic form is similar to that of the Sevier River Formation, which is also exposed in the valley, and the old alluvium may be equivalent in age to the Sevier River Formation. Much of the material underlying the outcrops of young alluvium in all the major valley basins probably is equivalent in age to the old alluvium.

The young alluvium includes alluvial-fan sediments in the valley basins and alluvium in meandering tributary valleys. Lake(?) or marsh(?) deposits, not exposed in the upper Sevier River basin, but punctuated by test holes and wells in Kanevram subbasin, are assigned to the young alluvium in this report, even though they are technically not of alluvial origin.

Both the old and young alluvium generally consist of interbedded, lenticular, and interfingery deposits of cobbles, pebbles, sand, silt, and clay. The pebbles and sand range in size from very fine to very coarse and contain small to large amounts of silt and clay. Sorting and stratification range from poor to moderately good. The most permeable water-bearing zones in the old and young alluvium are the gravel and sand beds which have been deposited in stream channels in alluvial fans.

The lake(?) or marsh(?) deposits identified only in the subbasins of Kanevram subbasin interfingery with and underlie the alluvial-fan sediments of the subbasin. They consist of regularly interbedded light- or blue-gray carbonaceous silt and clay and sand and pebbles. Some of the silt and clay beds contain fossil gastropod and pelecypod shells. The lake(?) or marsh(?) deposits were penetrated by test holes (C-27-1)Sena-2, (C-27-1)Hela 1, and (C-27-1)27baw 1 (Folbie and Robinson, 1934, p. 27-31).

The thickness of the combined old and young alluvium ranges from 60 to more than 200 feet in the upper Sevier River basin.

Fluvial-plain deposits

Fluvial-plain deposits, as shown on plate 1, consist of channel and oxbow deposits within the incised floor plains of the Sevier River, the East Fork Sevier River, and Otter Creek. Outcrops of the unit are well isolated from the old and young alluvium only by benches, and all deposits exposed within the present flood plains of these streams are classified as fluvial-plain deposits. Channel deposits generally are well-sorted and well-sorted sand and gravel which contain little silt, whereas oxbow deposits generally are sand, silt, and clay. Although ancient fluvial-plain deposits extend in the subsurface beyond the present flood plains of the three streams, their full extent is not known owing to the irregular character of the basin.

The maximum known thickness of the ancient and present fluvial-plain deposits is about 200 feet in Fargue's Valley north, about 100 feet in Otter Valley north, about 75 feet in East Fork Valley east, and about 170 feet in Goose Valley north (Fells and Robinson, 1932).

Water-bearing characteristics

The stratum is the principal aquifer in the upper Sevier River basin, and it yields small quantities of water to wells and springs. The main water-bearing beds are sand and gravel. The extent and richness of the alluvium is described in the section "Valley under" (p. 26), and water-bearing characteristics of the alluvium in much fuller measure are discussed in detail in the section "Ground-water distribution in the basin" (p. 31).

STRUCTURE

The major structural features of the upper Sevier River basin include (1) a prevailing north-south dip of both surface and subsurface strata in the basin plains (Gregory, 1921, p. 78), (2) two great faults of large displacement, the Sevier and Furgerson faults (pl. 1), which are the chief causes of continuous and nearly straight exposures in the area, and (3) three great and well-south-tipped faces of several plateaus and separated by depressions or basins which parallel the strike (Ferguson, 1925, p. 105).

REGIONAL DIP AND TOWNS

Most formations in the province in the upper Sevier River basin have a regional dip of $2-5^{\circ}$ N., NE., and E. (Gregory, 1921, p. 80; Brown, p. 108; 1931, p. 75-76). This regional dip is generally uniform, and despite the presence of dips in southerly or westerly directions generally due to local faults (see pl. 2). According to Gregory (1921, p. 78), the regional dip played a minor role in the present upland orientation of the upper Sevier River basin, not only

as a control to surface drainage, but to great exposure of formations as well. The regional dip also noticeably controls the movement of ground water in bedrock aquifers. For example, water moves down the dip through a permeable basalt that overlies an impervious conglomerate in the upper unit of the Hoven Head Formation and discharges through springs at the base of the fault along the west wall of Black Canyon.

Prominent or large scale folding is noticeable in the upper Sevier River basin. Small local folds are merely small deviations in the regional dip and usually occupy less than 1 square mile. They are of little significance in the structure of the area. A typical small fold in the Johns Valley anticline, 5 miles south of Wolfson (pl. 1).

FAULTS

The Sevier and Panguitch faults delineate the major valleys and plateaus in southern and central Utah. These two north-south-trending master faults are parallel and about 16-26 miles apart.

The Sevier fault is a normal fault, the downdropped block being on the west, and it forms the boundary between the Sevier and Panguitch Plateaus and the Panguitch and Circle Valley basins (pl. 1). The fault can be traced from northern Arizona to the upper end of Sargents Valley in central Utah (Thornsbury, 1931, p. 209; Gregory, 1931, p. 74-76). The throw along the Sevier fault, within the upper Sevier River basin ranges from 500 to about 2,000 feet and varies greatly within these distances (Gregory, 1931, p. 76). The fault generally is marked by a prominent scarp or fault-line escarp, the northern side forming the scarp.

The Panguitch fault is also a normal fault, the downdropped block being on the west. It can be traced from near the southern boundary of Utah, through the upper Sevier River basin to near the Fish Lake Plateau. It forms the boundary along the eastern edge of the Panguitch Plateau and, farther north, the boundary between the Table Cliff, Aquarius, Awapa, and Fish Lake Plateaus and the East Rock Valley and Grass Valley basins (pl. 1). The throw of the fault is mostly between 600 and 2,000 feet (Gregory, 1931, p. 77), and it becomes 3,000 feet along the Aquarius Plateau (Gregory, 1931, p. 80). Like the Sevier fault, its displacement varies greatly within short distances. The Panguitch fault generally is not as well expressed in the topography as the Sevier fault. The Panguitch fault generally lies in the foothills at a distance from the plateau; it is often covered by alluvium and in places displays topographic inversion, the downthrown block forming the plume.

Many other faults, shorter and having smaller displacements than the Sevier and Panguitch faults, occur in the highlands and foot-

hills of the upper Sevier River basin. Many of these faults parallel the two major fractures and lie in close proximity to them (pl. 4). Apparently the two master faults controlled the formation and orientation of the smaller faults.

VALLEY BASINS

Faulting, erosion, and deposition by streams have shaped the several ground-water basins in the upper Sevier River basin. The valley fill in these basins has been derived from the consolidated and unconsolidated formations in the uplands that surround the valleys. In Circle and Geneva Valley basins all the sediments are derived from volcanic rocks; in Panguitch and East Fork Valley basins, the sediments are derived from both volcanic and sedimentary rocks. The word "alluvium," as used in the following discussion, includes old alluvium, young alluvium, and flood-plain deposits. A description of the physiographic elements of the valley basins is given in the section "Physiography" (p. 7).

PANGUITCH VALLEY BASIN

Panguitch Valley basin is the segment of the upper Sevier River basin between the mouth of Mammoth Creek and the head of Circleville Canyon (pl. 1). It includes an area of about 75,000 acres. The basin is bounded on the south by sedimentary rocks which consist the valley, on the west by sedimentary and volcanic strata which descend from the eastern and northeastern Wasatchian Plateau and continue beneath the valley fill (pl. 1, sections A-A' and B-B'), on the east by the Tennessean and Sevier Plateaus and the Sevier fault, and on the north by sedimentary and igneous rocks. The Sevier fault is more responsible for the presence of Panguitch Valley basin than any other structural element. A maximum known thickness of 833 feet of valley fill, all of which is alluvium, was penetrated by test hole (C-33-5)13333-1 (O'Neil and Robinson, 1943, p. 14) in the northeastern part of the valley.

Panguitch Valley basin is separated from Circle Valley basin downstream by a constriction of volcanic rock between the Sevier Plateau and the southern Tushar Mountains. The Sevier River flows through this constriction in a steep-sided gorge about $5\frac{1}{2}$ miles long and about 100-800 feet wide called Circleville Canyon (pl. 1, geologic map and section D-D').

CIRCLE VALLEY BASIN

Circle Valley basin includes about 14,000 acres, and it occupies the area between the mouth of Circleville Canyon and the beltrock constriction west of Kingston (pl. 1). The basin was formed by an extension faulting in the surrounding volcanic rocks. It is bounded on the west by the southern Tushar Mountains and on the east by the

Serier Plateau. A constriction formed by volcanic rock at the north-west corner of the basin separates Cirra Valley basin from the central Serier Valley downstream (Young and Carpenter, 1963). A maximum known thickness of 660 feet of valley fill, all of which is alluvium, was penetrated by test hole (C-33-2)3324b - near the center of the basin.

EAST FORK VALLEY BASIN

East Fork Valley basin is the basin between Tropic Reservoir and the upper end of Kingston Canyon (pl. 1). The basin is subdivided into three subbasins by two bedrock constrictions, one formed by Fluke Mountain and the other by the rock at the lower end of Johns Valley subbasin (see pl. 1, section E-E').

EMERY VALLEY SUBBASIN

Emery Valley subbasin, between Tropic Reservoir and Fluke Mountain, includes an area of about 12,000 acres. Part of the subbasin is bounded on both sides by faults (pl. 1), along which the subbasin was uplifted; a horse was thus formed, which has since been eroded to form the present valley. The subbasin is bounded at its southern end and on its eastern and western sides by sedimentary bedrock and at its northern end by volcanic and sedimentary rocks. A maximum known thickness of 66 feet of valley fill, all of which is alluvium, was penetrated by well (7-36-1)36a-1 in the south-central part of the subbasin.

JOHNS VALLEY SUBBASIN

Johns Valley subbasin, between Fluke Mountain and the head of Black Canyon, includes an area of about 63,000 acres (pl. 1). It is bounded at its southern end by the volcanic-rock constriction formed by Fluke Mountain, on its western side by sedimentary and volcanic rocks of the Serier Plateau, on its eastern side by sedimentary and volcanic rocks at the Table Cliff and Aquarius Plateaus, and at its northern end by bedrock at the head of Black Canyon. The Pennsylvanian fault separates the valley from the Table Cliff and Aquarius Plateaus along much of the eastern valley margin and is the main structural element forming the subbasin. Several other faults are in the subbasin, one along the western side and one assumed at depth beneath the valley floor. A maximum known thickness of 360 feet of valley fill, all of which is alluvium, was penetrated by test hole (C-33-2)3324d-1 in the central part of the subbasin.

Johns Valley subbasin is separated from Antimony subbasin downstream by a bedrock constriction between the Aquarius and Serier Plateaus. The East Fork Serier River flows through the constriction in Black Canyon, a steep-sided gorge about 8 miles long, 100-400 feet wide, incised in sedimentary and volcanic bedrock (pl. 1, geologic map and section E-E').

ARTIMONY SUBBASIN

Artimony subbasin includes an area of about 6,000 acres between the mouth of Black Canyon and the head of Kingston Canyon (pl. 1, geologic map and section *Z-Z'*). It is a small valley bounded at its southern end by the entrance of Black Canyon, on its western side by volcanic and sedimentary rocks of the Sevier Plateau, on its eastern side by eastward-dipping sedimentary and volcanic rocks of the Aquarius Plateau, and at its northern end by junction with the Grass Valley basin and the bedrock at the head of Kingston Canyon (pl. 1). This subbasin, like Jones Valley subbasin, is due largely to the Panamintum fault, which extends several miles east of the valley and separates it from the Aquarius Plateau (pl. 1). A maximum known thickness of 261 feet of valley fill, all of which is alluvium, was penetrated by test hole (C-31) 21236-1 in the central part of the subbasin.

Downstream from Artimony subbasin, the West Fork Sevier River flows through the Sevier Plateau in Kingston Canyon, a narrow, steep-sided gorge, approximately 2 miles long, 100 feet to half a mile wide, incised in sedimentary (?) and volcanic rock.

GRASS VALLEY BASIN

Grass Valley basin is between the low topographic divide 2 miles north of Koshareau Reservoir and the Oiler Creek terrace plain near the head of Kingston Canyon (pl. 1). The low topographic divide at the north end of the basin separates the Oiler Creek drainage from the central Sevier Valley to the west and north. Grass Valley basin is divided into two subbasins by a bedrock constriction about 5-6 miles south of Greenweld (pl. 1, section *Z-Z'*).

KOSHAREAU SUBBASIN

Koshareau subbasin includes an area of about 30,000 acres between the low topographic divide north of Koshareau Reservoir and the bedrock constriction south of Greenweld (pl. 1). It is bounded by the volcanic rocks of the Sevier Plateau on the west and the volcanic rocks of the Anxpa and Fish Lake Plateaus on the east (pl. 1). The subbasin is a graben valley between the Panamintum fault on the east and an unnamed fault on the west. A maximum known thickness of 770 feet of valley fill, most of which is alluvium, was penetrated by test hole (C-22) 19736-1 near the central part of the subbasin.

ANGLE SUBBASIN

Angle subbasin includes an area of about 20,000 acres between the bedrock constriction south of Greenweld and Oiler Creek terrace plain, which is near the junction with Artimony subbasin and the head of Kingston Canyon (pl. 1). It also is a graben valley, bounded on

the east by the Awapa Plateau and the Panguitch fault, and on the west by the Sevier Plateau and an unnamed fault. Several large outcrops of volcanic rock within the subbasin define a north-south-trending canyon valley III (pl. 1, section A-A'). A maximum known thickness of 400 feet of valley fill, all of which is alluvium, was penetrated by well hole (C 26 2)66666 near Angles.

WATER RESOURCES

HISTORY OF WATER-RESOURCE DEVELOPMENT

Irrigation began in the upper Sevier River basin in the early 1890's when the first white settlers constructed diversion dams on some of the larger streams. Surface-water development reached its maximum in about 1920 (Woolley, 1947, p. 155).

Development of ground water in the basin began at about the same time as surface-water development but was limited mainly to the use of springs for public supply and irrigation. The first wells were constructed in about 1900 and the number has steadily increased to about 800. Most of the wells are used for domestic and stock supply, and periods of drought have increased interest in the possibilities of using additional acres from wells for irrigation.

Conflicts over water rights in the Sevier River system have occurred continually since the 1850's, mostly during drought periods. These controversies have resulted in many court decrees, including the Cox Decree of 1938 (Cox, 1938), which is used by the Utah State Engineer to distribute the water of the Sevier River system to the water users.

In the Cox decree, water rights pertaining to ground water are mostly for springs, but rights for a few ditches and wells are also listed. The decree made little mention of wells in the upper Sevier River basin because it was assumed that unappropriated ground water was not available for additional appropriation. This assumption has persisted and has been an important factor in deterring large-scale development of ground water. The rights in the decree concerning wells specify only use for irrigation. Water rights for many domestic, stock, public-supply, and industrial wells and some irrigation wells that are not listed in the decree are in the files of the State Engineer.

SURFACE WATER

The source of all streams in the upper Sevier River basin is precipitation within the basin. Most of the surface flow that leaves the basin is in the Sevier River and its largest tributary, the East Fork Sevier River. These streams carry about 1/2 mile north of the basin near Kingston. Some water also leaves the basin in irrigation canals

near Kingston and by a transmontane diversion from a point on the East Fork Sevier River below Tropic Reservoir eastward to Durin Valley in the Colorado River basin.

Surface water is stored in several reservoirs in the basin and is diverted from the river and its main tributaries by many canals. The Sevier River, its tributaries, reservoirs, and canals are discussed in the following pages.

THE SEVIER RIVER AND ITS TRIBUTARIES

The Sevier River above Kingston (locally called the South Fork Sevier River) drains about 1,110 square miles. The chief water-yielding areas are high in the Markagunt Plateau near Cedar Breaks National Monument and Navajo Lake (pl. 9), and the main stem of the river is formed by the merging of Assay and Munsutti Creeks south of Hatch. As the river flows northwest through Panguitch Valley and Circle Valley basins, it receives water from many tributaries and from ground-water discharge, part of which was originally water diverted for irrigation upstream.

The monthly flow of the Sevier River at three stream-gaging stations for the period 1923-32 is shown in figure 4, and the locations of the gaging stations are shown on plate 2. The Sevier River at Hatch had an average annual flow of 96,800 acre-feet for 40 years of record (1911-28, 1932-62); the Sevier River near Circleville, 17,166 acre-feet for 28 years of record (1914-23, 1923-24, 1940-62); and the Sevier River near Kingston, 94,121 acre-feet for 48 years of record (1917-62).

Both gains and losses have been recorded in the flow of the Sevier River between Hatch and Circleville and between Circleville and Kingston (fig. 4). The gains occur mainly during the nonirrigation season when little water is diverted from the main stream or its tributaries. The losses occur mainly during the growing season when much water is diverted for irrigation. The gains and losses in streamflow are discussed in greater detail on pages 40-43.

Near Kingston, the Sevier River unites with the East Fork Sevier River, its largest tributary, which drains both East Fork and Grass Valleys. The East Fork Sevier River originates high on the south end of the Panguitch Plateau. Otter Creek, whose source is high on the Fish Lake Plateau, is the chief tributary of the East Fork Sevier River, and it drains Grass Valley. Data for the principal perennial tributaries of the Sevier River, the East Fork Sevier River, and Otter Creek are listed in table 2. Some of these tributaries are perennial only in their upper reaches, and flow reaches the main stream only during periods of high runoff.

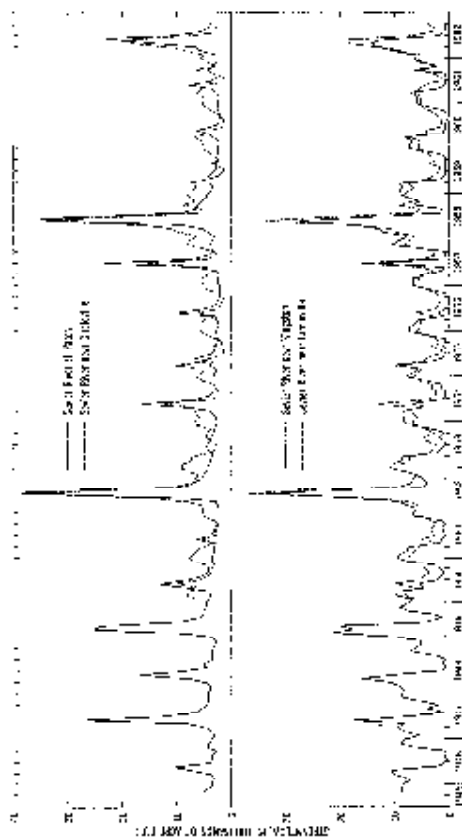


FIGURE 4.—Hydrographs of Sateh River flows for March, near Clatsville, and near Klappan.

TABLE 2. Data for the principal geophysical balances of the Sevier River and Five Sevier River and Glen Creek.

Stream catchment area	Sevier basin area, sq. miles	Area under irrigation and other uses, sq. miles	Area under irrigation and other uses, sq. miles	Perennial flow, cfs.	Annual precipitation, inches	Annual precipitation in mm.	Annual precipitation in mm.
Sevier basin							
Sevier River (main stem)	18	13	21,900	1934-59	20	4,900	Several small reservoirs along the main stem, One small reservoir in Troy, Utah, Creek.
Small Reservoir Army Creek	22	30	88,300	1910-13	14	4,300	Two reservoirs, Storage in Tropic, Utah, and in Tropic, Utah, Lake.
Marysville Creek	25	3	20,000	1917-18	8	100	One reservoir, One small reservoir.
Tropic Creek	2	28	2,800	1914-23	8	120	Several reservoirs.
Bandy Creek	2	21	1,300	1914-23	8	120	Several reservoirs.
Garfield Creek	22	48	3,000	1914-23	2	220	Several reservoirs.
Four Creeks	15	20	3,000	1914-23	2	370	Several reservoirs and small reservoirs.
Glen Valley basin							
Troy Creek	11	48	4,500	1914-23	1	32	One reservoir, One small reservoir.
Two, Dept. Survey Time	75	1,200	28,500	1914-23	10	32	Many reservoirs and small reservoirs.

WATER RESOURCES

Kurt Lark Research:		Kurt Lark Value Book:		Kurt Lark Value Book:		Kurt Lark Value Book:		Kurt Lark Value Book:	
16	7,700	1	200	One division, shares in					
14	2,100	2	\$,500	One division, shares in					
8	1,000	1	600	Plus Lark Research					
8	1,200	1	200	One division					
8	1,200	2	200	One division, One small					
6	1,100	1	200	division					
4	1,800	1	5	100					
7	1,100	1	5	200					
7	1,800	1	5	Several shares, One					
12	8,200	1	5	small, several					
12	8,200	1	5	Several divisions					
14	4,200	4	638	Several divisions, One					
10	8,000	2		small, several					
6	8,800	1		Several divisions, One					
6	8,800	1		small, several					
12	2,800	1	12	One division					
8	800	1	92	Several divisions					
15	10,000	15	1,978	Several divisions, One					
24	9,100	2	98	One division					
48	10,800	15		Many divisions and					
				several shares					

TABLE 2.—Data for 17 principal groups of boulders of the Nevada River, West Park Slide, Ribbon and Old-Father Creeks—Cont.

Principal group name	No. of boulders found	No. of boulders found in main crevasses	No. of boulders found in main crevasses	No. of boulders found in main crevasses	No. of boulders found in main crevasses	No. of boulders found in main crevasses	No. of boulders found in main crevasses	No. of boulders found in main crevasses	No. of boulders found in main crevasses	No. of boulders found in main crevasses	No. of boulders found in main crevasses	No. of boulders found in main crevasses	Group size class		Remarks	
													1, 100	Ext. noted		
Other Creek:																
Doody Hole Creek	3	11	1, 100	Ext. noted	1	760										One diversion over 500 ft. diversion from adjacent drainage into creek.
London Canyon (cont.)																
Doody Creek	1	20	1, 100	Ext. noted	1	1, 320										Several diversions.
Boyer Creek	4	21	1, 100	Ext. noted	4	423										Several diversions.
Knothaven Creek	11	21	1, 100	Ext. noted	6	75										One diversion.
Greenwich Creek	10	21	1, 100	Ext. noted	1	260										Several diversions.
Box (Boyer) Creek	12	21	1, 100	Ext. noted	1	110										Several diversions.
Pole Canyon Creek	8	13	100	Ext. noted	5											One diversion.

Total count

Intermittent and ephemeral tributaries of the Sevier River, the least Fork Sevier River, and Otter Creek drain areas that range from a few to more than 50 square miles. The quantity of water yielded by these tributaries is dependent largely upon precipitation, drainage area, topography, vegetative cover, and geology. The annual yield of an intermittent or ephemeral tributary is in general small compared to a perennial tributary, and it may range from a few to as much as several thousand acre-feet following a floodburst.

RESERVOIRS

The total storage capacity of reservoir in the upper Sevier River basin is about 80,000 acre-feet. The principal reservoirs are listed in table 3 and are shown in figure 6.

Besides the reservoirs listed in table 3, many small reservoirs and natural lakes (less than 20 acres in area) are scattered throughout the plateaus surrounding the valleys. They are particularly numerous on the Aquarius Plateau and on the southwestern part of the Monticant Plateau.

CANALS AND DITCHES

The principal canals and ditches that divert water for irrigation in the upper Sevier River basin from the Sevier River and its tributaries are shown in figure 6 and are listed in table 4. More than 20 irrigation companies maintain about 145 miles of canal and ditches. Individual canals vary in length from approximately 1 to 5 miles and discharge from about 500 to 25,000 acre-feet per year. Most of the canals and ditches are constructed of natural earth materials, but some of the canals are lined with concrete in places to prevent water losses.

TABLE 3.—Data for the principal surface-water reservoirs in the upper Sevier River basin

[Data taken from Weisler (1941)]				
Reservoir	Location	Mean storage capacity	Construction	Storage capacity (acre-feet)
Mesa Lake	S. 11 S., T. 33 N., R. 20 W.	1,000	1920	1,000
Concave Lake	T. 33 N., R. 20 W.	1,000	1920	1,000
Crater	S. 24 N., T. 37 N., R. 20 W.	1,000	1920	1,000
Pine Lake	S. 24 N., T. 37 N., R. 20 W.	1,000	1920	1,000
Twisty Hill	S. 24 N., T. 37 N., R. 20 W.	1,000	1920	1,000
Academy	T. 33 N., R. 20 W.	1,000	1920	1,000
Upper Row Creek	S. 24 N., T. 37 N., R. 20 W.	1,000	1920	1,000
Lower Row Creek	S. 24 N., T. 37 N., R. 20 W.	1,000	1920	1,000
Upper Otter Creek	T. 33 N., R. 20 W.	1,000	1920	1,000
West				14,400

GAINS AND LOSSES IN STREAMFLOW

The Sevier River and its principal tributaries gain or lose water in many places in the upper Sevier River basin. Gains are largely from tributaries, dyes, springs, and seeps; and losses are by diversion into canals and ditches, by evapotranspiration, and by seepage into stream beds and banks.

The gain and losses to the Sevier River between Hatch and Kingston for two water years, 1958-59, as indicated by measurements of streamflow and diversions, are summarized in table 5.

Panguitch Creek is the only measured tributary between the gaging stations at Hatch and Kingston, and except for a period between 1916 and 1930 it has been measured only since 1951. The unmeasured inflow from tributaries, however, is included in the measured flow of the river near Circleville and near Kingston. The quantities of water diverted by the 30 canals and ditches between Hatch and Kingston are shown in table 5. Although this water is lost from the stream at the point of diversion, part of it seeps to the ground-water reservoirs from the canals and ditches and from irrigated fields, and eventually some water leaves the ground-water reservoirs to return to the river downstream. Much of the water diverted in canals and ditches is consumed by evapotranspiration. Use of water by this means is discussed in the section "Evapotranspiration" (p. 52), so the amounts lost in this way are not listed in table 5.

Table 5 indicates that the Sevier River consistently gains water in both Panguitch and Circle Valley basins. In Panguitch Valley basin this gain is principally from the various tributaries to the river, return flow of irrigation water, and ground water from springs and seeps. The amount supplied by tributaries varies considerably from year to year, depending on the amount of precipitation. The amount of return flow of irrigation water also varies from year to year, depending on the amount of water diverted for irrigation, but the discharge from springs and seeps is more consistent from year to year. The average annual gain in the river for the 1958-59 season in Panguitch Valley basin is about 47,000 acre-feet. The authors' study of the Sevier River water conservation reports indicates that about 15 percent of the gain is ground-water discharge and that most of the water diverted by the Bear Creek Canal, Marshall Ditch, and Whittaker Ditches is ground-water discharge.

The gain in flow of the river in Circle Valley basin also comes principally from tributaries, return flow of irrigation water, and springs and seeps, but inflow from tributaries is smaller than it is in Panguitch Valley basin. The 1958-59 average gain in Circle Valley basin is about 21,000 acre-feet, of which about 80 percent is from ground-water

TABLE 6. Inflow, outflow, and prices of the Sevier River between Hatch and Kingdon, 10 miles, for the water years 1958-59

(Data from reports for the years in table 4 and 5, and data from U.S. Geological Survey manuscript papers of Sevier River no. 2, 6000 (1958) and 6100 (1959))

	1958	1959	1958	1959	1958	1959
Kingdon Valley basin						
Inflow						
Sevier River at Hatch	1,060	81,228	127,248	43,750	27,120	41,157
Kingdon Creek	17,370
Total inflow	1,060	81,228	127,248	43,750	27,120	58,527
Outflow						
Upper West Fork	573	211	0	1	590	170
Triumph Fork	1,251	1,810	860	362	573	1,201
Chickadee and Buzz Forks	1,553	1,022	780	22	573	1,140
Long and Lower South Forks	1,573	1,231	31,270	14,172	12,363	21,245
Other ditches	1,530	1,150	150	272	373	720
Lower West Fork	1,800	1,371	5,280	1,816	1,713	1,120
East Fork near Black Canyon	4,323	4,228	4,280	3,122	3,013	2,225
Black Canyon	2,223	4,172	1,140	2,144	4,013	5,174
East Fork Canal	2,223	2,128	1,120	1,122	2,223	2,120
Kingdon Ditch and South Fork	2,123	1,128	1,120	1,122	1,123	1,120
Whitaker Ditch	82	78	580	52	78	721
Sevier River near UP-Ventia	45,228	6,540	74,773	26,220	45,722	23,143
Total outflow	41,228	104,120	128,243	66,120	22,223	42,221
Gain	1,128	41,120	1,120	41,120	41,120	17,320
Circle Valley basin						
Inflow: Sevier River and Circle Ditch						
Sevier River	41,228	61,721	120,722	70,223	48,220	27,221
Circle Ditch	0
Total inflow	41,228	61,721	120,722	70,223	48,220	27,221
Outflow						
Upper West Fork	710	613	1	220	440	220
Lower West Fork	1,220	223	220	210	220	220
Long Creek	2,220	2,223	2,220	2,220	2,220	2,220
Circle Valley West Fork	2,220	10,220	1,220	2,220	2,220	2,220
Black Canyon	4,220	2,220	2,220	2,220	2,220	2,220
Whitaker Ditch	2,220	2,220	2,220	2,220	2,220	2,220
Sevier River near Kingdon	47,228	45,223	22,220	26,223	28,220	22,220
Total outflow	57,228	61,223	120,223	66,223	100,220	122,221
Gain	1,128	1,120	1,120	1,120	1,120	1,120

Kingdon. Nearly all the water diverted by the Junction Canal and Junction Middle Ditch is from return flow or ground-water discharge.

The East Fork Sevier River usually is dry between the Tripple and East Fork Canal diversion and a point south of Black Canyon in sec. 15, T. 23 S., R. 9 W., but there is enough inflow from tributaries in this reach to supply the Stem Canal and several smaller ditches.

The East Fork Sevier River gains about 20 cfs, or 15,000 acre-feet, in 10 miles, from the area south of Black Canyon to Anthony Creek. About 25 percent of this gain is from tributaries and about 75 percent is from springs. The entire flow of the East Fork Sevier River downstream from Anthony Creek generally is diverted into the Outer Canal, Reservoir, Header Canal. Between this river and Outer Canal, at the head of Kingston Canyon, the East Fork Sevier River gains about

5-10 cfs, or 3,000-7,000 acre feet. About half of this gain is the combined flow of Pole Canyon plus seepage from Otter Creek Reservoir, and half is discharge from drains and seeps.

Otter Creek consistently gains about 15 cfs, or 10,000 acre feet, between Koosharem Reservoir and a point about 18 miles downstream, in sec. 19, T. 28 N., R. 1 W., although the Koosharem Canal, Meridian Ditch, and several other ditches divert water from the flow. Almost the entire gain is from seeps and springs. Enough water enters Otter Creek during the irrigation season, when diversions are at a maximum, to supply the Jolley Ditch east angle.

GROUND WATER

SOURCE OCCURRENCE AND MOVEMENT

The source of all water in the upper Sevier River basin is precipitation within the basin. Water that reaches the land surface as precipitation either (1) evaporates, (2) transpired by plants, (3) becomes streamflow, or (4) seeps into the ground and either (1) is retained by soil moisture or (2) percolates downward to the zone of saturation and becomes part of the ground water body. The source of ground water is discussed in greater detail in the next section on "Recharge."

The principles of the occurrence of ground water have been discussed in detail by Meinzer (1930a, p. 2-102; 1930b). Only a few essential statements will be made here.

Water in an aquifer may be under either unconfined (artesian) or unconfined (water-table) conditions. Water is confined where a saturated permeable bed, such as gravel, is overlain by less permeable confining beds, such as clay or silt. Because it is confined, the water in the permeable bed is under hydrostatic pressure. A well that penetrates such a bed and flows at the ground surface is a flowing artesian well; a well that penetrates such a bed and does not flow is a nonflowing artesian well. The imaginary surface that everywhere coincides with the static level of the water in an artesian aquifer is called the piezometric surface.

If water is unconfined, that "surface" within the zone of saturation at which the pressure is everywhere atmospheric, is called the water table. If the water level in an artesian aquifer declines below the overlying confining bed, the aquifer will then be under water-table conditions. Where water-table conditions grade into artesian conditions within an aquifer, a common occurrence in the upper Sevier River basin, the water table and the piezometric surface are continuous or, in other words, are parts of the same surface.

Most of the available ground water in the upper Sevier River basin is contained in the sand and gravel deposits in the coarse ground-

water basins, and it occurs under both normal and water-table conditions.

Ground water is not stationary; it moves through an aquifer in the direction of greatest hydraulic slope. The rate of movement is slow, usually ranging from less than an inch on a few feet per day, but the quantity of water moving may be relatively large if the cross section of the aquifer is large.

RECHARGE

The principal source of recharge to the valley fill in the upper Serio River basin is infiltration from the Serio River and its tributaries, irrigation canals and ditches, and irrigated fields. Such recharge occurs only when the ground water is unconfined.

The Serio River and its tributaries recharge the valley fill where the streams flow across deposits of gravel and sand that are above the water table. Such areas of recharge are generally where streams cross the second ground-water basins. Thus for the major streams, the area of recharge is the upper end of the basin; but for small streams it is where they emerge from basins and alluvial fans bordering the valleys.

Canals and ditches recharge the ground-water reservoir when they cross permeable material, such as gravel, sand, and friable soil, along the margins of the various valleys. Water infiltrates from irrigated fields mainly in the upper ends and along the sides of the ground-water basins where the soils generally are more graded.

Another source of recharge to the valley fill is from unconfined aquifers in the mountains around the valleys. The aquifers in the mountains, in turn, are recharged from precipitation and runoff.

Water level contours may indicate areas of recharge, as ground water moves at right angles to the contours from areas of recharge toward points of discharge. Plate 2 indicates that the main recharge areas in Panguitch Valley basin are along the sides and at the upper end of the basin; recharge areas are in similar places in the other basins (pl. 2).

AQUIFER CHARACTERISTICS

The amount of ground water that can be withdrawn from an aquifer and the effects of withdrawal depend upon the hydraulic characteristics of the aquifer as well as its extent and saturated thickness. The principal hydraulic properties of an aquifer are its ability to store water, expressed by a "coefficient of storage," and its ability to transmit water, expressed by a "coefficient of transmissibility."

The coefficients of storage and transmissibility help determine, among other things, the magnitude, rate, and extent of the lowering of the water level in an aquifer caused by a discharging well. The

coefficient of storage of an aquifer is defined as the volume of water it releases from or takes into storage per unit surface area of the aquifer per unit change in the component of head normal to that surface. The coefficient of transmissibility is the rate of flow of water, at the prevailing water temperature, in gallons per day, through a vertical strip of the aquifer 1 foot wide extending the full saturated height of the aquifer under a hydraulic gradient of 100 percent. Methods used to determine the hydraulic characteristics of aquifers are described by Wenzel (1943), Ferris and others (1962), Jacob and Lightman (1958) and Theis, Brown and Meyer (1983).

The known range in coefficients of storage and transmissibility for each of the main ground-water basins in the upper Sevier River basin is shown in table 6. The coefficient of storage of unconsolidated aquifers in the basin ranges from about 0.0001 to 0.061 and that of water-table aquifers from about 0.05 to 0.15. Circle Valley basin contains the aquifers having the highest known coefficient of transmissibility, 80,000 gpd per ft (gallons per day per foot), whereas the aquifers in Tabor and Emery Valley subbasins have a maximum known coefficient of transmissibility of only 800 gpd per ft.

TABLE 6.—Range in measured and estimated coefficients of storage and transmissibility in the upper Sevier River basin

Subbasin	Coef. of storage	Coef. of transmissibility (gpd per ft)
Panguitch Valley	0.10 - 0.15	500-15,000
Circle Valley	0.03 - 0.15	100-80,000
East Park Valley	0.05 - 0.10	100-800
Tabor and Emery Valleys	0.001 - 0.15	1,000-20,000
Circle Valley (unconsolidated aquifers)	0.0001 - 0.061	100-80,000

1750-1800 meters below sea level of the center of range Circle Valley, Utah in Tabor Valley.

A wide range of values for the coefficient of storage and transmissibility, such as that shown in table 6, is common in alluvial aquifers where various degrees of sorting have taken place. If more complete data were available, however, they would probably show a range in coefficient of storage from 0.0001 to 0.15 in the basin.

ESTIMATE OF RECOVERABLE GROUND WATER IN STORAGE

The recoverable ground water in storage in the principal ground-water reservoir in the upper Sevier River basin was estimated from the areal extent, the saturated thickness, and the average coefficient of storage of the water-bearing sediments. The areal extent and thickness of the aquifers were determined by test drilling and a study of

drillers' logs. The average values of coefficient of storage assigned to the sand and gravel comprising the principal aquifers of the area were estimated to range from 0.33 to 0.15. The storage estimate was made only for the upper 500 feet of saturated valley fill because sediments at greater depths probably cannot be economically dewatered under present conditions. The estimated amount of recoverable ground water in the sand and gravel of the upper 500 feet of saturated valley fill in the various ground-water basins is about 1 million acre-feet (table 7).

The 1 million acre-feet does not represent all the recoverable ground water stored in the upper 500 feet of saturated valley fill; the rest is in silt and clay which do not yield water readily to wells. The silt and clay, however, could ultimately yield some water to the sand and gravel aquifers if and when the latter are depleted by pumping.

TABLE 7. Estimated amount of recoverable ground water in storage in the sand and gravel of the upper 500 feet of saturated valley fill in the upper Beaver River basin

Basin or subbasin	Estimated thickness of saturated sand and gravel (feet)	Estimated average coefficient of storage	Approximate area (square miles)	Estimated storage (acre-feet)
Faungtsoh Valley	50	0.15	78,000	570,000
Circle Valley	100	.15	13,000	210,000
East Rock Valley:				
Emore Valley	15	.06	12,000	6,000
Johns Valley	30	.10	35,000	90,000
Smiths Valley	60	.15	9,000	36,000
Circle Valley:				
Tookadeen	30	.10	30,000	90,000
Anglo	30	.10	20,000	60,000
Total				1,062,000

¹ Upper 500 feet of valley fill.

FLUCTUATIONS OF WATER LEVEL

Ground-water levels fluctuate primarily in response to the net withdrawal of water from or additions to the ground-water reservoir. The fluctuations may range in duration from minutes to years, and they are here classified as short-term, annual, and long-term.

SHORT-TERM FLUCTUATIONS

Short-term fluctuations of water levels can be caused by changes in streamflow, evapotranspiration, discharge from wells, and other factors. Some of the short-term changes observed in wells in the upper Beaver River basin are discussed below.

Changes in flow in nearby waterways cause change in water levels in wells (C-02-b)80ca-1 and (C-02-b)202bd-2 near Faungtsoh. Both wells tap unconfined water and were equipped with automatic

water-level recording gages. Well (C-32-4)24ba-1 is about 70 feet from an irrigation canal, and well (C-34-3)34ba-1 is about 500 feet from a small irrigation ditch and about 0.2 mile from Hungate's Creek. Records show that changes in flow in the waterways are followed in 1 to 3 days by changes in water level in the wells.

Daily fluctuations of water level are caused by evapotranspiration in areas where the water table is near the land surface. In such areas the water levels decline during the day and rise during the night. These fluctuations are relatively small and probably occur to some degree in all the areas in the basin that is covered by phreatophytes (p. 5).

Short-term fluctuations of water levels also are caused by discharge from wells. When a well discharges, the water table or piezometric surface of the aquifer perturbed by the well is depressed and a cone of an approximate form of an inverted cone will develop with the well at its apex. The extent and depth of this cone, called the cone of depression, depends on the hydraulic properties of the aquifer and the rate and duration of discharge. The cone of depression develops much faster under artesian conditions, where it is caused largely by the release of hydrostatic pressure, than it does under water-table conditions, where it is caused by gravity discharge of water from storage. When the spreading cone of depression reaches a nearby well, it causes a decline of water level in that well.

Records of a continuous water-level recording gage on well (C-36-4)34ba-1 show a typical cone of depression caused by pumping wells (C-36-4)34ba-1 and (C-36-4)34ba-2. Well (C-36-4)34ba-1 is 266 feet east and well (C-36-4)34ba-2 is 236 feet northwest of the well having the recording gage. The three wells tap the valley fill under water-table conditions at about the same depth. When well (C-36-4)34ba-1 was pumped for 48 hours on May 17-18, 1957, at a rate of about 25 gpm, the water level declined 0.21 foot in the gaged well; when the pump was turned off, the water level in the gaged well recovered 0.15 foot in 20 hours.

ANNUAL FLUCTUATIONS

Water levels fluctuate annually in most wells in the upper Sevier River basin. An annual rise of the water table is caused mostly by seepage of water from streams and by diversions of water from streams for irrigation. Annual fluctuations in erosion head generally are small, but they show some similarity to water-table fluctuations. The fluctuations in selected wells in each ground-water basin are shown on plate 9.

The pattern of annual fluctuation of water levels in wells that tap water-table aquifers is similar in all the ground-water basins in the upper Sevier River basin. Water levels usually begin to rise in March

or April in response to recharge resulting from spring runoff and early irrigation. The levels continue to rise throughout the irrigation season, and they usually are highest in July, August, or September, near the end of the irrigation season. Water levels usually decline between the end of the irrigation season and the following spring; but in some areas irrigation in the fall causes a slight rise in water levels.

Annual fluctuations in piezometric head are caused by discharge of flowing wells which are opened at the beginning of the irrigation season and closed at the end. This fluctuation is observed mainly in Kosharyn subbasin. This condition exists especially where there is a high content of wells, such as in secs. 26 and 24, T. 20 S., R. 1 W., and secs. 1 and 2, T. 27 S., R. 1 W. (see pl. 2).

LONG-TERM FLUCTUATIONS

Long-term fluctuations of water levels in the several upper Serier River ground-water basins were generally similar during the period 1835-63 (fig. 3). Water levels in all basins were at a low stage during the late 1830's and through the 1840's but declined during the 1850's, although water levels generally rose in 1852, 1858, and 1862, which were years of above-normal recharge. The correlation between water-level changes and precipitation and stream flow is shown in figures 3 and 4. Ground-water levels usually rise during periods of high precipitation and streamflow, whereas they decline during dry periods. Precipitation and streamflow were below normal from 1850 through 1856 (except for 1852), and ground-water levels generally declined during the same period.

DEVELOPMENT AND DISCHARGE

Although more than 300 wells have been constructed in the upper Serier River basin, springs supply most of the ground-water used in the basin. The wells supply water mostly for domestic use and stock, but the springs furnish the public supply for most of the communities and also much of the irrigation supply. Drains also supply some water for irrigation.

In 1963 the discharge of ground water in acre feet in the upper Serier River basin by wells, springs, and drains is summarized as follows:

Source	M.A. in acre feet			Average acre feet per acre	Total acre feet
	1963 actual	1963 estimated	1963 total		
Wells	1.00	1.60	3	1.00	14,000
Springs	1.50	08,400	5	1.50	14,000
Drains	0	2.10	3	0	0.00
Total	2.50	11,200	11	2.50	14,000

In addition to discharge from wells, springs, and drains, ground water is discharged by evapotranspiration and, in some basins, by subsurface outflow. Most of the discharge is directly from the valley fill, but several springs along the valley margins discharge from the bedrock of the surrounding highlands.

WELLS

More than 800 wells have been constructed in the upper Sevier River basin by digging, jetting, and cable-tool and rotary drilling. A description of these well-construction methods is given by Todd (1926, p. 116-119). The locations of selected wells are shown on plate 2 and details of construction and other features are given by Carpenter, Robinson, and Bjorklund (1934). Many domestic and stock wells were dug by hand before the other methods were introduced into the area. These dug wells, many of which are still in use, range from 14 to 126 inches in diameter and are from 8 to 500 feet deep. They generally are lined with rock or concrete. Most of the wells less than 4 inches in diameter were jetted, whereas most wells 4 to 16 inches in diameter were drilled by the cable-tool method. A few wells have been drilled by the rotary method.

Most of the drilled and jetted wells in the valley fill are less than 250 feet deep and are drilled just deep enough to produce a moderate amount of water. Generally only a small part of the aquifer is penetrated, especially in areas of artesian flow. Most of the well casings are unperforated and discharge water through the open bottom, but a few casings have been perforated at water-bearing zones. Wells designed to discharge large amounts of water usually are equipped with perforated casing and are developed by surging and pumping in order to remove silt and fine sand around the well.

The small-diameter domestic and stock wells are pumped mostly by gasoline or electrically driven centrifugal or piston pumps. Jet and small submersible turbine pumps supply water to many rural homes. Most of the irrigation and public supply wells are equipped with turbine pumps driven by electric motors. Water flows freely from many domestic, irrigation, and stock wells in areas where the ground water is under artesian pressure.

"Specific capacity" is a term used to indicate the efficiency of a well. It is calculated by dividing the discharge of a well by the water-level drawdown, after the well has been discharging at a constant rate for at least several hours; it is expressed in gallons per minute per foot (gpm per ft) of drawdown. The specific capacity of a given well varies slightly depending on the rate of discharge and the length of time pumped. Table 4 shows the observed specific capacities of wells in the upper Sevier River basin range from 5.01 to 53 gpm per ft.

TABLE 6. Range and average of specific capacities of wells in the upper Snake River basin.

Sub-basins	Well No. with data available	Range in Specific Capacity (gpm per ft.)	Average Specific Capacity (gpm per ft.)
Hangley's Valley.....	16	0.6-10	2.5
Grass Valley.....	9	1-10	5.0
East Fork Valley:			
John Valley-Imery Valley.....	11	0.1-1.6	2.4
Anthony.....	8	0.8-16	4.2
Grass Valley.....	113	0.02-35	2.0

The wide range in specific capacities of wells in the basin is mainly due to differences in methods of well construction, differences in the permeability of the water-bearing zones, or a combination of both. For example, well (C 30 4)3500-1, which has a specific capacity of 32 gpm per ft. is an irrigation well constructed to provide a large field. The well is 189 feet deep, penetrates 65 feet of saturated sand and gravel, and has a 12-inch casing, of which 59 feet is perforated. In contrast, well (C 30 5)2000-1, which has a specific capacity of about 4 gpm per ft., was constructed to provide only a small amount of water for stock. The well is 186 feet deep, penetrates 26 feet of saturated gravel, and has a 6-inch unperforated casing, which receives water only through its open end.

The average annual discharge from wells in the upper Snake River basin is about 1,300 acre-feet. Approximately 1,000 acre-feet is used for irrigation, 1,100 acre-feet for domestic use and stock, 100 acre-feet for public supply, and 3 acre-feet for industry. Of the 1,300 acre-feet used for irrigation, about 1,000 acre-feet is from flowing wells and about 300 is from pumped wells. The average discharge by wells in the four main basins, classified by use and type of well, are listed in table 6. The discharge by wells in Grass Valley basin is about 80 percent of the total discharge by wells in all four basins. The quantities in table 6 were estimated for 1952 from information on the type and period of use of wells, periodic measurements of discharge of selected wells, discharge measurements made during the well inventory, and yields reported by owners and drillers.

The discharge of flowing wells is greatest when artesian head is high, usually during years of high precipitation and high streamflow, when recharge also is high. Discharge of pumped wells is usually greatest when precipitation and streamflow are low, and wells are used to supplement streamflow and spring discharge.

TABLE 3.—List of water, number of wells, and calculated discharge from wells in 1966 in four basins of the upper Sevier River system.

Basin	Perennial flow		Intermittent		Irrigation		Domestic		Total		Wells	Discharge, cfs
	Wells	MGD	Wells	MGD	Wells	MGD	Wells	MGD	Wells	MGD		
All wells												
Panguitch Valley	24	5	0	0	0	0	0	0	44	5	78	425
Crane Valley	17	34	1	3	0	0	0	0	18	37	19	613
East Fork Valley	21	0	0	0	0	0	0	0	21	0	2	11
Clear Valley	66	1,153	0	0	23	33	0	0	89	1,186	173	2,443
Total	128	1,253	1	3	23	33	0	0	154	1,329	270	3,592
Mining wells												
Noncreek Valley	0	0	0	0	0	0	0	0	0	0	0	0
Crane Valley	3	0	0	0	0	0	0	0	3	0	0	0
East Fork Valley	0	0	0	0	0	0	0	0	0	0	0	0
Clear Valley	108	1,102	0	0	23	33	0	0	131	1,135	173	2,588
Total	111	1,102	0	0	23	33	0	0	134	1,135	173	2,588
Pumped wells												
Panguitch Valley	35	0	0	0	0	0	0	0	35	0	0	0
Crane Valley	22	1	1	0	0	0	0	0	23	1	0	501
East Fork Valley	23	0	0	0	0	0	0	0	23	0	0	13
Clear Valley	23	0	0	0	0	0	0	0	23	0	0	0
Total	73	1	1	0	0	0	0	0	74	1	0	514

DISCUSSION

Most of the ground water used beneficially in the upper Sevier River basin comes from springs. Springs furnish the public supplies for Panguitch, Cannonville, Kingston, Altonway, Buxville, and Kanab; most of these springs discharge from bedrock in the mountains and plateaus adjacent to the valleys. Development for public supply ordinarily consists of one or more collecting chambers at the spring site, a gravity conveyance system from the spring to the town, and a distribution system.

Many springs in the valleys and in the surrounding mountains and plateaus are important sources of water for irrigation. For example, springs discharging from bedrock in the Mammoth and Seay Creek drainages ordinarily contribute more than half the annual flow of the Sevier River at Hatch (Wilbur and Thomas, 1961, p. 35). The location of some of the principal springs in the upper Sevier River basin is shown on plate 2, and the discharge from these springs is given in table 10.

TABLE 10. Estimated discharge and use of water in 1960, in acre feet, from major springs in the upper Sevier River basin

Data	Total discharge	Use		Public supply
		acre feet	percent	
Turkey Valley	30,000	26,000	87 percent from bedrock, approximately 10 percent from valley fill and 3 percent from stream	70
Circle Valley	3,000	3,000	all from bedrock	10
East Fork Valley	25,000	15,000	50 percent from bedrock; 50 percent from stream	60
Green Hills	1,000	800	100 percent from bedrock; 10 percent from stream	40
Total estimated	59,000	45,800		50

1. All acre-feet.

About 85 percent of the spring discharge listed in table 10 is used for irrigation and stock and the remainder is used for public supply. Approximately 50 percent of the water discharged by these springs is from the valley fill and 50 percent is from bedrock. Many other bedrock springs are in remote parts of the mountains and plateaus surrounding the valleys, and the water discharged from them is accounted for in the flow of the perennial streams.

DEVELOPMENT

Control of water levels by artificial drawdown in cross underlain by massive aquifers has been attempted in Circle Valley basin and Altonway subbasin. The two drawdown systems yield about 5,000 acre-feet of water annually, and they have become more important as a source

of supply for irrigation downstream than as a means of controlling water levels. The drains are open channels, deep enough to penetrate to the water table in the saturated clay and silt near the surface but not deep enough to tap directly the underlying artesian aquifers. Water is forced through the confining silt and clay overlying the aquifer and it eventually moves into the drains. The drains also collect some water that has been applied for irrigation of arroyo alluvium to the wet bottom lands. Several canals have been constructed in Panguitch and Circle Valley basins and in Koozeboom subbasin to collect water from slough and spring areas and deliver it to irrigated land. Although these canals in a sense are drains, they have not lowered water levels significantly, and their intended result was not drainage but recovery of water for irrigation.

Drains and canals in arroyo areas such as the downstream parts of Panguitch Valley and Circle Valley basins and most of Anthony and Koozeboom subbasins have not lowered water levels greatly because they are not deep enough to tap the more permeable water-bearing beds in the valley fill. The sand and gravel deposits in arroyo areas generally are overlain by at least 5-20 feet of relatively impermeable silt and clay which will yield water to drains slowly but not in sufficient quantity to lower water levels significantly. Water levels could be lowered significantly by penetrating the underlying permeable deposits of gravel and sand with wells, deeper drains, a more efficient type of drain, or flowing wells in the bottom of drains.

Discharge of ground water by drains in the upper Sevier River basin is estimated to be about 8,000 acre-feet per year (table 11), and almost all the water is used for irrigation. The discharge from drains usually fluctuates in direct proportion to the amount of water discharged for irrigation.

EVAPOTRANSPIRATION

Evapotranspiration includes water discharged to the atmosphere by transpiration of vegetation or by direct evaporation. Water can evaporate directly from open-water surfaces, from the water table when it is at or near the land surface, from the soil, and from any exposed surface on which precipitation falls. About 12,000 acre-feet of surface water is evaporated annually from eight reservoirs in the upper Sevier River basin. In addition, about 40,000 acre-feet of water is discharged annually by capillary action from about 23,000 acres of wet land in the basin. Most of this 40,000 acre-feet of water is derived from the ground-water reservoir, but some seeps in from ad. areas irrigated areas.

TABLE 11. Estimated average annual discharge of water to the upper Sevier River basin.

Location	Length of stream (miles)	Average annual discharge (acre-feet)
Paria Plateau Valley.....	9	0
Clare Valley.....	5	3,000
East Fork of the Sevier River.....	4	2,000
Clare Valley.....	6	0
Total.....	9	5,000

ESTIMATION OF SURFACE-WATER EVAPORATION

The average annual evaporation from surface-water reservoirs in the upper Sevier River basin is more than five times the long-term average annual precipitation. Evaporation data have been collected for 45 years (1902) at Pivote Dam, which is 2 miles north of Wingston and about 6,000 feet above sea level; a standard U.S. Weather Bureau land pan was used. Since 1918 the average annual evaporation from May through November has been about 55 inches (U.S. Weather Bureau, written communication, 1963).

The annual evaporation from 11 major large surface-water reservoirs in the upper Sevier River basin is estimated to be about 12,000 acre-feet; it is summarized below:

Reservoir	Annual evaporation (acre-feet)	Reservoir	Annual evaporation (acre-feet)
Navajo Lake.....	1,200	Beaver Head.....	40
Paria Plateau.....	3,700	Rocky Point.....	500
Prophet.....	200	Lower Hot Creek.....	30
Pine Lake.....	200	Other Reservoirs.....	8,000
		Total (rounded).....	12,000

1 Based on an evaporation rate of 5 inches per year at Pivote Dam. Evaporation rates for other reservoirs are based on this evaporation value times the ratio of area.

DIRECT EVAPORATION OF GROUND-WATER

The amount of ground water discharged directly by evaporation depends upon many factors, including depth to the water table, soil type, and various climatological factors. Where the water table intersects the land surface, evaporation takes place directly from the ground-water body. Where the water table is only a few feet below the land surface and the soils are fine grained, the capillary fringe above the water table may reach the land surface; water then evaporates from the damp soil and is replaced from the ground-water reservoir.

voir by capillary action. (According to Meinzer (1923b, p. 26), "The capillary fringe * * * contains capillary moisture some or all of which are filled with water that is continuous with the water in the zone of saturation. * * *")

The amount of ground water that is discharged directly by evapotranspiration in the upper Sevier River basin is not known.

TRANSPIRATION

Transpiration is the discharge of water to the atmosphere by plants. If the water table or capillary fringe is within reach of the roots of plants, ground water will be discharged by transpiration. The rate of transpiration depends upon many conditions, including climate, plant type, size and density, depth to water, and the quality of the water. Transpiration of water by plants that has some recognized benefit to mankind is a consumptive use; transpiration of water by plants that do not benefit man is a consumptive waste (Thomas, 1951, p. 237).

Phreatophytes are plants that depend for their water supply on ground water that lies within reach of their roots (Robinson, 1958, p. 1). The principal phreatophytes in the upper Sevier River basin are alfalfa, willow, cottonwood, greasewood, and rabbitbrush.

Areas that contain small bodies of surface water fed by springs and areas where the water table is close to the land surface generally support extensive growths of phreatophytes. Studies and experiments in the western and northern United States, made under wide varieties of climate, plant growth density, depth to water, quality of ground water, and soil type, indicate that fully developed cottonwoods use from 5 to more than 7 acre-feet of water per acre per year and that alfalfa, willow, greasewood, and rabbitbrush use approximately 2 to 3 acre-feet per year (Robinson, 1958, p. 45-75).

Phreatophytes in the upper Sevier River basin probably consume water at 50-75 percent of the rates given by Robinson, because much of the data on which Robinson's figures are based were collected in areas having higher average temperatures and larger growing seasons. The gross rate of evapotranspiration for the valleys in the upper Sevier River basin is estimated to be 20-30 inches per acre per year. Values in this range were used in table B in estimating the average annual evapotranspiration from the principal areas of phreatophyte growth in wet areas in the basin. (See p. 2.)

SUBSURFACE OUTFLOW

Some ground water leaves the upper Sevier River basin or moves between the individual ground water basins in the west by subsurface outflow through both the valley fill and bedrock. The amount

TABLE 12.—Average annual component, in million of water from phreatic aquifers in the Great Basin, and in million of the upper Sevier River basin

Basin or subbasin	Area (sq. mi.)	Percentage of annual precipitation	Estimated annual outflow (million cu ft)
Panguitch Valley	2,500	20	14,000
Circle Valley	2,200	20	8,800
East Fork Valley:			
Emery Valley	3,000	20	5,000
Johns Valley	700	20	1,400
Archaic	2,100	20	4,200
Grand Valley:			
Kane	5,000	20	10,000
Angle	200	20	900
Total	28,400		43,600

leaving each ground-water basin through valley fill generally is small because subsurface bedrock barriers at the downstream end of each of the basins under the cross-sectional area of valley fill south. Circle Valley basin and Angle subbasin are the only areas from which there is any significant amount of subsurface outflow. Gravel and sand beds at the downstream end of Circle Valley basin transmit about 1,000,000 acre-feet of water per year in the central Sevier Valley downstream. About 1,000 acre-feet per year move from Angle subbasin to Archaic subbasin at the Otter Creek River on the west.

Ground water leaves the upper Sevier River basin by subsurface outflow through subsoil channels in the same of the Wasatch Formation near Nevada Lake. This water discharge southward from Cascade Spring in the NE¼SE¼SE¼ sec. 17, T. 38 S., R. 9 W., in the Virgin River basin at a rate of about 1,000,000 acre-feet annually (Wilson and Thomas, 1964). Some ground water also seeps through bedrock eastward into the Paria River drainage from the Panguitch Plateau (Marine, 1963, p. 161-162). A determination of the total amount seeping out of the upper Sevier River basin through bedrock is beyond the scope of this investigation, but recent studies by Goetz (1964) and the U.S. Soil Conservation Service (written contract, 1965) indicate that the amount from the Panguitch Plateau alone may be several thousand acre-feet annually.

RELATION BETWEEN GROUND WATER AND STREAMFLOW

The base flow of the Sevier River, the East Fork Sevier River, and Otter Creek in most parts of their channels is affected by discharge to or recharge from the ground-water reservoir. The streams lose water when the water table or piezometric surface is lower than the stream surface, especially where the stream beds are highly permeable

materials such as gravel or coarse sand. Conversely, the streams gain water where ground water levels are above the stream level. The water that enters the ground from the streams moves through the aquifers at velocities of only a few feet per day or less. The quantity of water moving through the aquifers, however, probably is relatively large because the aquifers generally have a high average permeability, a large cross-section area, and a hydraulic gradient of several feet per mile.

At several places in the upper Sevier River basin, subsurface barriers of bedrock impede the downstream movement of ground water, force the water toward the south, and thus cause the ground-water reservoirs to overflow. These barriers are at the downstream ends of Panguitch Valley and Circle Valley basins, Johns Valley and Antelope subbasins of East Fork Valley basin, and Koochekorn and Angle subbasins of Gross Valley basin. Upstream from these barriers, ground water is discharged mainly by evapotranspiration, by springs, and by seeps that return much of the water to the stream. For example, the base flow of the Sevier River in Cleverville Canyon can be correlated directly with water levels in the valley fill at the downstream end of Panguitch Valley basin. This direct relation is illustrated in figure 5, which shows that high water levels in the valley fill correspond to a high base flow in the Sevier River and low water levels correspond to a low base flow.

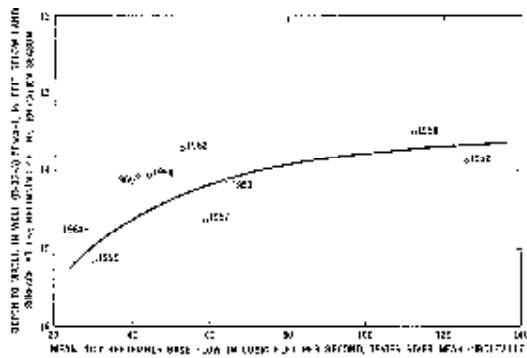


FIGURE 5. Graph showing the relation between the water level in well 62132 (62132-1) and the base flow of the Sevier River near Cleverville in Cleverville Canyon.

Withdrawals of ground water by wells and drains may lower water levels and consequently reduce the base flow of streams. If enough water is withdrawn, the natural discharge of ground water to streams may decrease significantly or stop. The surface water and ground-water systems in the upper Sevier River basin are in approximate equilibrium, and the removal of large amounts of ground water could eventually (1) increase discharge to the aquifers from surface water and thereby decrease streamflow, (2) decrease ground-water discharge to streams and from springs, flowing wells, and evapotranspiration, or (3) have combined effects of (1) and (2).

INFLOW-OUTFLOW ANALYSES OF THE GROUND-WATER BASINS

In any basin, the quantity of water entering by surface-water inflow, ground-water inflow, and precipitation is equal to the quantity of water leaving the basin, by surface-water outflow, ground-water outflow, and evapotranspiration, plus or minus the quantity gained or lost in surface and ground-water storage and changes in soil moisture. All these quantities can be related by means of an inflow-outflow analysis, a type of hydrologic budget.

Inflow-outflow analyses were made for each of the ground-water basins in the upper Sevier River basin for the 1961 and 1962 water years. The major difficulties in making the analyses were the complexity of the distribution system for irrigation water, insufficient precipitation data, and lack of data (1) for several important surface-water sites, (2) for inflow from perennial, intermittent, and ephemeral streams, and (3) for ground water entering each basin by inflow from bedrock. Because of these difficulties, some estimates and assumptions were necessary, and the data listed in tables 18-19 should not be considered as absolute.

Surface-water inflow and outflow were based upon measurements, where available, and upon estimates. Estimates of surface-water inflow were based largely upon size, altitude, and geology of the drainage area and upon precipitation on the drainage area. Some of the unaged inflow from intermittent and ephemeral streams is included in the item "Inflow from other sources."

Ground-water inflow and outflow at the upper and lower ends of the basins were estimated on the basis of the thickness of, permeability of, and hydraulic gradient in the valley fill. Sufficient information was not available to make a separate estimate of the amount of ground water moving into the basins directly from bedrock. An indirect estimate of this amount, however, is included in the item "Inflow from other sources."

Precipitation was estimated from records of the U.S. Weather Bureau. Precipitation data at a station within a basin were used

when available, but an average of 3 in. from surrounding stations was used when local data were not available.

The evapotranspiration from cultivated areas was estimated using a method described by Credille, Harris, and Willardson (1932). The croplands were classified according to crop type (including alfalfa, small grains, corn, potatoes, pasture, wild hay) or as idle land. The average of each crop type varied from year to year depending upon the water supply and other factors. Grass water-use requirements for each type were multiplied by the acreage of each type to determine the annual amount of water consumed.

The method of estimating evapotranspiration from noncultivated wet areas is described in the section on "Evapotranspiration." No data are available for evaporation from waterlogged land; therefore, the estimates are based on the evaporation from phreatophytes in all wet areas, including bare-soil bogs.

Evapotranspiration from noncultivated brushland was assumed to equal all the precipitation on these lands. Much of the area that comprises the ground-water basins in the upper Sevier River basin is not cultivated. It is covered with native brush and other vegetation that depend for their water supply entirely on soil moisture derived directly from precipitation. Little, if any, of the precipitation recharges the ground-water reservoir.

The method of estimating evaporation from Otter Creek and Koosler area reservoirs is described in the section on "Evaporation from surface-water reservoirs." The other reservoirs in the upper Sevier River basin are outside the ground-water basins. The rates of evaporation from Otter Creek and Koosler area Reservoirs are assumed to be similar for both the 1961 and 1962 water years. However, the large difference in storage in Otter Creek Reservoir during the period of high evaporation (May-September) caused a significant change in the total evaporation from 1961 to 1962.

The changes in storage in Otter Creek Reservoir were measured. Records of changes in storage in Koosler area Reservoir are not available, but about the same amount of water is in the reservoir at the beginning and end of every water year; therefore, there is little significant change in storage.

The changes in ground-water storage were determined as the product of three factors: (1) the area whose ground water is under water-table conditions, (2) the amount change in the level of the water table, and (3) the average storage coefficient of the water-table unit. Changes in storage in alluvial aquifers were not included in the analysis because they were considered to be negligible owing to the extremely small storage coefficient of alluvial aquifers and small changes in head. Changes in soil moisture were not considered in the

analyses because it was assumed that there was little net change on an annual basis.

The inflow from other sources is inflow not otherwise accounted for in the analyses. It includes surface flow from some perennial, intermittent, and ephemeral streams and trflow of ground water from the following sources: seepage from streams into the valley fill near the plateau and mountain fronts and seepage from bedrock in the mountains and plateaus directly to the valley fill of the ground-water basins.

The inflow from the other sources is the unknown quantity in the analyses, and it was approximated by taking the difference between all other items of estimated inflow and outflow, plus or minus changes in storage. This difference, of course, also includes all errors involved in making the estimates or assumptions.

PANGLOSS VALLEY BASIN

The inflow-outflow analyses of Pangloss Valley basin for the 1961 and 1962 water years are given in table 18 (next page). Precipitation generally was above normal during the 1961 water year throughout the upper Sevier River basin. Subsequently, streamflow generally was high during the 1962 water year. The inflow during these years was 187,000 and 175,000 acre-feet, respectively. Of this amount, about one-third to one-half left the basin in the Sevier River, whereas the remainder was consumed in the basin or else went into temporary ground-water storage.

During the 1961 and 1962 water years, an average of about 22 percent of the water consumed in the basin was used in cultivated areas, about 16 percent was used in noncultivated wet areas, and about 60 percent in noncultivated brushland. The measured streams supplied an average of about 10 percent of the total inflow, precipitation on the basin supplied about 48 percent, and inflow from other sources provided about 17 percent.

CIRCLE VALLEY BASIN

The inflow-outflow analyses of Circle Valley basin for the 1961 and 1962 water years are given in table 11. The inflow during these years was 75,000 and 108,000 acre-feet. Of this amount, about 60 percent left the basin in the Sevier River and in two canals, about 2 percent left the basin as underflow, about 30 percent was consumed in the basin, and about 2 percent went into temporary ground-water storage.

During the 1961 and 1962 water years, an average of about 54 percent of the water consumed in the basin was used in cultivated areas, about 31 percent was used in wet noncultivated areas, and about 15 percent in noncultivated brushland. The Sevier River supplied about 57 percent of the total inflow to the basin, precipitation on the basin

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contributed about 13 percent, and inflow from other sources supplied about 14 percent.

TABLE 13.—Inflow and outflow of water and change in storage, Panguitch Valley basin, in thousands of acre-feet

	Water years	
	1941	1942
Surface-water inflow at upper end (Beaver River plus West Hill, Canal and West Hatch Ditch).....	48	111
Ground-water inflow at upper end.....	Negligible	216
Precipitation on ground-water basin (78,400 acres).....	81	51
Inflow from other sources (includes Panguitch Creek).....	26	34
Total water entering the basin.....	167	173
Surface-water outflow (Beaver River).....	50	91
Ground-water outflow.....	Negligible	
Evapotranspiration from—		
Cultivated areas (10,300 acres).....	22	23
Noncultivated wet areas (8,500 acres).....	14	14
Noncultivated brushland (57,600 acres).....	71	88
Total water leaving the basin.....	169	165
Change in ground-water storage.....	+7	+10
Total water entering the basin.....	167	173

TABLE 14.—Inflow and outflow of water and change in storage, Ono Valley basin, in thousands of acre-feet

	Water years	
	1941	1942
Surface-water inflow at upper end (Beaver River).....	50	101
Ground-water inflow at upper end.....	Negligible	
Precipitation on ground-water basin (14,000 acres).....	18	9
Inflow from other sources.....	9	8
Total water entering the basin.....	77	108
Surface-water outflow (Beaver River plus Junction and Junction Middle Canals).....	46	78
Ground-water outflow.....	1	9
Evapotranspiration from—		
Cultivated areas (1,800 acres).....	11	11
Noncultivated wet areas (8,500 acres).....	8	9
Noncultivated brushland (6,000 acres).....	5	8
Total water leaving the basin.....	74	105
Change in ground-water storage.....	+3	+3
Total water entering the basin.....	77	108

EAST FORK PALLAS WASH

ONONY VALLEY SUBBASIN

The inflow-outflow analysis of Onony Valley subbasin for the 1931 and 1932 water years are given in table 15. The inflow during each of these years was about 26,000 acre-feet. Of this amount, about 27 percent left the subbasin in the East Fork Beaver River, about 13 percent

left the subbasin and the Sevier River drainage basin by transmountain diversion in the Tropic and East Fork Canal, and about 61 percent was consumed in the subbasin or went into temporary ground-water storage.

During the 1961 and 1962 water years, an average of about 88 percent of the water consumed in the subbasin was used in noncultivated wet areas and about 87 percent was used in noncultivated brushland. The East Fork Sevier River supplied about 19 percent of the total inflow to the subbasin, precipitation on the subbasin contributed about 54 percent, and inflow from other sources supplied about 27 percent.

Table 15.—Inflow and outflow of water and change in storage, Henry Valley subbasin, in thousands of acre-feet

	Water years	
	1961	1962
Surface-water inflow at upper end (East Fork Sevier River).....	4	6
Ground-water inflow at upper end.....	Negligible	Negligible
Precipitation on ground-water subbasin (12,000 acres).....	17	11
Inflow from other sources.....	5	9
Total water entering the subbasin.....	26	26
Surface-water outflow:		
East Fork Sevier River.....	2	5
Tropic and East Fork Canal.....	2	4
Ground-water outflow.....	Negligible	Negligible
Evapotranspiration from—		
Noncultivated wet areas (2,000 acres).....	6	7
Noncultivated brushlands (8,000 acres).....	12	6
Total water leaving the subbasin.....	20	22
Change in ground-water storage.....	14	4
Total water leaving the subbasin.....	34	26

JAMES HADLEY BURNARD

The inflow-outflow analyses of Johns Valley subbasin for the 1961 and 1962 water years are given in table 16. The inflow during those years was 67,000 and 47,000 acre-feet. Of this amount, about 45 percent left the subbasin in the East Fork Sevier River, about 61 percent was consumed in the subbasin, and about 4 percent went into temporary ground-water storage.

During the 1961 and 1962 water years, an average of about 33 percent of the water consumed in the subbasin was used in cultivated areas, about 3 percent was consumed in noncultivated wet areas, and about 64 percent was consumed in noncultivated brushland. The East Fork Sevier River supplied about 18 percent of the total inflow, precipitation on the subbasin contributed about 55 percent, and inflow from other sources supplied about 27 percent.

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Table 18.—Inflow and outflow of water and change in storage, John
Fisher subbasin, in thousands of acre feet

	Water year	
	1961	1962
Surface-water inflow in upper end (East Fork Sevier River).....	5	0
Ground-water inflow at upper end.....	Negligible	
Precipitation on cultivated-water subbasin (20,700 acres).....	11	19
Inflow from other sources.....	21	15
Total water entering the subbasin.....	37	34
Surface-water outflow (East Fork Sevier River).....	21	10
Ground-water outflow.....	Negligible	
Evapotranspiration from		
Cultivated areas (12,967 acres).....	5	4
Noncultivated wet areas (1,700 acres).....	1	1
Noncultivated brushland (20,900 acres).....	19	11
Total water leaving the subbasin.....	46	26
Change in ground-water storage.....	12	12
Total water entering the subbasin.....	62	47

ANTIMONY SUBBASIN

The inflow-outflow analyses of Antimony subbasin for the 1961 and 1962 water years are given in table 17. The inflow during each of these years was about 80,000 acre-feet. Of this amount, about 82 percent left the subbasin in the East Fork Sevier River and the Otter Creek Reservoir Feeder Canal for use downstream, and about 17 percent was consumed in the subbasin or went into temporary ground-water storage.

During the 1961 and 1962 water years, an average of about 56 percent of the water consumed in the subbasin was used in cultivated areas, about 48 percent was used in noncultivated wet areas, and about 14 percent in noncultivated brushland. The East Fork Sevier River supplied about 53 percent of the total inflow and Antimony Creek about 40 percent; precipitation on the subbasin contributed about 6 percent; inflow from other sources supplied about 5 percent, and outflow from Apple subbasin contributed about 2 percent.

GRASS VALLEY BASIN

TODD CREEK SUBBASIN

The inflow-outflow analyses of Koschieron subbasin for the 1961 and 1962 water years are given in table 18. The analyses indicate that 81,000 acre-feet of water entered the subbasin during each of these years. Of this amount, about 51 percent left the subbasin as surface flow in Otter Creek for use downstream, about 65 percent was consumed in the subbasin, and about 5 percent went into temporary ground-water storage.

WATER RESOURCES

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TABLE 17.—Inflow and outflow of water and change in storage, Antisawp subbasin, in thousands of acre-feet

	Water year 1961	Water year 1962
Surface water inflow at upper end (East York Reservoir inflow).....	20	26
Inflow from Antisawp Creek.....	10	17
Ground-water inflow:		
At upper end.....	Negligible	
From Antisawp hills.....	1	1
Precipitation on ground-water subbasin (8,000 acres).....	0	1
Inflow from other sources.....	4	2
Total water entering the subbasin.....	35	47
Surface-water outflow (East York Reservoir inflow and Otter Creek Reservoir Evaporation).....	47	43
Ground-water outflow.....		
Evapotranspiration from—		
Cultivated areas (2,000 acres).....	4	4
Noncultivated wet areas (2,000 acres).....	6	6
Noncultivated brushland (1,000 acres).....	2	1
Water seep leaving the subbasin.....	25	32
Change in ground-water storage.....	+3	0
Total water leaving the subbasin.....	80	82

TABLE 18.—Inflow and outflow of water and change in storage, Koochewic subbasin, in thousands of acre-feet

	Water year 1961	Water year 1962
Surface water inflow (from tributaries).....	20	24
Precipitation on ground-water subbasin (83,000 acres).....	33	35
Inflow from other sources.....	11	13
Total water entering the subbasin.....	64	72
Surface-water outflow (Otter Creek).....	15	22
Ground-water outflow.....		
Evapotranspiration from—		
Cultivated areas (6,400 acres).....	11	11
Noncultivated wet areas (5,000 acres).....	0	11
Noncultivated brushland (18,400 acres).....	20	11
Evaporation from Koochewic Reservoir.....	1	1
Total water leaving the subbasin.....	58	67
Change in ground-water storage.....	+3	+4
Total water entering the subbasin.....	61	71

During the 1961 and 1962 water years, an average of about 30 percent of the water consumed in the subbasin was used in cultivated areas, about 24 percent was used in noncultivated wet areas, about 22 percent in noncultivated brushland, and about 2 percent was evaporated from Koochewic Reservoir. The surface flow of tributaries supplied about 41 percent of the total inflow, precipitation on the subbasin contributed about 43 percent, and inflow from other sources supplied about 17 percent.

ANALYSIS

The inflow-outflow analysis of Angie subbasin for the 1961 and 1962 water years are given in table 18. The inflow during these years was 81,000 and 69,100 acre-feet of water. Of this amount, about 53 percent left the subbasin as surface flow through Otter Creek Reservoir for use downstream, about 2 percent left the subbasin as underflow, about 38 percent was measured in the subbasin, about 7 percent went into temporary surface-water storage, and about 2 percent went into temporary ground-water storage.

During the 1961 and 1962 water years, about 4 percent of the water contained in the subbasin was used in cultivated areas, about 4 percent in noncultivated wet areas, about 95 percent in noncultivated brushland, and about 37 percent evaporated from Otter Creek Reservoir. Otter Creek and Otter Creek Reservoir Feeder Canal supplied about 68 percent of the total inflow, precipitation on the subbasin contributed about 33 percent and inflow from other sources contributed about 2 percent.

TABLE 18.—Inflow and outflow of water and change in storage, Angie subbasin, in thousands of acre-feet

	Water years	
	1961	1962
Surface-water inflow by Otter and Otter Creek and Otter Creek Reservoir Feeder Canal	49	45
Precipitation on ground water subbasin (20,000 acres)	18	12
Inflow from other sources	3	2
Total water entering the subbasin	70	59
Surface water outflow (Otter Creek Reservoir outlet)	31	29
Ground water outflow	1	1
Evapotranspiration from—		
Cultivated areas (1,650 acres)	1	1
Noncultivated wet areas (200 acres)	1	1
Noncultivated brushland (20,150 acres)	18	12
Evaporation from Otter Creek Reservoir	4	3
Total water leaving the subbasin	54	46
Change in surface-water storage	+4	+3
Change in ground-water storage	+1	2
Total water entering the subbasin	81	69

GROUND-WATER CONDITIONS IN THE BASIN
PANGLOSS VALLEY BASIN

Availability and storage of ground water

Ground water is readily available to wells in Pangloss Valley basin, mainly in the valley fill from Hatch to the head of Circleville Canyon. The valley fill in the northern part of Pangloss Valley basin ranges in thickness from 0 to more than 200 feet (Falls and Johnson, 1963, p. 7-17). Test hole (C-33-6) 2hd-1, in the northeastern part of the valley, penetrated 332 feet in valley fill, all of

which is alluvium, without reaching bedrock. The thickest zone of valley fill, 400-600 feet thick, extends north-south through the central and eastern parts of the valley. From this zone the valley fill probably thins to the north, west, and south toward the basin boundaries (pl. 1, sections B-B' and D-D'). Generally it is coarsest on the eastern side of the valley in proximity to the Serier fault and the plateau, but the most permeable deposits are along the Soviet River channel. About 25-50 percent of the valley fill in the northern part of the basin is permeable sand and gravel.

The valley fill in the southern part of Panguitch Valley basin between Panguitch and Hatch is much thinner and less permeable than that in the northern part of the basin. On the basis of data from the few wells that have been drilled in this area, the valley fill was estimated to range in thickness from 0 to 200 feet.

Ground water is under artesian conditions in the valley fill in a small area at the lower end of the basin (see pl. 2). It is impounded there by a consolidation in the bedrock which forms a barrier to further subsurface movement toward the north. The ground water is confined in permeable gravel by 6-20 feet of overlying silty clay of low permeability, and the piezometric surface in wells in the gravel ranges from 0 to 5 feet above the land surface and averages about 3 feet above the land surface. At the lower end of the basin the artesian area is marked by marshes and meadowside.

Ground water generally is under water-table conditions in the southern four-fifths of Panguitch Valley basin. The observed water table ranges from less than 7 feet below the land surface in well (C-98-5)96db-7 to more than 89 feet in well (C-34-5)26db-2.

An estimated 575,000 acre-feet of ground water is stored in the sand and gravel in the upper 500 feet of saturated valley fill in the basin (table 7), mostly under water-table conditions. The sand and gravel deposits are separated by saturated silt and clay which are not permeable enough to yield water readily to wells.

The Soviet River Formation on both the east and west sides of the south-central part of the basin contains ground water, some of which is perched above the water levels shown in plate 2.

No production wells have been constructed in the bedrock that surround and underlie the valley. Therefore, although the rocks are known to contain ground water, it is not known if they will yield water readily to wells.

Existing use

Most of the ground water used in Panguitch Valley basin is discharged by springs which issue from either the valley fill or from bedrock. The largest springs that discharge from valley fill are in

66 GROUND WATER, UPPER SEVIER RIVER BASIN, UTAH

the Marshall and Vester Sloughs (sec. 35, T. 33 S., R. 5 W.). These springs have a combined discharge of about 1,800 gpm. Many smaller springs discharge from less than 1 to about 400 gpm from permeable zones in the alluvial fans and in the Sevier River Formation along the sides of the basin. Many of these springs are along the edge of the bluffs on the east side of the Sevier River between Hatch and Castle Canyon and along the edge of the alluvial fans on the west side of the river between Throssula Creek and Base Creek.

The bedrock springs are mostly in mountainous areas, generally, remote from the valley floor. Information on the major bedrock springs is summarized below:

Name	Location	Depth, feet	Date of installation	Type of use
Blue Spring	(C-36-7) Hatch	10	Aug. 1902	Irrigation and stock Dr.
Macintosh Spring	22dse	2-270	Apr-June 1937	Do.
Upper Asay Spring	(U-37-6) 20dse	8	Oct. 1902	Do.
Lower Asay Spring	24se	22-230	1954	Do.
Dink Creek Spring	(U-39-9) 20d	24-26	1954	Do.
Indian Hollow (or Panguitch) Springs	(C-34-6) 48	1	Dec. 1911	Public supply, Panguitch.

These springs usually have a combined flow of about 80 cfs and supply about 65,000 acre-feet of water annually in the Sevier River system. All except Indian Hollow Springs discharge from solution channels in the limestone of the Wasatch Formation, although the water from many of them emerges from broken basalt overlying the limestone. Indian Hollow Springs issue from volcanic rocks of Tertiary age.

Most of the wells in Panguitch Valley basin were constructed for domestic and stock use, but one well is used for public supply at Hatch. All the wells obtain water from the alluvial deposits or the Sevier River Formation, and yields from individual wells range from about 1 to 75 gpm. Wells produce less than 55 acre-feet of water annually in Panguitch Valley basin, and all the water is pumped.

There are approximately 70 wells in the basin. About 30 are dug wells, and they range from 24 to 54 inches in diameter and from 3 to 75 feet in depth. About 40 are drilled wells, and they range from 8 to 10 inches in diameter and from 35 to 685 feet in depth; most of them, however, are less than 200 feet deep. Most of the ground water pumped in Panguitch Valley basin is from well (C-36-5) 20dse-1, which yields about 30 acre feet annually for public supply at Hatch.

Potential development

About 7,000 acre-feet of additional ground water could be withdrawn annually in Panguitch Valley basin without greatly affecting the flow in the Sevier River if the water can be salvaged from existing uses. About 14,000 acre-feet of water (table 12) is discharged annually by evapotranspiration from 9,000 acres of marshes and wet meadowland which support growth of sedges and other phacelophytes. Probably about half of the 14,000 acre-feet could be salvaged by means of new drains or wells which would lower water levels in the gravel and sand deposits in the lower end of the basin and thereby decrease losses by evapotranspiration. The lowering of water levels, however, would undoubtedly decrease the flow of water from the Marshall Slough. The wells and drains used to lower water levels must be maintained within the wet areas if they are to lower water levels within these areas.

In addition to salvaging water, reduction of evapotranspiration would improve the productivity of some of the land by decreasing the precipitation of salts on the land surface. Fertilizers, if the land were drained, crops requiring much less water than do phacelophytes could then be grown. Lining of canals and mechanical eradication of phacelophytes are other methods of salvaging water.

CIRCLE VALLEY BASIN

Availability and storage of ground water

The valley fill is the main source of ground water in Circle Valley basin. The fill ranges in thickness from a thin edge near the valley margins to more than 600 feet near the center of the valley, where test holes have been drilled without penetrating bedrock (Bellis and Robinson, 1936, p. 18-21; Young, 1940, p. 2, 4-7). The valley fill consists of the flood-plain and alluvial-fan deposits, about 50-60 percent of which are well sorted and highly permeable. The fill in Circle Valley basin has the highest proportion of permeable material of any of the valley fill in the upper Sevier River basin. The most permeable deposits are along the Sevier River channel. Ground water in the valley fill is under artesian conditions at the lower end of the basin and under water-table conditions at the upper end of the basin (pl. 2).

In the artesian area, the subsurface movement of water is impeded by a ground-water barrier of volcanic breccia, and the water is confined in permeable sand and gravel under a layer of silty clay of low permeability which is 5-25 feet thick. The potentiometric surface in the artesian area ranges from about 5 feet above the land surface in well (C-20-3)19dec-1 to about 11 feet below the land surface in well (C-20-8)13dec-1. At the lower end of the basin the artesian area

contains springs and wet meadows. The artesian aquifers are recharged at the upper end and along the margins of the valley where the ground water is unconfined (pl. 3). The observed depth to the water table ranges from about 7 feet below the land surface in well (C-30-4)14ab-1 to about 94 feet in well (C-30-2)24dd-2.

An estimated 230,000 acre-feet of ground water is stored in the sand and gravel of the upper 250 feet of saturated valley fill in Circle Valley basin (table 7). The beds of sand and gravel are separated by saturated silt and clay of low permeability.

The bedrock formations that surround and underlie Circle Valley basin contain some ground water, but these formations generally are poor aquifers. Only one well, (C-30-2)16bb-1, is known to penetrate bedrock in Circle Valley basin, and it yields about 30 gpm of water from sedimentary or volcanic rocks of Tertiary age.

Recharge

Most of the ground water used in Circle Valley basin is obtained from springs which discharge from the valley fill. The largest of the springs are in the Mitchell Slough in sec. 17 and 18, T. 39 S., R. 2 W., and in sec. 18, T. 39 S., R. 4 W.; they have a combined discharge of about 3,670 gpm, and the water is used for irrigation and stock.

Several bedrock springs, which are in the mountains and plateaus surrounding Circle Valley basin, discharge less than 500 gpm each. Part of the public supply of Circleville is obtained from Circleville Spring, (C-30-4)16ab, which yielded 64 gpm in December 1966 from volcanic rocks of Tertiary age.

Other than from springs, ground water used in Circle Valley basin is obtained from only a few wells and drains which produce minor quantities of water. Pumped wells produce only about 640 acre-feet of water annually, and all except four wells are used for domestic or stock purposes. Well (C-30-4)28db-1 is pumped to supplement the Circleville public supply (Circleville Spring) during the summer, and it produces 31.50 acre-feet of water annually; well (C-30-4)28ad-3 produces about 4 acre-feet of water annually for a potato processing plant; and well (C-30-4)28ec-1, which is pumped for irrigation, yields most of the ground water pumped in the basin. The pumpage supplements a supply from the Sevier River, and it varies from four to six million gallons annually depending on the surface-water supply. The pumpage has varied from 0 in 1956 to 825 acre-feet in 1960, and it averaged about 500 acre-feet annually during the period 1957-62.

All the wells in the basin except one (C-30-3)16bb-1, tap valley fill, and individual well yields range from about 7 to 1,475 gpm. Drug wells range from 12 to 55 inches in diameter and from 12 to 90 feet in depth, and 18 drilled wells range from 1½ to 12 inches in diameter

and from 10 to 407 feet in depth. Most of the drilled wells are less than 200 feet deep. Three of the drilled artesian wells (C-30-3)19daa-1, (C-30-3)29baa-1, and (C-30-4)1adaa-1, flow, yield about 1-2 gpm of water each, and supply water for stock.

A few open drains have been constructed in the artesian area at the north end of Circle Valley basin. These drains, which are 3-8 feet deep and total about 3 miles in length, do not lower the water level appreciably because they are constructed in silty clay of low permeability, are not properly designed, and are inadequately maintained. They yield about 2,000 acre-feet of water to the Sevier River during most years.

Potential development

Wells that would yield several hundred gallons per minute could be constructed in the valley fill throughout Circle Valley basin, but wells drilled near the center of the valley would have the best yields. About 4,000 acre-feet of additional ground water could be withdrawn annually in Circle Valley basin without greatly affecting the flow in the Sevier River if the water can be salvaged from existing uses. Most of the water could be developed by lowering the water level in about 3,000 acres of wet phreatophyte-infested bottom land that comprises most of the artesian area. About 5,000 acre-feet of water is discharged by evapotranspiration annually in this wet area. Most of the area is wet because artesian ground water leaks to the land surface through the silty clay surface layer. Probably about half of the 8,000 acre-feet of loss could be salvaged by means of carefully spaced and designed wells and drains which would lower artesian heads in the sand and gravel deposits underlying the silty-clay layer. Furthermore, if the artesian head causing the upward leakage could be reduced, it would help alleviate water logging, but probably would result in a reduction of flow from the Mitchell Slough. This loss, however, would be compensated by water pumped from wells or obtained from more efficient drains. Lining of canals and mechanical emulsification of phreatophytes would salvage additional water.

EAST FORK VALLEY BASIN HENRY VALLEY SUBBASIN

Availability and storage of ground water

Ground water is under water-table conditions in the valley fill throughout Henry Valley sub-basin. Bedrock is near the land surface in most of the subbasin, and phyllite deposits at the downstream end indicate that ground water is impounded there. The valley fill is all alluvium and ranges from 0 to less than 100 feet in thickness (pl. 1, section E-A'), and about 10 percent is composed of sand and gravel.

The most permeable deposits are along the West Fork Sevier River channel. The observed depth to water ranges from about 4 feet below the land surface in well (C-36-4)34bds-2 to about 46 feet in well (C-36-3)6dbs-1. About 6,000 acre-feet of ground water is stored in the upper 100 feet of unstratified valley fill in the subbasin, and the principal water-bearing zones are beds of sand and gravel.

The bedrock underlying and surrounding the subbasin contains ground water, but the water-yielding characteristics of the bedrock and the quantity of water in storage are not well known. The available data, however, suggests that the bedrock facies are poor aquifers. Depth to water in the bedrock adjacent to the subbasin ranges from about 1 foot below the land surface in well (C-36-3)13ds-1 to about 892 feet in well (C-37-4)11ddd-1.

Existing use

Most of the ground water used in Henry Valley subbasin is obtained from the more than 50 wells that have been constructed in or adjacent to the subbasin. Nine of the wells in the subbasin obtain water from the valley fill, and the remainder obtain water from sedimentary formations of Tertiary or Cretaceous age. Wells in the valley fill generally yield less than 10 gpm, but one well, (C-36-4)34bds-1, is reported to yield 180 gpm. Discharge of wells penetrating bedrock ranges from less than 10 to 200 gpm. Most of the wells in or adjacent to the subbasin are drilled, range from 50 to 2,000 feet in depth, and range from 6 to 16 inches in diameter. Only two wells have been dug in Henry Valley subbasin; although several others adjacent to the subbasin were originally dug, they were later deepened by drilling.

Six wells are pumped for public supply and have a combined annual yield of more than 30 acre-feet; the other wells are pumped for domestic and stock use or are unused. Wells (C-36-3)7bbs-1 and (C-36-3)7bbs-2 penetrate the sedimentary formations of Cretaceous age underlying Henry Valley subbasin and supply water to the Federal Aviation Agency housing area near Bryce Canyon. Four wells supply water to Bryce Canyon National Park. Wells (C-36-4)32ds-1 and (C-36-4)34bds-2 obtain water from the valley fill; well (C-36-4)36ds-1, adjacent to the subbasin, penetrates limestone of the Wasatch Formation; and well (C-37-4)11ddd-2, also adjacent to the subbasin, penetrates sedimentary formations of Tertiary and Cretaceous age.

Some ground water is obtained from springs in and adjacent to the subbasin. Bryce Canyon National Park obtains water from a seep area in the valley fill of East Creek, NW $\frac{1}{4}$ sec. 36, T. 36 S., R. 4 W. The discharge of the seep ranges from 1 to 40 gpm, and the water is used for public supply. Other small springs and seeps in the valley

fill are used for stock watering and usually discharge less than 20 gpm.

Many small springs occur throughout the Wasatch and Kaiparowits Formations near Tropic Reservoir. The individual springs generally yield less than 10 gpm, and the water is used for stock.

Potential development

It is doubtful that wells capable of yielding more than 200 gpm could be pumped in Juniper Valley subbasin for irrigation without affecting streamflow. The most permeable aquifers are the flood-plain deposits of the East Fork Sevier River, but pumping wells close to the stream would cause losses in streamflow.

WATER TABLE CHARACTERISTICS

Availability and storage of ground water

Ground water is under water-table conditions in the valley fill throughout Juniper Valley subbasin. The fill is composed entirely of alluvium and consists in thickness from a thin edge on the valley sides to more than 250 feet in the center and east-central side of the valley (pl. 1, section C-C'). About 15 percent of the valley fill in the subbasin is composed of permeable sand and gravel. The most permeable deposits are near the East Fork Sevier River channel. The wet meadows at the lower end of the subbasin are evidence that ground water is impounded there by a hydrologic barrier (pl. 1, section E-E'). The observed depth to water in the valley fill ranges from about 10 feet below the land surface in test hole (C-27-2)273aa-1 to about 150 feet in test hole (C-24-2)240cd-1.

About 80,000 acre-feet of ground water is stored in sand and gravel beds in the upper 200 feet of saturated valley fill. The sand and gravel beds are the most permeable water-bearing deposits.

The sedimentary rocks of Tertiary and Cretaceous age underlying and surrounding the subbasin contain small quantities of water. The water-yielding characteristics of the bedrock and the quantity of water in storage are not known, but the available data suggest that, in general, the bedrock formations are poor aquifers. The observed depth to water in the bedrock ranges from about 30 feet in well (C-25-2)25dbb-1, which is adjacent to the subbasin, to about 300 feet in well (C-24-2)24dab-1.

Existing use

Most of the ground water used in Juniper Valley subbasin is obtained from springs which discharge from either the valley fill or bedrock. A considerable amount of ground water seeps from the valley fill into the East Fork Sevier River south of Black Canyon in secs. 11, 14, 16,

and 26, T. 42 S., R. 6 W. In this area, the stream gains about 4,000 acre-feet annually or 8 cfs in a channel length of about 2½ miles.

Large amounts of water are discharged from bedrock by springs in the plateaus adjacent to the subbasin. Two largest springs discharge from the Wasatch and Brian Head Formations of Tertiary age. The largest of these, Deer Creek Spring, (C 32 213340b), discharges about 1,040 gpm from fractures and joints in volcanic rock within the formations. Tom Best Spring, (C 34 212730c), discharges about 500 gpm from fractures and solution channels in the limestone of the same formations. Many other springs in Black Canyon discharge from the same formations along contacts between volcanic flows and an underlying conglomerate. Individual yields of these springs range from 60 to 460 gpm.

Little ground water is withdrawn from wells in the subbasin. The seven wells in the subbasin range in depth from 35 to 230 feet; one taps bedrock and six tap the valley fill. None of the wells were used in 1968.

Potential development

Information for yields of wells in John's Valley subbasin is not available, but wells that probably would each yield several hundred gallons per minute could be drilled into the flood-plain deposits of the valley fill along the East Fork Sevier River. Wells penetrating alluvial fans and benches probably would yield lesser amounts. It is estimated that wells yielding more than about 700 gpm each could be developed in the subbasin to furnish irrigation supplies without affecting runoff. Inasmuch as the most permeable aquifer in the subbasin is the flood-plain deposits of the East Fork Sevier River, pumping from wells in the lower part of the subbasin in areas C 4 and 34, T. 43 S., R. 2 W., probably would lower the water table and diminish the flow of the river.

ANTHONY SUBBASIN

Availability and storage of ground water

Ground water is under both artesian and water table conditions in the valley fill in Anthony subbasin. The valley fill, which is composed entirely of alluvium, generally is 60-75 feet thick in most parts of the subbasin, although in the valley bottom it is more than 200 feet thick (pl. 1, section G-E²). About 40 percent of the valley fill in Anthony subbasin is permeable gravel and sand. The fill in this subbasin has the highest proportion of permeable material of any in the East Fork Valley basin. The most permeable deposits are along the channel of the East Fork Sevier River.

The water is under artesian conditions in the lower part of the subbasin (pl. 2) where subsurface movement is impeded by a barrier

formed by bedrock near the head of Kingscon Canyon in sec. 33, T. 21 S., R. 2 W. The water is in beds of permeable sand and gravel, and it is confined by 5-20 feet of overlying silty clay of low permeability. The potentiometric surface is near the land surface throughout the alluvial area, which is marked by marshes, wet meadowland, and seepage areas. The alluvium aquifers are recharged in the upper part and along the margins of the valley where ground water is unconfined (pl. 2).

Bedrock is near the surface in most parts of the valley, and the observed depth to water in the valley fill in the water-table area ranges from about 11 feet below the land surface in well (C-31-2) 230ca-1 to about 14 feet in well (C-32-2) 230da-1.

Ground water also occurs in the bedrock of Tertiary age underlying the valley fill and adjacent to the mountains, and the observed depth to water in the bedrock underlying the Alluvial ranges from about 26 feet in well (C-31-2) 230cd-1 to about 163 feet in well (C-31-2) 231aa-1.

About 28,000 acre-feet of ground water is stored in the sand and gravel of the upper 100 feet of saturated valley fill in Anthony subbasin. Additional ground water is stored in the bedrock underlying and adjacent to the subbasin, but the water-yielding characteristics of the bedrock and the quantity in storage are not known.

Recharge

Most of the ground water used in Anthony subbasin comes from springs in the valley fill in the subbasin or from bedrock in the surrounding plateaus and adjacent to the valley floor. As much as 3 cfs, or 4,800 acre-feet, of ground water seeps from the valley fill in the alluvial area in the north end of the subbasin into the East Fork Snake River.

Bedrock springs on the Sevier Plateau outside the subbasin yield water for public supply to Anthony and Kingston. Anthony Spring, (C-31-2) 191a, discharges about 220 gpm from volcanic rocks of Tertiary age. Kingston is supplied by a spring in Kingscon Canyon, (C-30-2) 22aa, which yields about 15 gpm from volcanic rocks of Tertiary age.

Ground water has been fully developed by wells in Anthony subbasin. Of 16 wells in the subbasin, 12 are pumped for domestic and stock use and 4 are unconfined. The wells obtain water from the valley fill and from permeable zones in volcanic rocks or conglomerate of the Wasatch and Brian Head Formations. Yields of individual wells penetrating the valley fill average about 20 gpm and yields of wells penetrating bedrock range from about 4 to 36 gpm. Drilled wells

generally range from 4 to 6 inches in diameter and from 40 to 180 feet in depth.

A few open drains, which discharge about 3,000 acre-feet of water annually, have been excavated in the silt and clay overlying the artesian aquifer. The drains, which are 1-3 feet deep and total about 4 miles in length, are ineffective in lowering the water level because they are not deep enough to penetrate the underlying permeable beds of sand and gravel, are improperly designed, and are inadequately maintained.

Potential developments

Possibly 3,000 acre-feet of additional ground water could be withdrawn from wells and drains annually in Antimony subbasin without greatly affecting streamflow if water can be salvaged from existing uses. Construction of pumped wells and drains designed to penetrate confined aquifers would reduce artesian head and help drain the wet bottom land. The wells and drains could result in salvage of about 4,000 acre feet of water annually, which is approximately half of the annual loss of 5,200 acre-feet by evapotranspiration from about 2,100 acres of wet bottom land. Furthermore, crops requiring less water than phreatophytes could be grown on the drained land.

BEAR VALLEY BASIN

KOOCHAREM SUBBASIN

Availability and storage of ground water

Ground water is under both artesian and water-table conditions in the valley fill in Koochareem subbasin. The valley fill, most of which is alluvium, is more than 300 feet thick in the center of the valley south of Koochareem and more than 779 feet thick in the valley about 1 mile northeast of Greenwich (pl. 1, section *N-P*; see also Bellis and Robinson, 1968, p. 27-31). About 15 percent of the alluvium in the subbasin is permeable sand and gravel. The most permeable deposits are confined layers of sand and gravel in the lake(?) or marsh(?) deposits near the channel of Otter Creek between the vicinity of Burville and Greenwich.

Ground water is under artesian conditions throughout most of the valley fill (pl. 3), and the observed piezometric surface ranges from about 15 feet below the land surface in well (D-25-1)282b-1 to more than 31 feet above the land surface in well (C-20-1)282b-1. The water is confined under layers of silt and clay in the more permeable beds of sand and gravel that slope from the sides of the valley toward the center. The marsh and meadowland and the discharge of ground water to Otter Creek at the lower end of the valley indicate that ground water is impounded there by a bedrock constriction.

The artesian aquifers are recharged through permeable alluvial-fan deposits along the valley sides where the ground water is unconfined (pt. 9). The observed depth to water in the water-table areas ranges from about 8 feet below the land surface in well (C-27-1) 21ba-1 to about 120 feet in well (C 27 1)20da-1.

About 90,000 acre-feet of ground water is stored in the sand and gravel of the upper 200 feet of estimated valley fill in Koocharem subbasin. Small amounts of ground water are also in the volcanic rocks of Tertiary age underlying and adjacent to Koocharem subbasin, but the quantity in storage and the water-yielding potentialities of the rocks are not known.

Existing uses

Springs issuing from bedrock or the valley fill yield most of the ground water used in Koocharem subbasin. The bedrock springs on the surrounding plateaus and adjacent to the valley floor discharge from volcanic rocks of Tertiary age. Two of the largest are Rose Springs, (C-25-1)20ba, which yield about 1,420 gpm, and Red Cedar Green Springs, sec. 46, 14, and 28, T. 20 S., R. 1 W., which yield about 340 gpm. Many small springs and seeps issue to the valley fill, and they have a combined yield of several hundred gallons per minute. Many of the springs and seeps are on the toes of alluvial fans on the valley sides, and others are adjacent to Otter Creek.

Most of the water from springs in Koocharem subbasin is used for irrigation and stock; however, part of the discharge of Rose Springs is used for public supply in Burreville, and the discharge from Beaver Spring, (D 26 1)20ab, is used for public supply at Koocharem. Both springs discharge from volcanic rocks of Tertiary age.

More ground water is withdrawn from wells in Koocharem subbasin than in any of the other ground-water basins or subbasins in the upper Sevier River basin. Wells produce more than 2,400 acre-feet of water annually in this subbasin, mostly from flowing artesian wells. Of the approximately 154 wells that have been constructed in the subbasin, all but 1 obtain water from the valley fill and 143 are flowing artesian wells, 9 of the wells are dug, 13 are drilled, and 133 are jetted. The dug wells range from 10 to 100 feet in depth and from 20 to 120 inches in diameter, the drilled wells from 79 to 810 feet in depth and from 4 to 10 inches in diameter, and the jetted wells from 11 to 279 feet in depth and from 1 to 8 inches in diameter. Yields of individual wells penetrating the valley fill range from about 0.1 to more than 120 gpm; the well that penetrates bedrock, (C-27 1)20da-1, yields about 20 gpm. Most of the wells are used for domestic and stock purposes, but about 25 are used solely for

irrigating features. Individually owned wells are used for domestic water supply in Grosvenor, which has no public-supply system.

The 35 irrigation wells are flowing wells which discharge about 1,200 acre-feet of water annually. These wells are mostly 200-250 feet deep, are 2 inches in diameter, and obtain water through the open end of unperforated casing. Generally only 20-30 feet of casing was installed in these flowing wells, and the rest of the hole commonly has collapsed and restricted the flow. Many of these wells were constructed before 1939, and the casings have almost rusted away. Many local road spots, 10-50 feet in diameter, mark places where flowing wells once existed but have been virtually obliterated.

Drains have been installed in Kowshoran subbasin to develop ground water. However, some discharges by Red Cedar Grove Springs area, sec. 23, T. 26 S., R. 1 W., convey water from the springs for irrigation downstream.

Potential development

More than 8,000 acre-feet of water per year is discharged by evapotranspiration from about 6,800 acres of wet bottom land in Kowshoran subbasin. It is doubtful, however, that much of this water could be salvaged by additional withdrawal of ground water from the artesian areas without greatly affecting present water use. Lowering artesian heads would affect most of the flowing wells and the flow of artesian springs into Otter Creek. Otter Creek gains water in Kowshoran subbasin largely by upward leakage from artesian aquifers, and wells of large discharge would reduce artesian head and in turn reduce the discharge of ground water to the stream. However, constructing drains, lining canals, and stabilizing phreatophytes could salvage some water in the subbasin.

ANGLE SUBBASIN

Availability and storage of ground water

The valley fill is the main source of ground water in the Angle subbasin. The thickness of the valley fill, which is mostly alluvium, ranges from a thin edge near the valley margins and near bedrock outcrops within the valley to 400 feet near Angle, as indicated by the log of test hole (C-20-2)2524a-1 (Feltus and Robinson, 1963, p. 31). About 15 percent of the valley fill is permeable sand and gravel. The most permeable deposits are near the channel of Otter Creek.

Ground water is mostly under water-table conditions in the valley fill throughout the subbasin, but it may be under artesian conditions near the north end of Otter Creek Reservoir. The increased depth to water in the valley fill in Angle subbasin averages about 60 feet

below the land surface. Wells do not penetrate the bedrock underlying or adjacent to Angle subbasin, but knowledge of springs in the bedrock suggests that small quantities of water are available in bedrock.

About 80,000 acre feet of ground water is stored in the sand and gravel of the upper 200 feet of estimated valley fill in Angle subbasin. Thin (principal) water-bearing zones in the valley fill are deposits of sand and gravel.

Available use

Springs in bedrock or the valley fill provide most of the ground water used in Angle subbasin. The bedrock springs discharge from volcanic rocks of Tertiary age in the surrounding plateaus or adjacent to the valley floor. The water from the largest springs, Pale Canyon Spring, (C-29-2)15c, which discharges about 270 gpm, and Paul's Spring No. 1, (C-30-1)5b, which discharges about 225 gpm, is used for irrigation and stock. A small amount of ground water seeps from the valley fill bordering Otter Creek just above Otter Creek Reservoir and is used for irrigation and stock.

Of the total of seven wells in Angle subbasin, two are dug and five are drilled, all are used for domestic and stock purposes, and all penetrate the valley fill. Individual wells yield from about 5 to 10 gpm, although wells constructed by modern methods could yield as much as 100 gpm. The drilled wells range from 63 to 197 feet in depth and from 2 to 8 inches in diameter.

Potential development

Lowering water levels in Angle subbasin by means of additional wells and drains could salvage some water lost by evapotranspiration near the upstream end of Otter Creek Reservoir. However, inasmuch as the most permeable deposits are near Otter Creek, it is doubtful that well yielding more than about 500 gpm could be pumped without greatly affecting the flow of the creek.

IMPACTS OF PUMPING ADDITIONAL GROUND WATER IN THE UPPER SERRIA TRIBUTARY BASIN

Pumping additional water from wells in any of the ground-water basins in the upper Sierra River basin would eventually lower the water level and reduce erosion beds in that basin. The amount of water-level decline would be approximately proportional to the net amount of water pumped. If water is pumped from wells penetrating alluvial aquifers, the water-level decline would spread rapidly over a relatively large area and would eventually affect adjoining water table areas. If the water is pumped from wells penetrating water-table aquifers, the water-level decline would spread

slowly and be limited largely to an area in the vicinity of the pumped wells. If pumping from the water-table aquifers is continued long enough, the water-level decline would eventually extend to the artesian areas and reduce artesian head.

Important benefits could result from reducing artesian pressures. Pressure reduction would reduce or stop the seepage of ground water to the land surface, mostly at the lower ends of the basins. Eventually many sloughs and waterlogged areas would dry up except some wet areas that may be sustained by shallow movement of water from adjacent irrigated lands. Much of the water now being discharged by evapotranspiration in these areas might be salvaged and used beneficially. In addition, the waterlogged land, now impregnated with salts that are deposited when the ground water evaporates, could eventually be reclaimed if irrigation water were applied at intervals to leach the salts from these soils. If the overall use of water were more efficient, more water would be available to satisfy local and downstream demands.

Streamflow would decrease if water levels were lowered appreciably in the valley fill. In water-table areas adjacent to streams, lowering water levels would increase the hydraulic gradient from the stream bed to the reservoir, and seepage from the stream bed would thus be increased. In artesian areas the hydraulic gradient is from the ground-water reservoir to the streams. Although the streams are separated from the aquifers by layers of relatively impermeable silty clay, sand, amounts of water seep through the clay and discharge into the streams.

The construction of additional wells in the upper Sevier River basin should be carefully planned. The first production wells should be spaced several miles apart, and water levels should be measured periodically in a network of observation wells to determine the amount and extent of the change resulting from pumping. In artesian areas, the discharges of springs and flowing wells in the vicinity of production wells should be measured periodically to observe changes. In the water-table areas, where water levels are near the altitude of the streams, production wells should be at least half a mile from streams so that the cone of depression does not reach the streams. To be most effective, ground-water development should be coordinated with improvement of surface-water diversion, more effective drainage, improved distribution systems, and phreatophyte control. The most efficient use of water in the basin would require that the ground-water reservoir be managed in a way similar to the management of surface-water reservoirs.

About 12,000 acre-feet of water per year, in addition to the amount now pumped, eventually could be developed from the ground-water

reservoirs in the upper Sevier River basin. The 14,000-acre-feet would be salvaged from water now discharged by evapotranspiration from wet areas that support phreatophytes.

QUALITY OF WATERS

The chemical quality of the ground water in the upper Sevier River basin is good for most uses. The following sections describe the mineral constituents found in the water and the quality of the water in relation to use.

DISSOLVED MINERALS

The major chemical constituents in the water of the upper Sevier River basin are silica, calcium, magnesium, sodium, potassium, chloride, sulfate, and nitrate. The chemical constituents commonly present in smaller amounts are iron, fluoride, manganese, and boron. Other properties and characteristics that help describe water quality are temperature, specific conductance, pH, and hardness. Chemical analyses of water from several wells and springs in the basin and from a few sites along the Sevier River and its tributaries are included in a compilation of basic data by Carpenter, Robinson, and Bjorklund (1964).

QUALITY IN RELATION TO USE

SALINITY

The characteristics of water that appear to be most important in determining the suitability of water for irrigation are "(1) total concentration of soluble salts; (2) relative proportion of sodium to other cations; (3) concentrations of boron or other elements that may be toxic; and (4) under some conditions, the bicarbonate concentration as related to the concentration of calcium plus magnesium" (U.S. Salinity Lab. Staff, 1955, p. 69).

1. The total concentration of soluble salts, or salinity, may be expressed in units of dissolved solids concentration or of specific conductance. Chemical analyses were made of samples of ground water from 32 wells and 33 springs in the upper Sevier River basin. The dissolved solids range from 80 to 773 ppm (parts per million) and average 265 ppm for 40 samples, and the specific conductance ranges from 80 to 760 micromhos per centimeter and averages 338 micromhos per centimeter for 40 samples. Thus, the ground water has a salinity hazard that ranges from low to medium for irrigation, according to the classification of the U.S. Salinity Laboratory Staff (1951, p. 70-211).

The relation of dissolved solids and specific conductances for surface water in the upper Sevier River basin at certain times of the year is quite similar to that of ground water. (See fig. 6.)

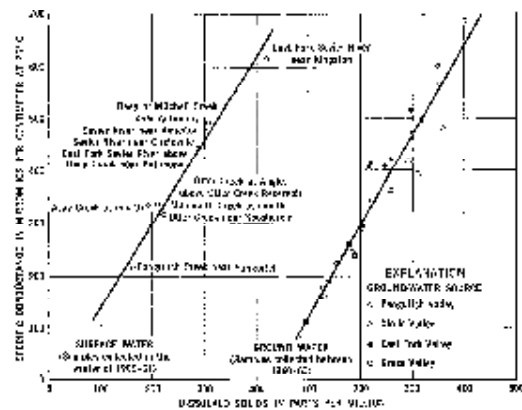


FIGURE 6.—Graphs showing the relation between the amount of solids and specific conductance of selected surface and ground water samples.

Chemical analyses of 10 samples of surface water collected during the winter of 1960-61 indicate that the dissolved solids ranged from 168 to 460 ppm and averaged 262 ppm, and the specific conductance ranged from 216 to 518 micromhos per centimeter and averaged 304 micromhos per centimeter. The surface-water samples were collected during a period of low flow when most of the streamflow was derived from ground water. During periods when much of the streamflow is derived from snowmelt or rainfall, however, the dissolved-solids content generally is less.

2. The proportion of sodium to other cations, and the probable extent to which a soil may adsorb sodium from water (and thereby become less permeable) is expressed in terms of the sodium-adsorption ratio (SAR). The SAR of the ground water in the upper Sevier River basin ranges from 0.1 to 1.8 and averages about 0.5. Thus the ground water in the basin has a low sodium hazard for irrigation, according to the classification of the U.S. Salinity Laboratory Staff (1954, p. 79-81).
3. A small quantity of boron is essential to the normal growth of all plants, but excessive concentrations are toxic. Toxicity varies

according to the tolerance of individual species (U.S. Salinity Lab. Staff, 1961, tables 9, 10). In general, water containing less than 0.23 ppm of boron is not harmful to any plant, whereas water containing more than 3.75 ppm may be toxic to all crops. The amount of boron in 23 ground-water samples collected in the upper Sevier River basin ranges from 4.02 to 6.14 ppm and averaged 0.65. These small concentrations are not harmful to plants.

4. The relation between the bicarbonate concentration and the concentration of calcium plus magnesium is expressed as residual sodium carbonate (RSC). The U.S. Salinity Laboratory (1957, p. 80) states that "waters with more than 3.5 meq per l (milliequivalents per liter) residual sodium carbonate are not suitable for irrigation purposes." None of the ground-water samples collected in the upper Sevier River basin had a RSC that exceeded 3.5 meq per l.

Ground water in the valley fill in Panguitch, Circle, and Cross Valleys deteriorates in quality slightly from the upper to the lower end of each valley (pl. 4). Although few data are available for the quality of water in the valley fill of East Park Valley, the fluctuation in quality of the surface water deteriorates downstream indicates that this deterioration probably occurs in the ground water. The deterioration in quality in all the valleys in a downstream direction is due largely to use and reuse of water for irrigation.

DOMESTIC AND PUBLIC SUPPLY

The U.S. Public Health Service (1960) has recommended the following maximum concentrations for some of the more common constituents in water used for domestic and public supply:

Constituent	Limit (mg per liter)
Chloride	250
Fluoride	1
Iron	0.3
Manganese	0.05
Nitrate	45
Sulfate	250
Dissolved solids	500

The maximum maximum sulfate concentration is variable, depending on the location. The maximum similar to that at Panguitch, the maximum recommended fluoride concentration is 1.8 ppm. (See U.S. Public Health Service, 1962, p. 3.)

The concentrations of abundant constituents observed in samples of ground water from the upper Sevier River basin consistently are less than the maximum recommended by the Public Health Service. The recommended concentrations were exceeded in a few of the ground

water samples as follows: the concentration of fluoride in three samples, iron in eight samples, manganese in one sample, and dissolved solids in two samples. The recommended fluoride concentration was exceeded in samples collected from spring (C-30-3)24cab (3.2 ppm), well (C-30-3)16bb-1 (3.1 ppm), and well (C-30-4)264b-1 (3.3 ppm). Igneous rocks often yield water with a high fluoride concentration, and springs (C-30-4)24cab and well (C-30-3)16bb-1 tap volcanic rocks of Tertiary age and sedimentary or volcanic rocks of Tertiary age, respectively. Well (C-30-4)264b-1 obtains water from valley fill which is derived from volcanic rocks.

The recommended iron concentration was exceeded in samples collected from wells (C-30-5)13cab-1 (3.0 ppm) and (C-30-3)103bb-1 (0.93 ppm) and from springs (C-30-3)117d (0.82 ppm) and (C-30-3)10ba (0.8 ppm); it was also exceeded in two samples from well (C-30-4)264b-1 (2.8 and 1.6 ppm) and well (C-30-3)6dba-1 (1.0 and 0.90 ppm). The source of the iron is believed to be igneous- or metamorphic rocks that supply water directly to six of the eight springs and wells. Wells (C-30-4)264b-1 and (C-30-3)6dba-1 tap valley fill that is derived largely from igneous- and carbonate-type rocks, respectively. Some of the iron, however, may possibly be derived from the well casing or pipe-conduit systems.

The recommended manganese content was exceeded in a sample from a well (C-30-3)16bb-1 (0.13 ppm). The well obtains water from volcanic or sedimentary rocks of Tertiary age which are probably rich in manganese.

The recommended dissolved solids content was exceeded in samples from two wells. The high concentration in the water from well (C-30-3)7cab-1 (778 ppm) may be caused by return flow from irrigation. The high concentration in the water from well (C-30-3)24cab-1 (612 ppm) may be due to the fact that the sample was collected during deepening of the well and could have been contaminated with drilling fluid.

The hardness of water is important in domestic and public supply because soap consumption for washing and laundering increases as the hardness increases and hardness causes part of the incrustation (boiler scale) found in pipes, coils, and boilers. The U.S. Geological Survey uses the following classification for hardness of water: less than 60 ppm, soft; 61-120 ppm, moderately hard; 121-180 ppm, hard; and more than 180 ppm, very hard. Water having a hardness of more than 200 ppm needs to be softened for most purposes.

Of the ground-water samples from 24 wells and 25 springs for which hardness was determined, 3 contained less than 60 ppm of hardness, 16 contained 61-120, 3 contained 121-180, and 19 contained more than

180. The hardness of the water in the 47 samples ranged from 35 to 508 ppm and averaged 170 ppm. The hardness is generally highest in water obtained from the valley fill and lowest in water obtained from volcanic rocks of Tertiary age. The three samples which contained less than 50 ppm of hardness are all from springs in volcanic rocks of Tertiary age: springs (C-20-6) 36cb (67 ppm), (C-20-4) 11ah (27 ppm), and (11-28-1) 37nb (17 ppm). Most of the samples containing more than 40 ppm of hardness are from the valley fill, but some are from consolidated sedimentary rocks. Plate 3 shows graphically the values of hardness for a few selected samples collected from the valley fill or bedrock.

LIVESTOCK

Although animals are more able to tolerate water having a high dissolved-solids content than man, prolonged periods of drinking highly mineralized water may cause physiological disturbances such as wasting, gastrointestinal disorders, disease, and even death. Other effects include reduced lactation and rate of reproduction. The State of Montana (W. E. Storer, oral commun., 1951) rates water containing less than 2,500 ppm of dissolved solids as good for livestock use, from 2,500 to 3,500 ppm as fair, from 3,500 to 4,500 ppm as poor, and more than 4,500 ppm as unfit. On the basis of this classification, the water sampled in the upper Snake River basin is good for livestock.

INDUSTRY

The chemical characteristics of water that are most important in determining the suitability of the water for industrial use vary according to the particular use involved and the product manufactured. Two characteristics that are significant in practically all industries, however, are hardness (discussed in this section on "Domestic and public supply") and silica content. Silica forms a hard, adherent scale in boilers; Moore (1940, p. 263) has suggested the following allowable concentration of silica in water for boiler operating at various pressures: for a pressure less than 150 psi (pounds per square inch), 40 ppm; 150-250 psi, 20 ppm; 250-400 psi, 5 ppm; and more than 400 psi, 1 ppm.

Of the ground-water samples collected from 28 wells and 23 springs in the upper Snake River basin that were analyzed for silica, 17 contained more than 40 ppm of silica, 24 contained more than 20 ppm, and all but 1 contained more than 5 ppm. The average silica content of the ground-water samples was 32 ppm. The sample with less than 5 ppm silica was from well (C-27-1) Lidd-1 (1.7 ppm) which derives water from limestone of the Wasatch Formation. In the upper Snake River basin, igneous rocks generally yield water having the greatest

content of dissolved silica and limestone yields water that contains the least silica.

Temperature is an important characteristic of water used for cooling. Low temperatures, of course, are preferred, and water having a relatively constant temperature is considered desirable. The temperature of water from wells in the upper Sevier River basin commonly ranges from 53° to 54° F. The average temperature of water from 261 wells is 53° F, the range being from 41° to 61° F; the average temperature of water from 47 springs is 51° F, the range being from 40° to 63° F. By comparison, the temperature of surface water in the basin varies with the season and the stream and ranges from freezing to tepid. The temperature of the water from a spring in sec. 47, T. 33 S., R. 5 W., is 80° F; however, this water issues from considerable depth along a fault, and its temperature is not representative of ground-water temperature in the basin.

SUMMARY

The upper Sevier River basin contains four ground-water basins which were formed by geological processes including faulting and erosion and flow. They are Paungnitck Valley basin, Circle Valley basin, East Fork Valley basin, and Grass Valley basin. East Fork Valley basin is divided into Emery Valley, John's Valley, and Ardenroy subbasins. Grass Valley basin is divided into Koshareta and Angle subbasins.

Ground water occurs under both artesian and water-table conditions in the valley fill in Paungnitck and Circle Valley basins and in Ardenroy and Koshareta subbasins. It is under water-table conditions in the valley fill in John's Valley, Emery Valley, and Angle subbasins. In Paungnitck and Circle Valley basins and Ardenroy subbasins, the artesian conditions are on the downstream ends, and the water-table conditions are at the upstream ends. Ground water is under artesian conditions throughout most of Koshareta subbasin but is under water-table conditions in places along the sides. Depth to water in wells in the valley fill ranges from practically 0 to about 100 feet below the land surface. Many wells flow in the artesian areas, and artesian heads reach a maximum of about 80 feet above the land surface.

The valley fill in the basins and subbasins consists of gravel, sand, silt, and clay. An average of about 26 percent of the valley fill is permeable sand and gravel which yields water readily to wells and springs. The approximate percentages of sand and gravel in the valley fill are: 26-30 percent in Paungnitck Valley basin, 30-40 percent in Circle Valley basin, 30 percent in Emery Valley subbasin, 13 percent in John's Valley subbasin, 43 percent in Ardenroy subbasin, 15 percent in Koshareta subbasin, and 15 percent in Angle subbasin.

About 1 billion acre feet of ground water plus is recoverable by wells is stored in the upper 200 feet of unconsolidated valley fill in the various basins and subbasins. The amounts of water in the sand and gravel deposits are (in acre-feet): Pangnitch Valley basin, 570,000; Circle Valley basin, 210,000; Crazy Valley subbasin, 6,000; Johna Valley subbasin, 90,000; Antimony subbasin, 84,000; Roundstone subbasin, 80,000; and Angle subbasin, 80,000. The silt and clay deposits in such basins and subbasins contain large quantities of water, but little of this water is readily available to wells. Some of the water in the silt and clay, however, is indirectly available to wells because it would move into the permeable gravel and sand deposits if water were removed from those deposits.

The bedrock surrounding and underlying the various basins and subbasins also contains ground water, but the quantity is not known. In places the bedrock will yield significant amounts of water to wells, but in most of the basin the bedrock has low permeability.

The ground-water reservoirs are recharged mostly by the Serier River and its tributaries at the upper ends and sides of the ground-water basins and by seepage from irrigation systems and irrigated lands in water-table areas. Inflow from bedrock aquifers surrounding the valleys also recharges the reservoir. The ultimate source of all recharge is precipitation within the upper Serier River basin.

Water is discharged from the ground-water reservoir by flowing and pumped wells, springs, drains, evapotranspiration, and subsurface outflow. The discharge in 1962 from the valley fill by wells was about 3,000 acre-feet, by drains about 3,600 acre-feet, by springs about 33,000 acre-feet (springs in bedrock discharged an additional 75,000 acre-feet), and by evapotranspiration from areas of phreatophytes about 15,000 acre-feet. A slight decline in ground-water levels in the valley fill during the 1938-63 period indicates that the total discharge of ground water slightly exceeded the recharge.

The surface- and ground-water systems in the upper Serier River basin are interrelated, and increasing the ground-water discharge will, in general, decrease the surface-water discharge. The most efficient use of water in the basin, however, requires that the ground-water reservoir be managed in a way similar to the management of surface-water reservoirs.

About 43,000 acre-feet of the ground water discharged in the upper Serier River basin is consumed by phreatophytes in wet areas in the valleys; part of this water might be salvaged without significantly decreasing surface-water discharge and ground-water discharge from existing wells, springs, and drains. If new large wells and drains were carefully designed and spaced, they could lower water levels

enough to dry up wet areas; thus about 14,000 acre-feet of water could be salvaged, and large decrease would result in the flow of existing wells, springs, and streams in most basins.

Of the 14,000 acre-feet of water to be salvaged from existing uses, about 7,000 acre feet could be supplied by wells and drains in Hanzguth Valley basin, about 4,000 acre-feet could be supplied by wells and drains in Circle Valley basin, and about 3,000 acre feet could be supplied by wells and drains in Antimony subbasin. Additional withdrawal of ground water, however, in (1) Johns Valley or Emery Valley subbasins, would ultimately decrease the flow of East Fork Sevier River and in (2) Koocheckum or Angle subbasins would decrease the yield of flooding wells and the flow of Otter Creek.

The ground water in the upper Sevier River basin generally is suitable in chemical quality for irrigation, domestic and public supply, livestock, and industry. The dissolved-solids content of the ground water within individual basins generally increases downstream, owing mostly to repeated use of the water for irrigation.

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Attachment F – Water Quality Handling & Analysis Plan

WATER QUALITY SAMPLING, HANDLING, AND ANALYSIS PLAN

**A Compliance Document for
Groundwater Discharge Permit Application**

FOR:

**DALTON FINISHER SITES
GARFILE COUNTY, UTAH**

November 29, 2017

**Prepared For:
Dalton Hay Company, LLC
P.O. Box 189
Circleville, Utah 84723**

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Table A-1	Base Line Water Sample Analysis Parameters
Table A-2	Steady State Water Sample Analysis Parameters
Figure A-1	Monitor Well Water Level Measurements Log
Figure A-2	Water Quality Sampling Field Record
Figure A-3	Field Water Sample Label
Figure A-4	Field Water Sample Chain-of-Custody Record

Water Quality Handling and Analysis Plan

1.0 INTRODUCTION

The following **Water Quality Sampling, Handling and Analysis Plan** (The Plan) presents the organization and procedures for water quality investigations near Delta, Utah. This plan is required by the Utah State Department of Environmental Quality (DEQ), Division of Water Quality as a condition of the Final Ground Water Discharge Permit for the Dalton Finisher Hog Production Sites.

1.1 Implementation

The Plan is submitted as a Compliance Document for the Utah Ground Water Discharge Permit (“the Permit”). The Plan has been approved by Jade Dalton and Circle Four Farms.

2.0 PROJECT DESCRIPTION

2.1 Purpose

Specific objectives of the Groundwater Monitoring Plan:

- A. To evaluate background water quality at the Dalton Finisher Site approximately 2.6 miles south of Circleville, Utah.
- B. To provide information for the DEQ to establish ground water protection levels for the facility.
- C. To establish procedures for groundwater monitoring and sample collection at the facility.

2.2 Methodology

Engineering Activities for Achieving the Specific Objectives: Water quality data reports will be submitted to the DEQ on a regular schedule, in accordance with the requirements of the Groundwater Quality Discharge Permit for the facility.

- A. Installation of monitoring wells in the most shallow aquifer, upgradient and downgradient from the facility.
- B. Measurement of groundwater elevations at the monitor wells.

Water Quality Handling and Analysis Plan

- C. Evaluation of hydrologic gradients in all aquifers penetrated by monitor wells.
- D. Collection and analysis of ground water quality samples from the monitor wells according to a schedule recommended by the Utah State Division of Water Quality in the Permit.
- E. Preparation and submission of quarterly “Groundwater Sampling Reports” during the one year accelerated background monitoring period.

3.0 PROJECT ORGANIZATION AND RESPONSIBILITY

3.1 Organization

Organization for studies and field investigations required by this Plan

- A. **Construction Management Company:**
 - Dalton Finisher Sites
 - Contact: Jade Dalton - Owner
 - Construction Manager (CM) will be appointed by Jade Dalton.

- B. **Quality Assurance Company:**
 - GEM Engineering, Inc.
 - Contact: Joel A. Myers, P.E. – President
 - Quality Assurance Officer (QAO) will be appointed by GEM Engineering.

- C. **Department of Environmental Quality Official:**
 - Ed Hickey, P.G. – Environmental Scientist
 - State of Utah – Department of Environmental Quality
 - Division of Water Quality

3.2 Responsibilities

- A. The CM and the QAO review and conduct or oversee the field activities described in the Plan. They will review all data generated during the investigation and will be responsible for validating and submitting data to the DEQ.

Water Quality Handling and Analysis Plan

- B. Analytical results of each completed sampling round will be submitted to the Division of Water Quality.
- C. The CM and the QAO will review and approve the Plan, review all quality control data and identify problems, if any. The QAO will report directly to the CM and recommend corrective measures.
- D. The state official will advise the owner of any comments, or objections to the Plan, its implementation, or any proposed changes to the Plan.

4.0 MONITOR WELL INSTALLATION

4.1 Site

Monitor wells are installed in the shallowest aquifer where unconsolidated quaternary sand and gravel contain unconfined water.

4.2 Construction

Requirements for monitor wells constructed for the Dalton Finisher facilities are included in the section of the Groundwater Discharge Permit Report. Unless required by the Division of Environmental Quality additional specifications will not be included as part of this Plan.

4.3 Published Standards

Well construction conforms to the EPA RCRA Groundwater Monitoring Technical Enforcement Guidance Document and the National Water Well Association's Handbook of Suggested Practices for the Design and Installation of Groundwater Monitoring Wells.

5.0 ANALYTICAL PARAMETERS AND QA OBJECTIVES

Required analytical parameters and holding times are given in Tables A-1 and A-2. Specific conductance, temperature and pH will be measured in the field. Table A-1 provides parameters which will be analyzed on a quarterly basis, until the State official determines an adequate base line has been established. After this the samples will be analyzed on a semi-annual or annual basis, as determined by the state, for the parameters listed in Table A-2.

Water Quality Handling and Analysis Plan

5.1 Procedures

- A. Check analyses for the field parameters pH and specific conductance will be run in the laboratory. Chemical analysis for all certified constituents will be performed by a commercial laboratory certified under either, The Clean Water Act, The Safe Drinking Water Act or The Resource Conservation and Recovery Act.

5.2 Quality Assurance

- A. Internal quality assurance for this project will be in accordance with the Utah DEQ protocol. Laboratory certification will be monitored by the QAO.
- B. Routine analysis of samples will be performed in accordance with standard EPA procedures. Special analyses will be performed according to EPA methods for chemical analyses of water and wastes.
- C. Specific analytical methodologies and references are listed in Table A-1. These methodologies specify the documentation needed to complete and evaluate the data. They also define acceptable accuracy and precision criteria that must be met for the data to be considered valid.
 - 1. Accuracy: defined by the EPA as the percent recovery of a spiked sample. Laboratory matrix spikes are actual field samples spiked in the laboratory with a representative group from the list of required parameters as per Table A-1. One sample per alternate set of field samples will be split for matrix spike analysis.
 - 2. Precision: defined by the EPA as the relative percent difference of duplicate sample analyses of similar matrix.
- D. Re-sampling will be required if contaminant concentration in a trip blank (to be submitted on alternate sampling rounds) are within one order of magnitude of actual field sample concentrations.

5.3 Data Quality Objectives

- A. The data collected as part of this investigation is intended for use by the State of Utah DEQ and by Blue Mountain Dalton Finisher and its consultants.

Water Quality Handling and Analysis Plan

- B. Laboratory and field procedures have been designed to provide a high confidence level in the analytical results based on precision, accuracy, completeness and comparability.

5.4 Data Quality Control Management

- A. Field data quality control will be managed by the QAO in consultation with the State DEQ official for each type of data defined in this Plan.
- B. Field data will be compared to previously collected data at the site to test for probable consistency. Historic data will also be assessed for accuracy to assure consistency and comparability of all data taken at the site.
- C. Data will be compared in the same area and / or at similar depths during this study to determine whether or not the results are reasonable and consistent.
- D. Unreasonable data points will be evaluated by technical personnel who will decide whether re-sampling or retesting are required.

6.0 FIELD PROCEDURES

This section presents the water quality research methods for water level measurements, sample collection and handling.

6.1 Water Level Measurements

- A. Static water level measurements are to be made in all monitor wells during this investigation. Water levels will be measured before sampling with a steel tape or electric sounding device to the nearest 0.01 foot. The measuring device and reel will be cleaned with distilled water before and after each measurement.
- B. Measurements will be made from a standard reference point at the top of the well casing.
- C. Interpolation will be used to estimate the depth to the nearest 0.01 foot. Sufficient “runs” to the top of the ground water will be attempted to assure accuracy of the measurements. The total depth of each well will be measured after the water level is determined to verify the integrity of the well.

Water Quality Handling and Analysis Plan

- D. Water levels will be reported as depths below the standard reference point and as elevations relative to mean seal level.
 - 1. Measurements obtained while drilling and immediately after completion of each monitor well will be reported on the boring logs.
 - 2. Measurements obtained during the water quality sampling program will be recorded on a field log (Figure A-1) and will be transferred to permanent records.
- E. All field and office records will be retained for reference.

6.2 Groundwater Sampling for Laboratory Analysis

A. Collection Methods

- 1. Groundwater samples will be collected following monitor well development.
- 2. Development will continue until water removed from the well is reasonably free of sand, silt and clay so that the well can be sampled without damage to the pump or bailer.
- 3. If possible, turbidity will be less than 5 NTU.
- 4. Analytes will be sampled in order of decreasing volatility.
- 5. Teflon, PVC or stainless steel bailers will be used to sample wells that do not yield adequate quantities of water to be purged by pumping. Each well will be ailed until the field parameters (temperature, pH and conductance) have stabilized, thus assuring that the sample will be representative of groundwater conditions.
- 6. Any abnormal sampling conditions that may have an effect on sampling will be recorded in the field sampling notes. Examples of such conditions would include, but would not be limited to; equipment malfunctions, unusual recharge rates of the well, unusual pumping rates, or conditions which could lead to contamination of the sample. Field notes will also record:
 - a. Whether high (pump) or low (bailer) yield procedures for well evacuation were followed.

Water Quality Handling and Analysis Plan

- b. The types of samples taken during a particular sampling event.
- c. The sample numbers.

B. Measurements

1. Field measurements and observations will be recorded on field logs which will be copied and stored for reference. A field log from for groundwater sampling is included with this Plan as Figure A-2.
2. Water Levels will be measured before sampling. The height of the water column above the screened completed interval will be used to determine three casing volumes for evacuation prior to sampling.
3. Estimated discharge rates and pumping durations necessary for ensuring evacuation of three casing volumes will be prepared to guide sampling personnel after completion of the monitor well drilling program.

C. Equipment

1. A Groundfos MP1 submersible pump will be used to pump wells. Alternatively a stainless steel PVC or Teflon bailer may be used.
2. Pumping and bailing shall be conducted to ensure that three casing volumes are evacuated before sample retention. A work sheet showing water column calculations for each of the monitor wells is enclosed as Figure A-2. Pump or bailer discharge shall be measured to verify the evacuation volume.

D. Calibration

1. Field instruments for pH and specific conductivity will be calibrated according to manufacturer's recommendations before sampling begins. Cole-Parmer pH and conductivity meter or their functional equivalents will be used.
2. Calibration standards for pH and conductivity will be chosen to be representative of values expected in the naturally occurring waters.
3. Calibrations will be rechecked after sample collection, and all calibration procedures will be documented on the sampling field log. Measurements of pH,

Water Quality Handling and Analysis Plan

conductivity and temperature will be made at the beginning and just before the end of voiding three casing volumes.

E. Storage and Handling

1. Groundwater samples will be bottled directly from the discharge of the pump or bailer. Bottles will be labeled prior to filling and stored on ice immediately after collection.
2. Sample bottles of appropriate size and with the required preservative will be obtained from the selected certified laboratory.

6.3 Procedures to Avoid Contaminating Groundwater Samples

- A. Restrict pump and bailing discharge rates so that drawdown does not cause sample aeration.
- B. Decontaminate sampling equipment prior to utilization at another site.
Decontamination methods will include:
 1. Cleaning with a non phosphate detergent.
 2. Rinsing pump and hose with culinary water
 3. Rinsing bailers with deionized or distilled water.

6.4 Sample Handling

- A. Sample containers will be (1) stored out of direct sunlight and (2) preserved, shipped and analyzed within the maximum allowable holding times as specified in Tables A-1 & A-2.
- B. Samples will be shipped to the appropriate laboratory as soon as possible on the same day as collection, but in all cases within the time required by the accepting laboratory.
- C. Other specific laboratory requirements and EPA guidelines will be observed for each parameter, including container type, preservation dosages and refrigeration.

Water Quality Handling and Analysis Plan

7.0 SAMPLE CUSTODY

7.1 Field Operations

- A. Documentation of field collection procedures and sample integrity from collection to reporting are essential parts of the Plan.
- B. Documentation of sample possession assures that samples may be traced from the time of collection through analysis and final statistical evaluation.
 - 1. Documentation of the history of the sample is referred to as chain-of-custody.

7.2 Necessary Records and Actions

- A. Sample Labels: prevent misidentification of samples. The sample label shown as Figure A-3 or its equivalent will be filled out and attached to each sample bottle before collection.
- B. Field Sampling and Analysis Records will be maintained. Pertinent field measurements and observation will be recorded.
- C. Equipment used to measure the field parameters shall be calibrated before the collection of each sample.
- D. Appropriate forms such as Figure A-2 will be filled out for each sample site. Documentation of the sources of buffers, standards, reagents, sample containers and so forth will be recorded on these forms.
- E. A chain-of-custody record (equivalent to Figure A-4) will be filled out for each set of samples. A copy will accompany every sample shipment from the time of collection through receipt by the analytical results for inclusion in the yearly reports.
- F. A copy of the form sent to the laboratory with each sample shipment will be retained with the analytical results for inclusion in the yearly reports.
- G. Jade Dalton, at his option may elect to protect sample integrity by use of seals applied in the field immediately after sampling. Such seals may be required by the State of Utah in the event that sampling is related to enforcement issues.

Water Quality Handling and Analysis Plan

7.3 Laboratory Operations

- A. The analytical laboratory will acknowledge receipt of the samples by signing and dating in the appropriate box in the form shown as Figure A-4. This form will be returned to Jade Dalton with the analytical results.
- B. The laboratory will maintain internal chain-of-custody control in accordance with protocol as per the Utah DEQ.

8.0 CALIBRATION PROCEDURES AND FREQUENCY

8.1 General

- A. Meters used to measure pH and specific conductance will be calibrated as outlined below prior to and during use. Source and identification of standards used to calibrate will be recorded on the form as presented in Figure A-2.

8.2 Field pH

- A. Field pH will be determined via a Cole Parmer pH Tester Meter (or equivalent). The meter has automatic temperature correction capabilities.
- B. Field personnel will follow the manufacturer's instructions for operation and standardization of instruments.

8.3 Standardization

- A. Standardization will utilize a buffer of 7 pH units.
- B. The meter will be sterilized prior to each sample collection and checked against the standard after each sample collection. Where sample pH values vary widely, the meter will be standardized with buffers having pH of 7 and 10.

8.4 Equipment Storage and Cleaning

- A. The pH meter electrode will be stored in accordance with the manufacturer's recommendation.
- B. Any oil on the electrodes shall be cleaned with methanol f HCL as needed.

Water Quality Handling and Analysis Plan

8.5 Field Specific Conductance

- A. Field specific conductance will be measured with a Col-Parmer Model 0481-40, or equivalent. This meter automatically indicates specific conductance normalized to 25°C.
- B. Calibration will be accomplished according to manufacturer's instruction before each measurement.

8.6 Temperature and Water Levels

- A. Temperature will be measured using a good grade mercury thermometer. Temperatures will be reported to the nearest 0 degree Fahrenheit.
- B. Water level measurements will be made with a steel tape or electronic sounding device capable of accuracy to within 0.01 feet.
- C. Water levels will be recorded in the field on the form shown as Figure A-1 along with pertinent observations.

9.0 INTERNAL QUALITY CONTROL

9.1 Field Operations

- A. At least one blind field groundwater duplicate sample will be prepared and submitted to the laboratory during alternate sampling events.
- B. Obtaining Water Samples for Duplicates:
 - 1. Water samples will be obtained directly from the pump discharge line.
 - 2. One field equipment blank will also be collected during alternate sampling events.
- C. Preparing Field Equipment Blank Sample (one of the following methods):
 - 1. Pump distilled water through the submersible pump.
 - 2. Fill sample containers from the bailer in the same manner as is done for a typical sample.

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9.2 Preservation

- A. Preservatives are planned for use in sample bottles.
- B. A trip blank for each one of the preserved sample bottle types will be included for alternate sampling events.
- C. Each of these trip blank bottles will be prepared by the laboratory (filled with distilled water and appropriate preservatives) and be subjected to the same field conditions and laboratory analytical tests as required for ground water samples.

9.3 Laboratory Operations

- A. The laboratory will conduct quality control checks in accordance with the State of Utah certification requirements.
- B. This quality control check will include running at least 5 percent duplicated and spike samples.
- C. The laboratory will summarize the results of these quality control checks and submit them with the analytical results.
- D. At least one groundwater sample from alternate sampling events will be utilized for laboratory matrix spike duplicate analyses. Field personnel will ensure that sufficient sample material is provided to the appropriate laboratory for the matrix spike.

9.4 Summary of Quality Control Samples

- A. The following “extra samples” will be analyzed during alternate sampling events.
 - 1. Groundwater duplicate samples from each upgradient well.
 - 2. One field equipment blank.
 - 3. One trip blank for each of the preserved bottle types (prepared by the laboratory).
 - 4. One laboratory matrix spike duplicate sample.

Water Quality Handling and Analysis Plan

10.0 DATA REDUCTION MANAGEMENT, VALIDATION, AND REPORTING

All field data and chain-of-custody forms generated from sampling will be appropriately identified and included in each water quality data report.

10.1 Standardization

- A. Use of standardization forms will enable consistent presentation of the data throughout the project life. Therefore, standardization data forms will be used by all field personnel as well as by the laboratory during the project.

10.2 Validation

- A. Validation of all analytical data will be performed. Laboratory will be required to submit results which are supported by sufficient back up data and QA/QC reports to enable the Quality Assurance Officer to determine the quality of the data.
- B. Validity of all data will be determined from the precision and accuracy assessments outlined in Section 5.0 of this Plan. All data will be stored and maintained according to the procedures outlined.

10.3 Data Processing

- A. Data will be processed through an orderly, easily traceable and logical sequence. Field data will be assessed for accuracy.
- B. Subsequent analysis, interpretation and reporting of results will be conducted by trained professionals, using documents which are initialed and dated whenever appropriate.
- C. Backup copies of electronic media will be prepared daily. Any calculations will be checked and all assumptions necessary for calculations will be approved by the QAO.
- D. Results will be reported with all necessary supporting documentation after proper review.

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11.0 AUDIT PROCEDURES

The CM and the QAO will monitor and audit performance of the quality assurance procedures outlined in this report. The QAO will conduct random field and office audits which will assure that the information being gathered is reliable and of good quality. This information will be provided to the DEQ Official.

11.1 Field Audits

- A. The CM or his representative will conduct unscheduled field activity audits during each sampling event. Audits will evaluate the execution of (1) sample identification, (2) sample control, (3) chain-of-custody procedures, (4) field documentation, (5) equipment calibration and (6) sampling operations.
- B. Evaluation: The following list of items will be used to evaluate the water sampling and handling:
 - 1. Field documents pertaining to sample identification and control will be examined for completeness and accuracy.
 - 2. Field documents will be reviewed to see that (1) all entries are dated and signed with waterproof ink or pencil and that (2) the contents are legible, accurate and inclusive.
 - 3. The field documents form the basis for reports and will contain all measurements and observations.
 - 4. Field instruments will be checked for proper calibration and completely prepared calibration documentation.
- C. Conformance and Security
 - 1. Sampling operations will be evaluated for conformance to Section 6.0 of this Plan. The proper number of samples will be collected at the assigned locations in proper containers with correct labels and appropriate preservatives.
 - 2. Required field measurements and quality assurance checks will be performed and documented as directed by the CM and the QAO.

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3. The CM or his representative will check chain-of-custody procedures and confirm that samples are kept in secure custody at all times.

11.2 Office Audits

- A. Upon completion of each sampling event, the individual files will be assembled, organized and securely stored.
- B. Documents will be examined to determine that all necessary signatures, dates and project numbers are included. The CM or his representative will examine all documents and determine if they have been handled and stored in the proper manner. Such files will be maintained by Jade Dalton or a member of his company.
- C. The CM or his representative will review product quality to assure that the project is being performed in accordance with approved quality assurance procedures.
- D. Prior to the production of the draft Background Groundwater Quality Report, all work products will undergo review by the QAO.
- E. QAO assessment will include review of calculation, test analysis, graphs, tables, computer input/outputs and any other document which involves interpretation of the field data.

12.0 CORRECTIVE ACTION

12.1 Criteria

- A. Corrective action will be undertaken if sample collection deficiencies or unreliable analytical results prevent QA objectives for the project from being met.
- B. Specific criteria for acceptable data collection are given in section 5.0. The QA program(s) of the selected laboratory will provide the criteria for acceptable analytical results.
- C. Analytical results supplied by the laboratory will have been subjected to the internal QA plan and will be considered to be acceptable unless the results significantly contradict previously acquired data.

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- D. If significant contradiction occurs, the QAO will request that the laboratory review the quality control documentation for the sample or analysis in question.
- E. Further corrective action will be based on the results of the documentation review.

12.2 Correction

- A. The principal corrective action that may be required as a result of deficiencies in sample collection is re-sampling. Re-sampling will be required if one or more of the following problems occur:
 - 1. Contaminating samples due to collection procedure errors which result in a sample not representative of site conditions.
 - 2. Loosing sample in transit to the laboratory.
 - 3. Surpassing holding times for required parameters.
 - 4. Trip blank showing contaminant concentrations within one order of magnitude of the original field sample.
 - 5. Ion balance in error (either plus or minus) by more than 5%.
- B. Variations between duplicate analyses, which are outside control limits, will be evaluated by the CM QAO and DEQ Official to determine whether re-sampling is required.
- C. Re-analysis may be substituted for re-sampling if the holding time has not expired and sample condition is satisfactory.
- D. A request for corrective action (RCA) may be initiated by the CM, the QAO or the DEQ Official.

13.0 QUALITY ASSURANCE REPORTS

Water quality data reports will be submitted every three months during the initial background groundwater quality report study period and annually thereafter. Quarterly sampling reports will document any deviations from field, handling or laboratory procedures contained in the approved plan.

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QA reports will be prepared annually and submitted in conjunction with water quality data reports to the DEQ, Division of Water Quality.

13.1 Contents

- A. Quality Assurance reports will contain:
 - 1. Results of system and / or performance audits of sample collection activities.
 - 2. A summary of the laboratory QA report(s), including notation of QA modifiers.
 - 3. Listing and basis for any unacceptable data.
 - 4. Discussion of significant QA problems and recommended solutions.

13.2 Format

- A. The QA report will be prepared by the QAO and the CM or his representative and distributed to the DEQ Official.
- B. The final background groundwater quality report will contain a separate QA section which will summarize the data quality information.

14.0 MONITORING STATIONS

A map of the monitor wells to be sampled is included as Figure A-5. The map shows the physical location of the wells with respect to the proposed facility location.

ATTACHMENTS

Dalton Finisher Sites

Table A-1 -- Base Line Water Sample Analysis Parameters

Parameters	Units	Analytical Methods		Preservation	Max Holding Time
		EPA	Std Methods		
Alkalinity, Carbonate as CaCO ₃	mg/l		2320 B	Cool, ≤ 6°C	14 days
Ammonia-nitrogen as N	mg/l	350.1	4500-NH ₃	Cool, ≤ 6°C, H ₂ SO ₄ to pH<2	28 days
Bicarbonate	mg/l	310.2	2320 B	Cool, ≤ 6°C	14 days
Bromide	mg/l	300.0		None Req'd	28 days
Calcium	mg/l	215.1	3111 B	HNO ₃ to pH<2	6 months
Carbon dioxide	mg/l				--
Carbonate	mg/l	310.2	2320 B	Cool, ≤ 6°C	14 days
Chloride	mg/l		4500-Cl-B	None Req'd	28 days
Hardness, Ca + Mg	mg/l		2340 B or C	HNO ₃ , H ₂ SO ₄ to pH<2	14 days
Hydroxide	mg/l				--
Inorganic nitrogen (nitrate and nitrite) as N	mg/l	353.2	4500--NO ₃ -F	Cool, ≤ 6°C, H ₂ SO ₄ to pH<2	28 days
Magnesium	mg/l	242.1	3111 B	HNO ₃ to pH<2	6 months
pH					on site
Phosphate-phosphorus as P	mg/l	365.3	4500-P-E	Cool, ≤ 6°C, H ₂ SO ₄ to pH<2	28 days
Potassium	mg/l	258.1	3111 B	HNO ₃ to pH<2	6 months
Sodium	mg/l	273.1	3111 B	HNO ₃ to pH<2	6 months
Solids, Total Dissolved	mg/l	160.1	2540-C	Cool, ≤ 6°C	7 days
Solids, Total Suspended (TSS)	mg/l	160.1	2540-C	Cool, ≤ 6°C	7 days
Specific conductance	uS/cm	120.1	2510 B	Cool, ≤ 6°C	7 days
Sulfur, sulfate (SO ₄) as SO ₄	mg/l	375.2		Cool, ≤ 6°C	28 days
Turbidity	NTU	180.1	2130 B	Cool, ≤ 6°C	48 hours

Table A-1



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Dalton Finisher Sites

Table A-2 -- Steady State Water Sample Analysis Parameters

Parameters	Units	Analytical Methods		Preservation	Max Holding Time
		EPA	Std Methods		
Alkalinity, Carbonate as CaCO ₃	mg/l		2320 B	Cool, ≤ 6°C	14 days
Ammonia-nitrogen as N	mg/l	350.1	4500-NH ₃	Cool, ≤ 6°C, H ₂ SO ₄ to pH<2	28 days
Bicarbonate	mg/l	310.2	2320 B	Cool, ≤ 6°C	14 days
Bromide	mg/l	300.0		None Req'd	28 days
Carbon dioxide	mg/l				--
Carbonate	mg/l	310.2	2320 B	Cool, ≤ 6°C	14 days
Chloride	mg/l		4500-Cl-B	None Req'd	28 days
Hydroxide	mg/l				--
Inorganic nitrogen (nitrate and nitrite) as N	mg/l	353.2	4500--NO ₃ -F	Cool, ≤ 6°C, H ₂ SO ₄ to pH<2	28 days
Kjeldahl Nitrogen, Total (TKN)	mg/l		4500-Norg B or C and 4500-NH ₃ B	Cool, ≤ 6°C	28 days
pH					on site
Phosphate-phosphorus as P	mg/l	365.3	4500-P-E	Cool, ≤ 6°C, H ₂ SO ₄ to pH<2	28 days
Solids, Total Dissolved	mg/l	160.1	2540-C	Cool, ≤ 6°C	7 days
Solids, Total Suspended (TSS)	mg/l	160.1	2540-C	Cool, ≤ 6°C	7 days
Specific conductance	uS/cm	120.1	2510 B	Cool, ≤ 6°C	7 days
Sulfur, sulfate (SO ₄) as SO ₄	mg/l	375.2		Cool, ≤ 6°C	28 days
Turbidity	NTU	180.1	2130 B	Cool, ≤ 6°C	48 hours

Table A-2



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**Dalton Finisher Sites
Monitor Well Water Level Measurements Log**

Well	Date	Time	Reference Point	Ref. Pt. Elevation	Depth (ft)	Depth to Water (ft)	Water Elevation	By:

Figure A-1



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Dalton Finisher Sites Water Quality Sampling Field Record

Well Name: _____ Date: _____

Sampling Personnel: _____

Instrument Calibrations

pH meter Calibrated? Yes

Conductivity Meter Calibrated? Yes

Field Measurements

Time	Volume Evacuated	Temp. (F)	pH	Conductivity	Comments

Base intake slots (feet below ground) _____

Top water surface (feet below ground) _____

Water Column (feet): _____ Casing - Inside Diameter: _____

Gallons of Water in Casing: _____ Gallons X 3: _____

Note: One gallon - 231 cubic inches. Height of water column in inches is obtained by multiplying the water column in feet by 12; this column height is then multiplied by the area of the casing to obtain the volume of water in cubic inches. This volume is then divided by 231 to obtain the volume of water in gallons.

Pump Started - Time: _____	Pump Stopped - Time: _____
Pump Started - Time: _____	Pump Stopped - Time: _____
Pump Started - Time: _____	Pump Stopped - Time: _____
Pump Started - Time: _____	Pump Stopped - Time: _____
Pump Rate (gpm): _____	Total Time Pumped (min): _____

Volume evacuated before sampling (gal): _____

Notes: _____

Figure A-2

**Dalton Finisher Sites
Field Water Sample Label**

Well Name: _____

Sample Number: _____

Analytical parameter(s): dfsd

Date Sampled: _____

Time Sampled: _____

Sampler: _____

Preservative: Acid Base Filtered

Destination Laboratory: _____

Figure A-3

Dalton Finsier Sites Field Water Sample - Chain-of-Custody Record

Sampler Signature: _____

Sample ID	Sample Source	Sampled Date & Time:	# of Containers	Parameters to Analyze		
				Group 1 Characteristics	Group 2 Characteristics	Other

Group 1 Characteristics:

- | | | | |
|--|---|-------------------|---|
| Alkalinity, Carbonate as CaCO ₃ | Ammonia-nitrogen as N | Bicarbonate | Carbon Dioxide |
| Carbonate | Chloride | Hydroxide | Inorganic nitrogen (nitrate & nitrite) as N |
| pH | Phosphate-phosphorus as P | Solids, Dissolved | Solids, Total Suspended (TSS) |
| Specific Conductance | Sulfur, sulfate (SO ₄) as SO ₄ | Turbidity | |

Group 2 Characteristics:

- | | | | | |
|---------|-----------|-----------|-----------|--------|
| Calcium | Hydroxide | Magnesium | Potassium | Sodium |
|---------|-----------|-----------|-----------|--------|

Relinquished By:	Date & Time	Sent Via	Received By:	Date & Time

Notes:

Figure A-4



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